

# **DRAINAGE REPORT**

**For**

**Route 85 Realty Corp.**

**PROPOSED**

***Bank with Drive Thru***

***182 & 186 Hartford Avenue  
Bellingham, Massachusetts  
Norfolk County***

Prepared by:

BOHLER ENGINEERING  
352 Turnpike Road  
Southborough, MA 01772  
(508) 480-9900 TEL.

John A Kucich  
Massachusetts P.E. Lic. #41530

**BOHLER //**

February 5, 2025  
#MAA240489.00

**TABLE OF CONTENTS**

I. EXECUTIVE SUMMARY .....	1
II. EXISTING SITE CONDITIONS .....	2
Existing Site Description.....	2
On-Site Soil Information .....	2
Existing Collection and Conveyance .....	2
Existing Watersheds and Design Point Information.....	2
III. PROPOSED SITE CONDITIONS.....	4
Proposed Development Description .....	4
Proposed Development Collection and Conveyance .....	4
Proposed Watersheds and Design Point Information.....	4
IV. METHODOLOGY .....	6
Peak Flow Calculations .....	6
V. STORMWATER MANAGEMENT STANDARDS.....	7
Standard #1: No New Untreated Discharges .....	7
Standard #2: Peak Rate Attenuation .....	7
Standard #3: Recharge .....	7
Standard #4: Water Quality .....	8
Standard #5: Land Use with Higher Potential Pollutant Loads .....	8
Standard #6: Critical Areas .....	8
Standard #7: Redevelopment.....	8
Standard #8: Construction Period Pollution Prevention and Erosion and Sedimentation Control.....	8
Standard #9: Operation and Maintenance Plan (O&M Plan).....	9
Standard #10: Prohibition of Illicit Discharges .....	9
VI. SUMMARY .....	10

**LIST OF TABLES**

Table 1.1: Design Point Peak Runoff Rate Summary .....	1
Table 2.1: Existing Soil Information .....	2
Table 2.2: Existing Sub-Catchment Summary.....	3
Table 3.1: Proposed Sub-catchment Summary.....	5
Table 4.1: NOAA Rainfall Intensities .....	6
Table 6.1: Design Point Peak Runoff Rate Summary .....	10

## **APPENDICES**

APPENDIX A: MASSACHUSETTS STORMWATER MANAGEMENT CHECKLIST

APPENDIX B: PROJECT LOCATION MAPS

- USGS MAP
- FEMA FIRMETTE

APPENDIX C: SOIL AND WETLAND INFORMATION

- NCRS CUSTOM SOIL RESOURCE REPORT

APPENDIX D: EXISTING CONDITIONS HYDROLOGIC ANALYSIS

- EXISTING CONDITIONS DRAINAGE MAP
- EXISTING CONDITIONS HYDROCAD COMPUTATIONS

APPENDIX E: PROPOSED CONDITIONS HYDROLOGIC ANALYSIS

- PROPOSED CONDITIONS DRAINAGE MAP
- PROPOSED CONDITIONS HYDROCAD CALCULATIONS

APPENDIX F: STORMWATER CALCULATIONS

- MA STANDARD #3 – RECHARGE AND DRAWDOWN TIME
- MA STANDARD #4 – WATER QUALITY AND TSS REMOVAL
- NOAA RAINFALL DATA

APPENDIX G: OPERATION AND MAINTENANCE

- STORMWATER OPERATION AND MAINTENANCE PLAN
- INSPECTION REPORT
- INSPECTION AND MAINTENANCE LOG FORM
- LONG-TERM POLLUTION PREVENTION PLAN
- ILLICIT DISCHARGE STATEMENT
- SPILL PREVENTION
- PROPOSED OPERATION AND MAINTENANCE MAP
- MANUFACTURER'S INSPECTION AND MAINTENANCE MANUALS

APPENDIX H: CONSTRUCTION INSPECTION AND CONTROL

- STORMWATER INSPECTION REPORT
- STORMWATER SEDIMENTATION AND EROSION CONTROL PLANS
- STAGNATION PREVENTION AND MOSQUITO CONTROL PLAN

## I. EXECUTIVE SUMMARY

This report examines the changes in drainage that can be expected as the result of the development of a proposed bank with drive thru located on the northwest corner of the intersection of Hartford Avenue and Cedar Hill Road in the Town of Bellingham, Massachusetts. The site, which contains approximately 0.91 acres of land, contains two (2) existing residences. The remaining portion of the site is undeveloped consisting of grass and landscaped areas.

The proposed project includes the construction of a new 3,500 sf freestanding bank with drive thru along with new paved parking areas, landscaping, storm water management components and associated utilities. This report addresses a comparative analysis of the pre- and post-development site runoff conditions. Additionally, this report provides calculations documenting the design of the proposed stormwater conveyance/management system as illustrated within the accompanying Site Development Plans prepared by Bohler. The project will also provide erosion and sedimentation controls during the demolition and construction periods, as well as long term stabilization of the site.

For the purposes of this analysis the pre- and post-development drainage conditions were analyzed at one (1) "design point" where stormwater runoff currently drains to under existing conditions. This design point is described in further detail in **Section II** below. A summary of the existing and proposed conditions peak runoff rates for the 2-, 10-, 25-, and 100-year storms can be found in **Table 1.1** below. In addition, the project has been designed to meet or exceed the Stormwater Management Standards as detailed herein.

**Table 1.1: Design Point Peak Runoff Rate Summary**

Point of Analysis	2-Year Storm			10-Year Storm			25-Year Storm			100-Year Storm		
	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ
DP1	1.40	0.21	<b>-1.19</b>	3.50	1.96	<b>-1.54</b>	5.33	3.65	<b>-1.68</b>	8.50	5.61	<b>-2.89</b>

*\*Flows are represented in cubic feet per second (cfs)*

## II. EXISTING SITE CONDITIONS

### Existing Site Description

The site consists of approximately 0.91 acres of land located at the northwest corner of the intersection of Hartford Avenue and Cedar Hill Avenue in the Town of Bellingham, Massachusetts. The southern portion of the site contains two (2) existing residences. The remaining portion of the site is undeveloped consisting of grass and landscaped areas.

### On-Site Soil Information

Soils within the analyzed area consist of the following as classified by the Natural Resource Conservation Service (NRCS):

**Table 2.1: Existing Soil Information**

<b>Soil Unit Symbol</b>	<b>Soil Name / Description</b>	<b>Hydrologic Soil Group (HSG)</b>
103B	Charlton-Hollis-Rock outcrop complex	A
300C	Montauk fine sandy loam	C

Refer to **Appendix C** for additional information.

### Existing Collection and Conveyance

The site drains to the south and into the Hartford Avenue municipal drainage system. Slopes on the site range from 1%-40% with on-site elevations ranging from 279 along the northern property boundary to 265 at the southwest property corner adjacent to Hartford Avenue.

### Existing Watersheds and Design Point Information

For the purposes of this analysis, the pre- and post-development drainage conditions were analyzed at one (1) "design point" as described below where stormwater runoff currently drains to under existing conditions. The existing site was subdivided into one (1) separate sub catchments, as described below, to analyze existing and proposed flow rates at each design point. The minimum time of concentration for all proposed areas is calculated as 6 minutes (0.1 hr).

Design Point #1 (DP1) is the Hartford Avenue right-of-way. Under existing conditions, this design point receives stormwater flows from approximately 1.0 acres of land, designated as watershed "ED1.1". Refer to Table 2.1 below for additional detail.

**Table 2.2: Existing Sub-Catchment Summary**

Sub-catchment Name	Total Area (acres)	Cover Description	Curve Number (CN)	Time of Concentration (Tc, minutes)
ED1.1	1.0±	Rooftops, paved parking, grass	71	6.0

Refer to **Table 1.1 and 6.1** for the existing conditions peak rates of runoff. Refer to **Appendix D** and the Drainage Area Maps in the appendices of this report for a graphical representation of the existing drainage areas.

### III. PROPOSED SITE CONDITIONS

#### Proposed Development Description

The proposed project consists of the construction of a new 3,500 sf freestanding bank with drive thru including paved parking areas, landscaping, associated utilities, and a new stormwater management system. The site, including the proposed parking areas, has been designed to drain to deep-sump, hooded catch basins. The catch basins will capture and convey stormwater runoff, via an underground pipe system, to a proposed underground infiltration system. Pretreatment of stormwater runoff will be provided by a combination of the deep-sump, hooded catch basins and an Isolator Row of chambers prior to discharge to the proposed underground infiltration system. Rooftop runoff has been designed to flow to the underground system as well.

#### Proposed Development Collection and Conveyance

Deep sump hooded catch basins are proposed to collect and route runoff from the paved parking areas to the proposed underground infiltration system. Pipes have been designed for the 25-year storm using the Rational Method.

The best management practices (BMPs) incorporated into the proposed stormwater management system have been designed to meet, or exceed, the standards set forth in the Massachusetts Department of Environmental Protection Stormwater Handbook standards. Refer to **Section V** for additional information.

#### Proposed Watersheds and Design Point Information

The project has been designed to maintain existing drainage watersheds to the greatest extent possible, with the same design points described in **Section II** above. The site was subdivided into two (2) separate sub catchments for the proposed conditions as described below. The minimum time of concentration for all proposed areas is calculated as 6 minutes (0.1 hr).

Under proposed conditions DP1 receives stormwater flows from approximately 1.0 acres of land, designated as watersheds "P1.1" and "P1.2". Refer to Table 3.1 below for additional detail.

**Table 3.1: Proposed Sub-catchment Summary**

Sub-catchment Name	Total Area (acres)	Cover Description	Curve Number (CN)	Time of Concentration (Tc, minutes)	Hydrologic Routing
P1.1	0.82±	Rooftops, paved parking, grass	85	6.0	UGS1 / DP1
P1.2	0.22±	Paved parking, grass	62	6.0	DP1

Refer to **Table 1.1 and 6.1** for the calculated proposed conditions peak rates of runoff. For additional hydrologic information, refer to **Appendix D** and the Drainage Area Maps in the appendices of this report for a graphical representation of the proposed drainage areas.

#### IV. METHODOLOGY

##### Peak Flow Calculations

Methodology utilized to design the proposed stormwater management system includes compliance with the guidelines set forth in the latest edition of the Massachusetts DEP Stormwater Handbook. The pre- and post-development runoff rates being discharged from the site were computed using the HydroCAD computer program. The drainage area and outlet information were entered into the program, which routes storm flows based on NRCS TR-20 and TR-55 methods. The other components of the model were determined following standard NRCS procedures for Curve Numbers (CNs) and times of concentrations documented in the appendices of this report. The rainfall data utilized and listed below in table 4.1 below for stormwater calculations is based on NOAA. Refer to **Appendix F** for more information.

**Table 4.1: NOAA Rainfall Intensities**

Frequency	2 year	10 year	25 year	100 year
Rainfall* (inches)	4.18	6.55	8.42	11.5

\*Values derived from NOAA ATLAS 90% Intervals on 02/03/2025

The proposed stormwater management as designed will provide a decrease in peak rates of runoff from the proposed facility for the 2-, 10-, 25- and 100-year design storm events. Additionally, the proposed project meets, or exceeds, the MADEP Stormwater Management standards. Compliance with these standards is described further below.

## V. STORMWATER MANAGEMENT STANDARDS

### **Standard #1: No New Untreated Discharges**

The project has been designed so that proposed impervious areas (including the building roof and paved parking/driveway areas) shall be collected and passed through the proposed drainage system for treatment prior to discharge.

### **Standard #2: Peak Rate Attenuation**

As outlined in **Table 1.1** and **Table 6.1**, the development of the site and the proposed stormwater management system, have been designed so that post-development peak rates of runoff are below pre-development conditions for the 2-, 10-, 25- and 100-year storm events at all design points.

### **Standard #3: Recharge**

The stormwater runoff from the project will be collected and diverted to a proposed underground infiltration system. The project as proposed will involve the creation of 26,727 square feet of new impervious area and is required to infiltrate 433 cubic feet of stormwater as defined in Stormwater Standard 3. The proposed infiltration basin will provide 4,225 cubic feet of volume below the lowest outlet for groundwater recharge. Refer to **Appendix F** of this report for calculations documenting required and provided recharge volumes.

The DEP Stormwater Standards require that the infiltration BMP drains completely within 72 hours of the end of the storm event. Calculations showing that the proposed underground infiltration system will drain within 71.3 hours are included in **Appendix F** of this report.

A four (4) foot separation to estimated seasonal high groundwater is provided and a groundwater mounding analysis is not required.

**Standard #4: Water Quality**

Water quality treatment is provided via deep-sump, hooded catch basins, an Isolator Row of chambers, and an underground infiltration system. TSS removal calculations are included in **Appendix F** of this report. The project as proposed will involve the creation of 26,727 square feet of new impervious area and is required to treat 2,227 cubic feet of water quality volume as defined in Stormwater Standard 4. The proposed infiltration basin provides 4,225 cubic feet of water quality volume below the lowest outlet for water quality treatment. Refer to **Appendix F** of this report for calculations documenting required and provided water quality volumes.

**Standard #5: Land Use with Higher Potential Pollutant Loads**

Not Applicable for this project.

**Standard #6: Critical Areas**

Not Applicable for this project.

**Standard #7: Redevelopment**

Not Applicable for this project.

**Standard #8: Construction Period Pollution Prevention and Erosion and Sedimentation Control**

The proposed project will provide construction period erosion and sedimentation controls as indicated within the site plan set provided for this project. This includes a proposed construction exit, protection for stormwater inlets, protection around temporary material stock piles and various other techniques as outlined on the erosion and sediment control sheets. Additionally, the project is required to file a Notice of Intent with the US EPA and implement a Stormwater Pollution Prevention Plan (SWPPP) during the construction period. The SWPPP will be prepared prior to the start of construction and will be implemented by the site contractor under the guidance and responsibility of the project's proponent. Refer to **Appendix H**.

**Standard #9: Operation and Maintenance Plan (O&M Plan)**

An Operation and Maintenance (O&M) Plan for this site has been prepared and is included in **Appendix G** of this report. The O&M Plan outlines procedures and time tables for the long term operation and maintenance of the proposed site stormwater management system, including initial inspections upon completion of construction, and periodic monitoring of the system components, in accordance with established practices and the manufacturer's recommendations. The O&M Plan includes a list of responsible parties and an estimated budget for inspections and maintenance.

**Standard #10: Prohibition of Illicit Discharges**

The proposed stormwater system will only convey allowable non-stormwater discharges (firefighting waters, irrigation, air conditioning condensates, etc.) and will not contain any illicit discharges from prohibited sources. An Illicit Discharge Statement is included in **Appendix G** of this report.

## VI. SUMMARY

In summary, the proposed stormwater management system illustrated on the drawings prepared by Bohler results in a reduction in peak rates of runoff from the subject site when compared to pre-development conditions for the 2-, 10-, 25- and 100-year storm frequencies. In addition, the proposed best management practices will result in an effective removal of total suspended solids from the post-development runoff. The pre-development versus post-development stormwater discharge comparisons are contained in **Table 6.1** below:

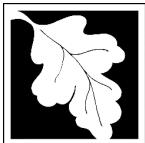
**Table 6.1: Design Point Peak Runoff Rate Summary**

Point of Analysis	2-Year Storm			10-Year Storm			25-Year Storm			100-Year Storm		
	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ
DP1	1.40	0.21	<b>-1.19</b>	3.50	1.96	<b>-1.54</b>	5.33	3.65	<b>-1.68</b>	8.50	5.61	<b>-2.89</b>

*\*Flows are represented in cubic feet per second (cfs)*

As outlined in the table above, the proposed stormwater management system as designed will provide a decrease in peak rates of runoff from the proposed facility for the 2-, 10-, 25- and 100-year storm events. Additionally, the project meets or exceeds the MADEP Stormwater Management Standards as described further herein.

**APPENDIX A: MASSACHUSETTS STORMWATER MANAGEMENT CHECKLIST**



# Checklist for Stormwater Report

## A. Introduction

**Important:** When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.<sup>1</sup> This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8<sup>2</sup>
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

<sup>1</sup> The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

<sup>2</sup> For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



# Checklist for Stormwater Report

---

## B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

**Note:** Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

---

### Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

\_\_\_\_\_  
Signature and Date

---

### Checklist

**Project Type:** Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
- Redevelopment
- Mix of New Development and Redevelopment



# Checklist for Stormwater Report

---

## Checklist (continued)

**LID Measures:** Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
  - Credit 1
  - Credit 2
  - Credit 3
- Use of "country drainage" versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe): Underground Infiltration System

---

## Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

### Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
  - Static
  - Simple Dynamic
  - Dynamic Field<sup>1</sup>
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
  - Site is comprised solely of C and D soils and/or bedrock at the land surface
  - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
  - Solid Waste Landfill pursuant to 310 CMR 19.000
  - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

---

<sup>1</sup> 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

### Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
  - is within the Zone II or Interim Wellhead Protection Area
  - is near or to other critical areas
  - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
  - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
  - The ½" or 1" Water Quality Volume or
  - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the proprietary BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

### Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does **not** cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

### Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- Limited Project
- Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.

Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



# Checklist for Stormwater Report

---

## Checklist (continued)

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is **not** covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

### Standard 9: Operation and Maintenance Plan

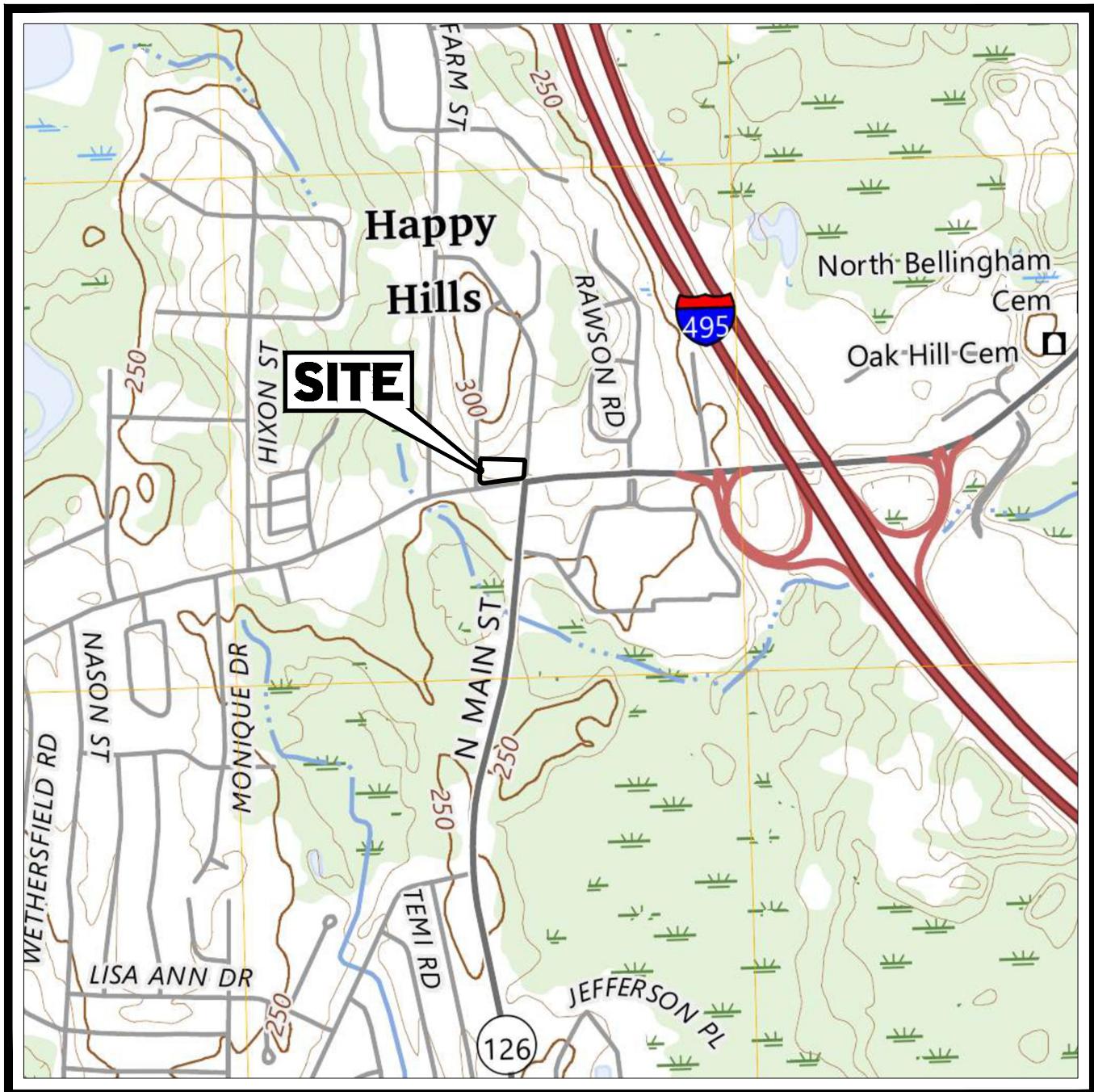
- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
  - Name of the stormwater management system owners;
  - Party responsible for operation and maintenance;
  - Schedule for implementation of routine and non-routine maintenance tasks;
  - Plan showing the location of all stormwater BMPs maintenance access areas;
  - Description and delineation of public safety features;
  - Estimated operation and maintenance budget; and
  - Operation and Maintenance Log Form.
- The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
  - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
  - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

### Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

## **APPENDIX B: PROJECT LOCATION MAPS**

- USGS MAP
- FEMA FIRMETTE



## TAX MAP

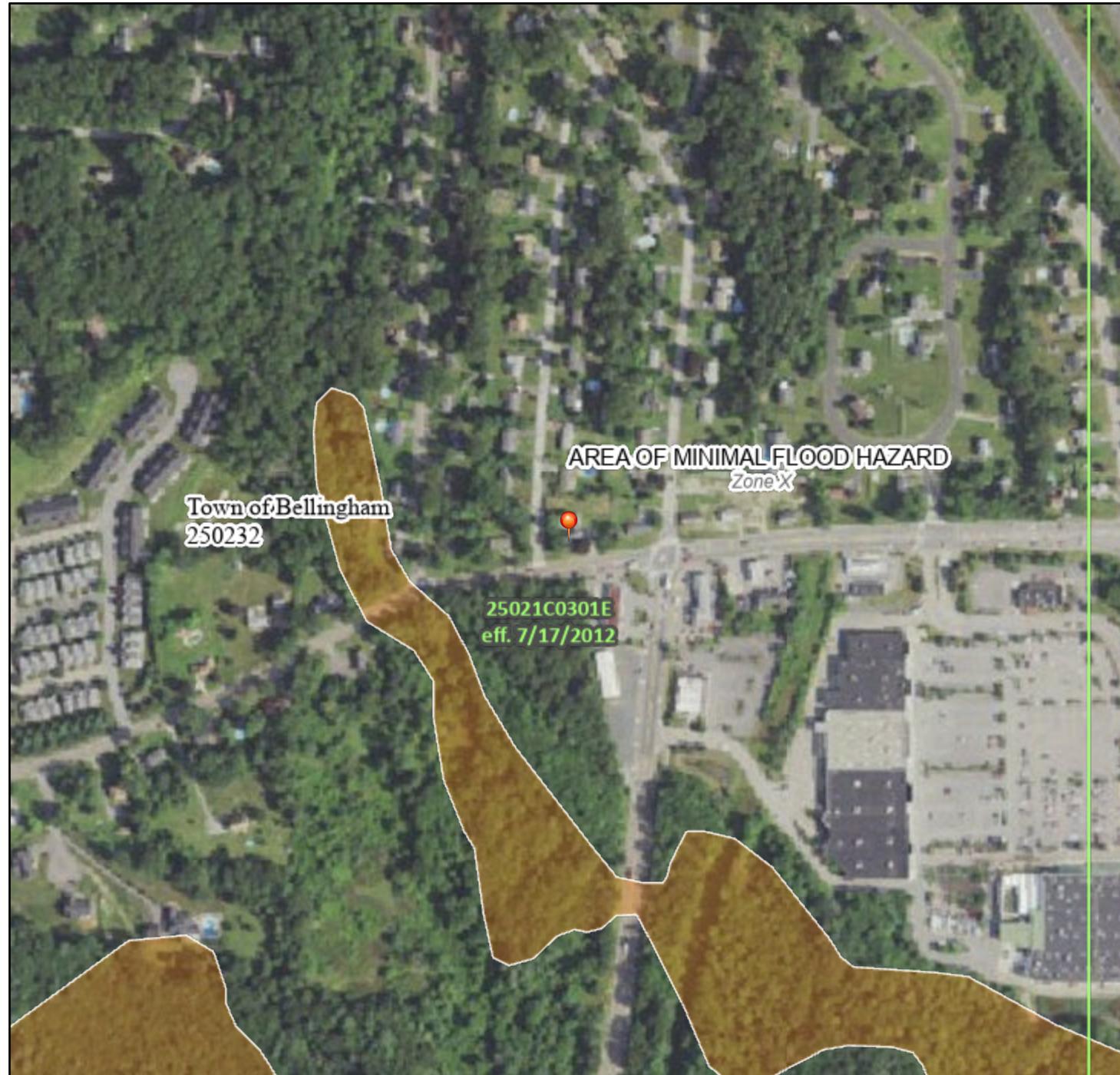
SCALE: 1" = 1,000'

SOURCE: USGS FRANKLIN, MA,  
RI QUADRANGLE, 2024

# National Flood Hazard Layer FIRMette



71°28'44"W 42°7'4"N



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

### SPECIAL FLOOD HAZARD AREAS

- Without Base Flood Elevation (BFE)  
Zone A, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
- Future Conditions 1% Annual Chance Flood Hazard Zone X

### OTHER AREAS OF FLOOD HAZARD

- Area with Reduced Flood Risk due to Levee. See Notes. Zone X
- Area with Flood Risk due to Levee Zone D

### OTHER AREAS

- NO SCREEN Area of Minimal Flood Hazard Zone X
- Effective LOMRs

### GENERAL STRUCTURES

- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation

- 8 Coastal Transect
- 513 Base Flood Elevation Line (BFE)

- Limit of Study
- Jurisdiction Boundary

- Coastal Transect Baseline
- Profile Baseline

- Hydrographic Feature

- Digital Data Available
- No Digital Data Available
- Unmapped



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 2/5/2025 at 10:34 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

## **APPENDIX C: SOIL AND WETLAND INFORMATION**

- NCRS CUSTOM SOIL RESOURCE REPORT

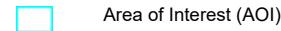
Hydrologic Soil Group—Norfolk and Suffolk Counties, Massachusetts



Natural Resources  
Conservation Service

Web Soil Survey  
National Cooperative Soil Survey

1/23/2025  
Page 1 of 4

**MAP LEGEND****Area of Interest (AOI)****Soils****Soil Rating Polygons**

	A
	A/D
	B
	B/D
	C
	C/D
	D
	Not rated or not available

**Soil Rating Lines**

	A
	A/D
	B
	B/D
	C
	C/D
	D
	Not rated or not available

**Soil Rating Points**

	A
	A/D
	B
	B/D

## C

## C/D

## D

## Not rated or not available

**Water Features**

## Streams and Canals

**Transportation**

## Rails



## Interstate Highways



## US Routes



## Major Roads



## Local Roads

**Background**

## Aerial Photography

**MAP INFORMATION**

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts  
Survey Area Data: Version 20, Aug 27, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
103B	Charlton-Hollis-Rock outcrop complex, 3 to 8 percent slopes	A	0.6	39.1%
300C	Montauk fine sandy loam, 8 to 15 percent slopes	C	0.9	60.9%
<b>Totals for Area of Interest</b>			<b>1.5</b>	<b>100.0%</b>

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

*Aggregation Method:* Dominant Condition

*Component Percent Cutoff:* None Specified

*Tie-break Rule:* Higher

## **APPENDIX D: EXISTING CONDITIONS HYDROLOGIC ANALYSIS**

- EXISTING CONDITIONS DRAINAGE MAP
- EXISTING CONDITIONS HYDROCAD COMPUTATIONS



**BOHLER //**

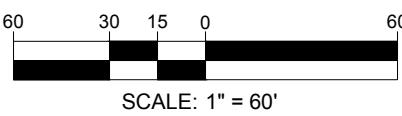
352 TURNPIKE ROAD, 3rd FLOOR  
SOUTHBOROUGH, MA 01772  
Phone: (508) 480-9900

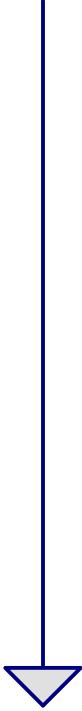
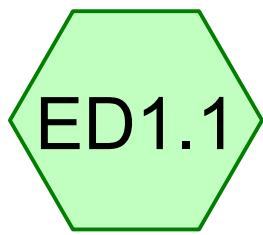
[www.BohlerEngineering.com](http://www.BohlerEngineering.com)

# PREDEVELOPMENT WATERSHED MAP

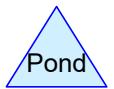
ROUTE 85  
REALTY CORP  
BELLINGHAM, MA 02019

01/23/2025 | SB | MAA240489.00 | REV 0a





Right of Way



**Routing Diagram for MAA240489 - Existing**  
Prepared by Bohler, Printed 2/5/2025  
HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
0.246	39	>75% Grass cover, Good, HSG A (ED1.1)
0.560	74	>75% Grass cover, Good, HSG C (ED1.1)
0.073	98	Unconnected pavement, HSG A (ED1.1)
0.081	98	Unconnected pavement, HSG C (ED1.1)
0.026	98	Unconnected roofs, HSG A (ED1.1)
0.051	98	Unconnected roofs, HSG C (ED1.1)
<b>1.037</b>	<b>71</b>	<b>TOTAL AREA</b>

**MAA240489 - Existing**

Prepared by Bohler

HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

Printed 2/5/2025

Page 3

**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.345	HSG A	ED1.1
0.000	HSG B	
0.692	HSG C	ED1.1
0.000	HSG D	
0.000	Other	
<b>1.037</b>		<b>TOTAL AREA</b>

**MAA240489 - Existing**

Prepared by Bohler

HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

Printed 2/5/2025

Page 4

**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.246	0.000	0.560	0.000	0.000	0.806	>75% Grass cover, Good	ED1.1
0.073	0.000	0.081	0.000	0.000	0.155	Unconnected pavement	ED1.1
0.026	0.000	0.051	0.000	0.000	0.076	Unconnected roofs	ED1.1
<b>0.345</b>	<b>0.000</b>	<b>0.692</b>	<b>0.000</b>	<b>0.000</b>	<b>1.037</b>	<b>TOTAL AREA</b>	

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment ED1.1:**

Runoff Area=45,180 sf 22.25% Impervious Runoff Depth=0.00"  
Flow Length=144' Tc=6.0 min UI Adjusted CN=67 Runoff=0.00 cfs 0.000 af

**Link DP1: Right of Way**

Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.000 af Average Runoff Depth = 0.00"**  
**77.75% Pervious = 0.806 ac 22.25% Impervious = 0.231 ac**

### Summary for Subcatchment ED1.1:

Runoff = 0.00 cfs @ 23.98 hrs, Volume= 0.000 af, Depth= 0.00"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 1-inch Rainfall=1.00"

Area (sf)	CN	Adj	Description
1,112	98		Unconnected roofs, HSG A
3,187	98		Unconnected pavement, HSG A
10,724	39		>75% Grass cover, Good, HSG A
2,204	98		Unconnected roofs, HSG C
3,548	98		Unconnected pavement, HSG C
24,405	74		>75% Grass cover, Good, HSG C
45,180	71	67	Weighted Average, UI Adjusted
35,129			77.75% Pervious Area
10,051			22.25% Impervious Area
10,051			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.2	50	0.0600	0.26		<b>Sheet Flow, A-B</b> Grass: Short n= 0.150 P2= 4.18"
0.9	69	0.0362	1.33		<b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps
0.1	20	0.2000	3.13		<b>Shallow Concentrated Flow, C-D</b> Short Grass Pasture Kv= 7.0 fps
0.0	5	0.0150	2.49		<b>Shallow Concentrated Flow, D-E</b> Paved Kv= 20.3 fps
1.8					<b>Direct Entry, Adjust to 6-minute minimum</b>
6.0	144	Total			

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 22.25% Impervious, Inflow Depth = 0.00" for 1-inch event

Inflow = 0.00 cfs @ 23.98 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 23.98 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment ED1.1:**

Runoff Area=45,180 sf 22.25% Impervious Runoff Depth=1.26"  
Flow Length=144' Tc=6.0 min UI Adjusted CN=67 Runoff=1.40 cfs 0.109 af

**Link DP1: Right of Way**

Inflow=1.40 cfs 0.109 af  
Primary=1.40 cfs 0.109 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.109 af Average Runoff Depth = 1.26"**  
**77.75% Pervious = 0.806 ac 22.25% Impervious = 0.231 ac**

### Summary for Subcatchment ED1.1:

Runoff = 1.40 cfs @ 12.10 hrs, Volume= 0.109 af, Depth= 1.26"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 2-Year Rainfall=4.18"

Area (sf)	CN	Adj	Description
1,112	98		Unconnected roofs, HSG A
3,187	98		Unconnected pavement, HSG A
10,724	39		>75% Grass cover, Good, HSG A
2,204	98		Unconnected roofs, HSG C
3,548	98		Unconnected pavement, HSG C
24,405	74		>75% Grass cover, Good, HSG C
45,180	71	67	Weighted Average, UI Adjusted
35,129			77.75% Pervious Area
10,051			22.25% Impervious Area
10,051			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.2	50	0.0600	0.26		<b>Sheet Flow, A-B</b> Grass: Short n= 0.150 P2= 4.18"
0.9	69	0.0362	1.33		<b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps
0.1	20	0.2000	3.13		<b>Shallow Concentrated Flow, C-D</b> Short Grass Pasture Kv= 7.0 fps
0.0	5	0.0150	2.49		<b>Shallow Concentrated Flow, D-E</b> Paved Kv= 20.3 fps
1.8					<b>Direct Entry, Adjust to 6-minute minimum</b>
6.0	144	Total			

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 22.25% Impervious, Inflow Depth = 1.26" for 2-Year event

Inflow = 1.40 cfs @ 12.10 hrs, Volume= 0.109 af

Primary = 1.40 cfs @ 12.10 hrs, Volume= 0.109 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment ED1.1:**

Runoff Area=45,180 sf 22.25% Impervious Runoff Depth=2.95"  
Flow Length=144' Tc=6.0 min UI Adjusted CN=67 Runoff=3.50 cfs 0.255 af

**Link DP1: Right of Way**

Inflow=3.50 cfs 0.255 af  
Primary=3.50 cfs 0.255 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.255 af Average Runoff Depth = 2.95"**  
**77.75% Pervious = 0.806 ac 22.25% Impervious = 0.231 ac**

### Summary for Subcatchment ED1.1:

Runoff = 3.50 cfs @ 12.10 hrs, Volume= 0.255 af, Depth= 2.95"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10-Year Rainfall=6.55"

Area (sf)	CN	Adj	Description
1,112	98		Unconnected roofs, HSG A
3,187	98		Unconnected pavement, HSG A
10,724	39		>75% Grass cover, Good, HSG A
2,204	98		Unconnected roofs, HSG C
3,548	98		Unconnected pavement, HSG C
24,405	74		>75% Grass cover, Good, HSG C

45,180	71	67	Weighted Average, UI Adjusted
35,129			77.75% Pervious Area
10,051			22.25% Impervious Area
10,051			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.2	50	0.0600	0.26		<b>Sheet Flow, A-B</b> Grass: Short n= 0.150 P2= 4.18"
0.9	69	0.0362	1.33		<b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps
0.1	20	0.2000	3.13		<b>Shallow Concentrated Flow, C-D</b> Short Grass Pasture Kv= 7.0 fps
0.0	5	0.0150	2.49		<b>Shallow Concentrated Flow, D-E</b> Paved Kv= 20.3 fps
1.8					<b>Direct Entry, Adjust to 6-minute minimum</b>
6.0	144	Total			

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 22.25% Impervious, Inflow Depth = 2.95" for 10-Year event

Inflow = 3.50 cfs @ 12.10 hrs, Volume= 0.255 af

Primary = 3.50 cfs @ 12.10 hrs, Volume= 0.255 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment ED1.1:**

Runoff Area=45,180 sf 22.25% Impervious Runoff Depth=4.47"  
Flow Length=144' Tc=6.0 min UI Adjusted CN=67 Runoff=5.33 cfs 0.387 af

**Link DP1: Right of Way**

Inflow=5.33 cfs 0.387 af  
Primary=5.33 cfs 0.387 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.387 af Average Runoff Depth = 4.47"**  
**77.75% Pervious = 0.806 ac 22.25% Impervious = 0.231 ac**

### Summary for Subcatchment ED1.1:

Runoff = 5.33 cfs @ 12.09 hrs, Volume= 0.387 af, Depth= 4.47"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25-Year Rainfall=8.42"

Area (sf)	CN	Adj	Description
1,112	98		Unconnected roofs, HSG A
3,187	98		Unconnected pavement, HSG A
10,724	39		>75% Grass cover, Good, HSG A
2,204	98		Unconnected roofs, HSG C
3,548	98		Unconnected pavement, HSG C
24,405	74		>75% Grass cover, Good, HSG C

45,180	71	67	Weighted Average, UI Adjusted
35,129			77.75% Pervious Area
10,051			22.25% Impervious Area
10,051			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.2	50	0.0600	0.26		<b>Sheet Flow, A-B</b> Grass: Short n= 0.150 P2= 4.18"
0.9	69	0.0362	1.33		<b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps
0.1	20	0.2000	3.13		<b>Shallow Concentrated Flow, C-D</b> Short Grass Pasture Kv= 7.0 fps
0.0	5	0.0150	2.49		<b>Shallow Concentrated Flow, D-E</b> Paved Kv= 20.3 fps
1.8					<b>Direct Entry, Adjust to 6-minute minimum</b>
6.0	144	Total			

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 22.25% Impervious, Inflow Depth = 4.47" for 25-Year event

Inflow = 5.33 cfs @ 12.09 hrs, Volume= 0.387 af

Primary = 5.33 cfs @ 12.09 hrs, Volume= 0.387 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment ED1.1:**

Runoff Area=45,180 sf 22.25% Impervious Runoff Depth=7.16"  
Flow Length=144' Tc=6.0 min UI Adjusted CN=67 Runoff=8.50 cfs 0.619 af

**Link DP1: Right of Way**

Inflow=8.50 cfs 0.619 af  
Primary=8.50 cfs 0.619 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.619 af Average Runoff Depth = 7.16"**  
**77.75% Pervious = 0.806 ac 22.25% Impervious = 0.231 ac**

### Summary for Subcatchment ED1.1:

Runoff = 8.50 cfs @ 12.09 hrs, Volume= 0.619 af, Depth= 7.16"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-Year Rainfall=11.50"

Area (sf)	CN	Adj	Description
1,112	98		Unconnected roofs, HSG A
3,187	98		Unconnected pavement, HSG A
10,724	39		>75% Grass cover, Good, HSG A
2,204	98		Unconnected roofs, HSG C
3,548	98		Unconnected pavement, HSG C
24,405	74		>75% Grass cover, Good, HSG C
45,180	71	67	Weighted Average, UI Adjusted
35,129			77.75% Pervious Area
10,051			22.25% Impervious Area
10,051			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.2	50	0.0600	0.26		<b>Sheet Flow, A-B</b> Grass: Short n= 0.150 P2= 4.18"
0.9	69	0.0362	1.33		<b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps
0.1	20	0.2000	3.13		<b>Shallow Concentrated Flow, C-D</b> Short Grass Pasture Kv= 7.0 fps
0.0	5	0.0150	2.49		<b>Shallow Concentrated Flow, D-E</b> Paved Kv= 20.3 fps
1.8					<b>Direct Entry, Adjust to 6-minute minimum</b>
6.0	144	Total			

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 22.25% Impervious, Inflow Depth = 7.16" for 100-Year event

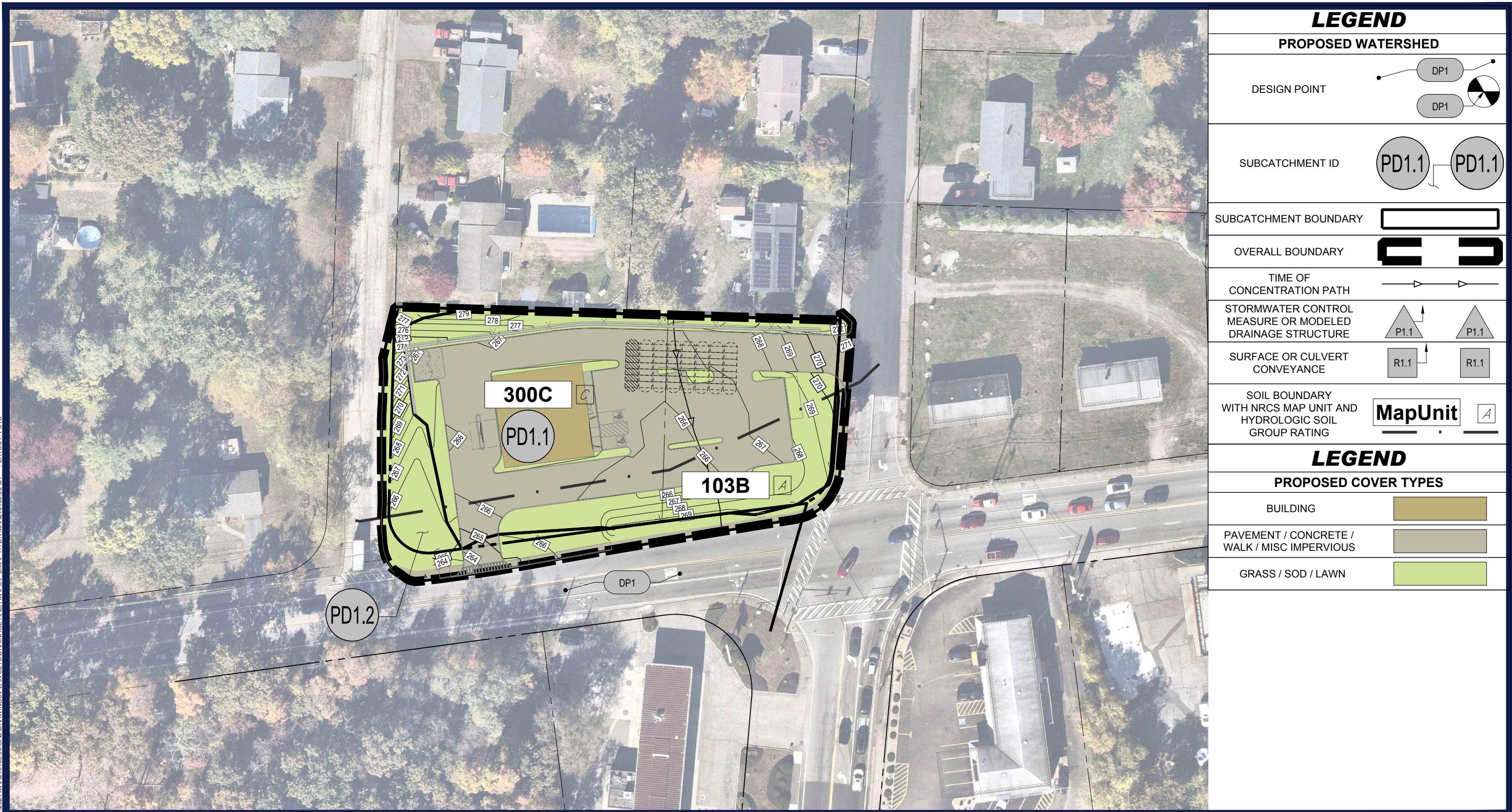
Inflow = 8.50 cfs @ 12.09 hrs, Volume= 0.619 af

Primary = 8.50 cfs @ 12.09 hrs, Volume= 0.619 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

## **APPENDIX E: PROPOSED CONDITIONS HYDROLOGIC ANALYSIS**

- PROPOSED CONDITIONS DRAINAGE MAP
- PROPOSED CONDITIONS HYDROCAD CALCULATIONS



**BOHLER //**

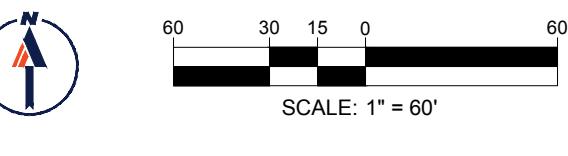
352 TURNPIKE ROAD, 3rd FLOOR  
SOUTHBOROUGH, MA 01772  
Phone: (508) 480-9900

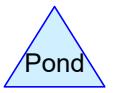
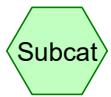
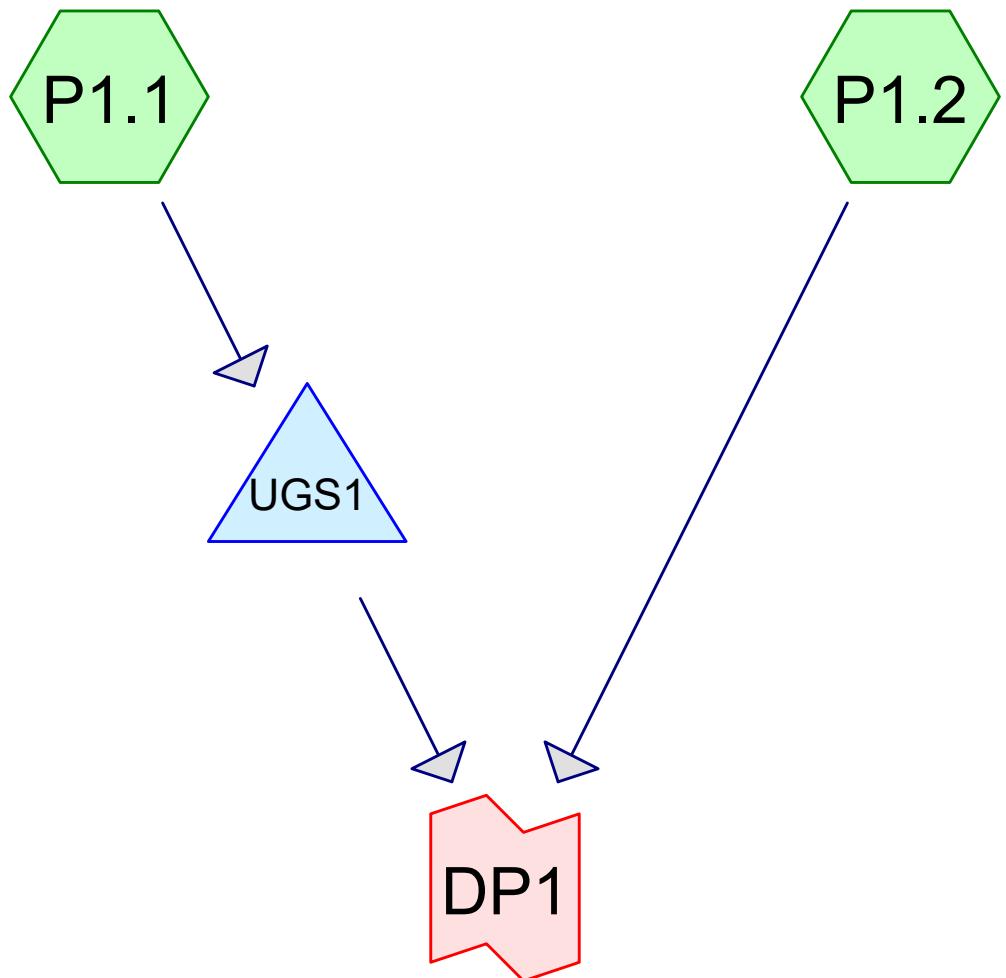
[www.BohlerEngineering.com](http://www.BohlerEngineering.com)

# POST-DEVELOPMENT WATERSHED MAP

ROUTE 85  
REALTY CORP  
BELLINGHAM, MA 02019

01/23/2025 | SB | MAA240489.00 | REV 0a





**Routing Diagram for MAA240489 - Proposed**  
Prepared by Bohler, Printed 2/5/2025  
HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

**MAA240489 - Proposed**

Prepared by Bohler

HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

Printed 2/5/2025

Page 2

**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
0.211	39	>75% Grass cover, Good, HSG A (P1.1, P1.2)
0.213	74	>75% Grass cover, Good, HSG C (P1.1, P1.2)
0.134	98	Unconnected pavement, HSG A (P1.1, P1.2)
0.399	98	Unconnected pavement, HSG C (P1.1, P1.2)
0.080	98	Unconnected roofs, HSG C (P1.1)
<b>1.037</b>	<b>81</b>	<b>TOTAL AREA</b>

**MAA240489 - Proposed**

Prepared by Bohler

HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

Printed 2/5/2025

Page 3

**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.345	HSG A	P1.1, P1.2
0.000	HSG B	
0.692	HSG C	P1.1, P1.2
0.000	HSG D	
0.000	Other	
<b>1.037</b>		<b>TOTAL AREA</b>

**MAA240489 - Proposed**

Prepared by Bohler

HydroCAD® 10.20-5c s/n 03478 © 2023 HydroCAD Software Solutions LLC

Printed 2/5/2025

Page 4

**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.211	0.000	0.213	0.000	0.000	0.424	>75% Grass cover, Good	P1.1, P1.2
0.134	0.000	0.399	0.000	0.000	0.533	Unconnected pavement	P1.1, P1.2
0.000	0.000	0.080	0.000	0.000	0.080	Unconnected roofs	P1.1
<b>0.345</b>	<b>0.000</b>	<b>0.692</b>	<b>0.000</b>	<b>0.000</b>	<b>1.037</b>	<b>TOTAL AREA</b>	

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**SubcatchmentP1.1:** Runoff Area=35,781 sf 67.73% Impervious Runoff Depth=0.17"  
Tc=6.0 min CN=85 Runoff=0.12 cfs 0.012 af

**SubcatchmentP1.2:** Runoff Area=9,400 sf 26.54% Impervious Runoff Depth=0.00"  
Tc=6.0 min UI Adjusted CN=62 Runoff=0.00 cfs 0.000 af

**Pond UGS1:** Peak Elev=257.39' Storage=0.003 af Inflow=0.12 cfs 0.012 af  
Discarded=0.02 cfs 0.012 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.012 af

**Link DP1: Right of Way** Inflow=0.00 cfs 0.000 af  
Primary=0.00 cfs 0.000 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.012 af Average Runoff Depth = 0.14"**  
**40.84% Pervious = 0.424 ac 59.16% Impervious = 0.614 ac**

**Summary for Subcatchment P1.1:**

Runoff = 0.12 cfs @ 12.12 hrs, Volume= 0.012 af, Depth= 0.17"  
 Routed to Pond UGS1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 1-inch Rainfall=1.00"

Area (sf)	CN	Description	
3,607	98	Unconnected pavement, HSG A	
5,707	39	>75% Grass cover, Good, HSG A	
3,500	98	Unconnected roofs, HSG C	
17,126	98	Unconnected pavement, HSG C	
5,841	74	>75% Grass cover, Good, HSG C	
35,781	85	Weighted Average	
11,548		32.27% Pervious Area	
24,233		67.73% Impervious Area	
24,233		100.00% Unconnected	
Tc (min)	Length (feet)	Slope (ft/ft)	
Velocity (ft/sec)	Capacity (cfs)	Description	
6.0			<b>Direct Entry, 6-minute minimum</b>

**Summary for Subcatchment P1.2:**

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 1-inch Rainfall=1.00"

Area (sf)	CN	Adj	Description
2,234	98		Unconnected pavement, HSG A
3,476	39		>75% Grass cover, Good, HSG A
261	98		Unconnected pavement, HSG C
3,429	74		>75% Grass cover, Good, HSG C
9,400	67	62	Weighted Average, UI Adjusted
6,905			73.46% Pervious Area
2,495			26.54% Impervious Area
2,495			100.00% Unconnected
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)
Capacity (cfs)	Description		
6.0			<b>Direct Entry, 6-minute minimum</b>

### Summary for Pond UGS1:

Inflow Area = 0.821 ac, 67.73% Impervious, Inflow Depth = 0.17" for 1-inch event  
 Inflow = 0.12 cfs @ 12.12 hrs, Volume= 0.012 af  
 Outflow = 0.02 cfs @ 12.25 hrs, Volume= 0.012 af, Atten= 87%, Lag= 8.1 min  
 Discarded = 0.02 cfs @ 12.25 hrs, Volume= 0.012 af  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routed to Link DP1 : Right of Way

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 257.39' @ 13.85 hrs Surf.Area= 0.060 ac Storage= 0.003 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 87.9 min ( 988.2 - 900.2 )

Volume	Invert	Avail.Storage	Storage Description
#1A	257.25'	0.103 af	<b>73.92'W x 35.64'L x 6.75'H Field A</b> 0.408 af Overall - 0.151 af Embedded = 0.257 af x 40.0% Voids
#2A	258.00'	0.151 af	<b>ADS_StormTech MC-4500 b +Capx 56 Inside #1</b> Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.02'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 56 Chambers in 8 Rows Cap Storage= 39.5 cf x 2 x 8 rows = 632.0 cf
0.254 af Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	257.25'	<b>0.270 in/hr Exfiltration over Surface area</b>
#2	Primary	259.90'	<b>12.0" Round Culvert</b> L= 63.2' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 259.90' / 259.31' S= 0.0093 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Discarded OutFlow** Max=0.02 cfs @ 12.25 hrs HW=257.32' (Free Discharge)  
 ↗1=Exfiltration (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=257.25' TW=0.00' (Dynamic Tailwater)  
 ↗2=Culvert ( Controls 0.00 cfs)

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 59.16% Impervious, Inflow Depth = 0.00" for 1-inch event  
 Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**SubcatchmentP1.1:** Runoff Area=35,781 sf 67.73% Impervious Runoff Depth=2.62"  
Tc=6.0 min CN=85 Runoff=2.46 cfs 0.179 af

**SubcatchmentP1.2:** Runoff Area=9,400 sf 26.54% Impervious Runoff Depth=0.96"  
Tc=6.0 min UI Adjusted CN=62 Runoff=0.21 cfs 0.017 af

**Pond UGS1:** Peak Elev=260.08' Storage=0.119 af Inflow=2.46 cfs 0.179 af  
Discarded=0.02 cfs 0.038 af Primary=0.12 cfs 0.044 af Outflow=0.13 cfs 0.083 af

**Link DP1: Right of Way** Inflow=0.21 cfs 0.061 af  
Primary=0.21 cfs 0.061 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.197 af Average Runoff Depth = 2.27"**  
**40.84% Pervious = 0.424 ac 59.16% Impervious = 0.614 ac**

### Summary for Subcatchment P1.1:

Runoff = 2.46 cfs @ 12.09 hrs, Volume= 0.179 af, Depth= 2.62"  
 Routed to Pond UGS1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 2-Year Rainfall=4.18"

Area (sf)	CN	Description		
3,607	98	Unconnected pavement, HSG A		
5,707	39	>75% Grass cover, Good, HSG A		
3,500	98	Unconnected roofs, HSG C		
17,126	98	Unconnected pavement, HSG C		
5,841	74	>75% Grass cover, Good, HSG C		
35,781	85	Weighted Average		
11,548		32.27% Pervious Area		
24,233		67.73% Impervious Area		
24,233		100.00% Unconnected		
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
6.0				<b>Direct Entry, 6-minute minimum</b>

### Summary for Subcatchment P1.2:

Runoff = 0.21 cfs @ 12.11 hrs, Volume= 0.017 af, Depth= 0.96"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 2-Year Rainfall=4.18"

Area (sf)	CN	Adj	Description	
2,234	98		Unconnected pavement, HSG A	
3,476	39		>75% Grass cover, Good, HSG A	
261	98		Unconnected pavement, HSG C	
3,429	74		>75% Grass cover, Good, HSG C	
9,400	67	62	Weighted Average, UI Adjusted	
6,905			73.46% Pervious Area	
2,495			26.54% Impervious Area	
2,495			100.00% Unconnected	
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
6.0				<b>Direct Entry, 6-minute minimum</b>

### Summary for Pond UGS1:

Inflow Area = 0.821 ac, 67.73% Impervious, Inflow Depth = 2.62" for 2-Year event  
 Inflow = 2.46 cfs @ 12.09 hrs, Volume= 0.179 af  
 Outflow = 0.13 cfs @ 14.54 hrs, Volume= 0.083 af, Atten= 95%, Lag= 147.0 min  
 Discarded = 0.02 cfs @ 9.75 hrs, Volume= 0.038 af  
 Primary = 0.12 cfs @ 14.54 hrs, Volume= 0.044 af  
 Routed to Link DP1 : Right of Way

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 260.08' @ 14.54 hrs Surf.Area= 0.060 ac Storage= 0.119 af

Plug-Flow detention time= 459.1 min calculated for 0.082 af (46% of inflow)  
 Center-of-Mass det. time= 343.7 min ( 1,158.4 - 814.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	257.25'	0.103 af	<b>73.92'W x 35.64'L x 6.75'H Field A</b> 0.408 af Overall - 0.151 af Embedded = 0.257 af x 40.0% Voids
#2A	258.00'	0.151 af	<b>ADS_StormTech MC-4500 b +Capx 56 Inside #1</b> Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.02'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 56 Chambers in 8 Rows Cap Storage= 39.5 cf x 2 x 8 rows = 632.0 cf
0.254 af Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	257.25'	<b>0.270 in/hr Exfiltration over Surface area</b>
#2	Primary	259.90'	<b>12.0" Round Culvert</b> L= 63.2' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 259.90' / 259.31' S= 0.0093 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Discarded OutFlow** Max=0.02 cfs @ 9.75 hrs HW=257.32' (Free Discharge)  
 ↗ 1=Exfiltration (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=0.12 cfs @ 14.54 hrs HW=260.08' TW=0.00' (Dynamic Tailwater)  
 ↗ 2=Culvert (Inlet Controls 0.12 cfs @ 1.16 fps)

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 59.16% Impervious, Inflow Depth = 0.71" for 2-Year event  
 Inflow = 0.21 cfs @ 12.11 hrs, Volume= 0.061 af  
 Primary = 0.21 cfs @ 12.11 hrs, Volume= 0.061 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**SubcatchmentP1.1:** Runoff Area=35,781 sf 67.73% Impervious Runoff Depth=4.82"  
Tc=6.0 min CN=85 Runoff=4.43 cfs 0.330 af

**SubcatchmentP1.2:** Runoff Area=9,400 sf 26.54% Impervious Runoff Depth=2.48"  
Tc=6.0 min UI Adjusted CN=62 Runoff=0.60 cfs 0.045 af

**Pond UGS1:** Peak Elev=260.72' Storage=0.148 af Inflow=4.43 cfs 0.330 af  
Discarded=0.02 cfs 0.041 af Primary=1.68 cfs 0.192 af Outflow=1.70 cfs 0.233 af

**Link DP1: Right of Way** Inflow=1.96 cfs 0.236 af  
Primary=1.96 cfs 0.236 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.375 af Average Runoff Depth = 4.33"**  
**40.84% Pervious = 0.424 ac 59.16% Impervious = 0.614 ac**

**Summary for Subcatchment P1.1:**

Runoff = 4.43 cfs @ 12.09 hrs, Volume= 0.330 af, Depth= 4.82"  
 Routed to Pond UGS1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10-Year Rainfall=6.55"

Area (sf)	CN	Description	
3,607	98	Unconnected pavement, HSG A	
5,707	39	>75% Grass cover, Good, HSG A	
3,500	98	Unconnected roofs, HSG C	
17,126	98	Unconnected pavement, HSG C	
5,841	74	>75% Grass cover, Good, HSG C	
35,781	85	Weighted Average	
11,548		32.27% Pervious Area	
24,233		67.73% Impervious Area	
24,233		100.00% Unconnected	
Tc (min)	Length (feet)	Slope (ft/ft)	
		Velocity (ft/sec)	
		Capacity (cfs)	
6.0			<b>Direct Entry, 6-minute minimum</b>

**Summary for Subcatchment P1.2:**

Runoff = 0.60 cfs @ 12.10 hrs, Volume= 0.045 af, Depth= 2.48"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10-Year Rainfall=6.55"

Area (sf)	CN	Adj	Description
2,234	98		Unconnected pavement, HSG A
3,476	39		>75% Grass cover, Good, HSG A
261	98		Unconnected pavement, HSG C
3,429	74		>75% Grass cover, Good, HSG C
9,400	67	62	Weighted Average, UI Adjusted
6,905			73.46% Pervious Area
2,495			26.54% Impervious Area
2,495			100.00% Unconnected
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)
			Capacity (cfs)
6.0			<b>Direct Entry, 6-minute minimum</b>

### Summary for Pond UGS1:

Inflow Area = 0.821 ac, 67.73% Impervious, Inflow Depth = 4.82" for 10-Year event  
 Inflow = 4.43 cfs @ 12.09 hrs, Volume= 0.330 af  
 Outflow = 1.70 cfs @ 12.34 hrs, Volume= 0.233 af, Atten= 62%, Lag= 15.1 min  
 Discarded = 0.02 cfs @ 8.05 hrs, Volume= 0.041 af  
 Primary = 1.68 cfs @ 12.34 hrs, Volume= 0.192 af  
 Routed to Link DP1 : Right of Way

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 260.72' @ 12.34 hrs Surf.Area= 0.060 ac Storage= 0.148 af

Plug-Flow detention time= 238.7 min calculated for 0.232 af (70% of inflow)  
 Center-of-Mass det. time= 148.4 min ( 945.8 - 797.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	257.25'	0.103 af	<b>73.92'W x 35.64'L x 6.75'H Field A</b> 0.408 af Overall - 0.151 af Embedded = 0.257 af x 40.0% Voids
#2A	258.00'	0.151 af	<b>ADS_StormTech MC-4500 b +Capx 56 Inside #1</b> Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.02'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 56 Chambers in 8 Rows Cap Storage= 39.5 cf x 2 x 8 rows = 632.0 cf
0.254 af Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	257.25'	<b>0.270 in/hr Exfiltration over Surface area</b>
#2	Primary	259.90'	<b>12.0" Round Culvert</b> L= 63.2' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 259.90' / 259.31' S= 0.0093 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Discarded OutFlow** Max=0.02 cfs @ 8.05 hrs HW=257.32' (Free Discharge)  
 ↑ 1=Exfiltration (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=1.67 cfs @ 12.34 hrs HW=260.72' TW=0.00' (Dynamic Tailwater)  
 ↑ 2=Culvert (Inlet Controls 1.67 cfs @ 2.43 fps)

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 59.16% Impervious, Inflow Depth = 2.73" for 10-Year event  
 Inflow = 1.96 cfs @ 12.32 hrs, Volume= 0.236 af  
 Primary = 1.96 cfs @ 12.32 hrs, Volume= 0.236 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**SubcatchmentP1.1:** Runoff Area=35,781 sf 67.73% Impervious Runoff Depth=6.62"  
Tc=6.0 min CN=85 Runoff=5.99 cfs 0.453 af

**SubcatchmentP1.2:** Runoff Area=9,400 sf 26.54% Impervious Runoff Depth=3.88"  
Tc=6.0 min UI Adjusted CN=62 Runoff=0.96 cfs 0.070 af

**Pond UGS1:** Peak Elev=261.43' Storage=0.178 af Inflow=5.99 cfs 0.453 af  
Discarded=0.02 cfs 0.043 af Primary=3.03 cfs 0.313 af Outflow=3.05 cfs 0.355 af

**Link DP1: Right of Way** Inflow=3.65 cfs 0.383 af  
Primary=3.65 cfs 0.383 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.523 af Average Runoff Depth = 6.05"**  
**40.84% Pervious = 0.424 ac 59.16% Impervious = 0.614 ac**

**Summary for Subcatchment P1.1:**

Runoff = 5.99 cfs @ 12.09 hrs, Volume= 0.453 af, Depth= 6.62"  
 Routed to Pond UGS1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25-Year Rainfall=8.42"

Area (sf)	CN	Description		
3,607	98	Unconnected pavement, HSG A		
5,707	39	>75% Grass cover, Good, HSG A		
3,500	98	Unconnected roofs, HSG C		
17,126	98	Unconnected pavement, HSG C		
5,841	74	>75% Grass cover, Good, HSG C		
35,781	85	Weighted Average		
11,548		32.27% Pervious Area		
24,233		67.73% Impervious Area		
24,233		100.00% Unconnected		
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
6.0				<b>Direct Entry, 6-minute minimum</b>

**Summary for Subcatchment P1.2:**

Runoff = 0.96 cfs @ 12.10 hrs, Volume= 0.070 af, Depth= 3.88"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25-Year Rainfall=8.42"

Area (sf)	CN	Adj	Description	
2,234	98		Unconnected pavement, HSG A	
3,476	39		>75% Grass cover, Good, HSG A	
261	98		Unconnected pavement, HSG C	
3,429	74		>75% Grass cover, Good, HSG C	
9,400	67	62	Weighted Average, UI Adjusted	
6,905			73.46% Pervious Area	
2,495			26.54% Impervious Area	
2,495			100.00% Unconnected	
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
6.0				<b>Direct Entry, 6-minute minimum</b>

### Summary for Pond UGS1:

Inflow Area = 0.821 ac, 67.73% Impervious, Inflow Depth = 6.62" for 25-Year event  
 Inflow = 5.99 cfs @ 12.09 hrs, Volume= 0.453 af  
 Outflow = 3.05 cfs @ 12.24 hrs, Volume= 0.355 af, Atten= 49%, Lag= 9.1 min  
 Discarded = 0.02 cfs @ 6.95 hrs, Volume= 0.043 af  
 Primary = 3.03 cfs @ 12.24 hrs, Volume= 0.313 af

Routed to Link DP1 : Right of Way

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 261.43' @ 12.24 hrs Surf.Area= 0.060 ac Storage= 0.178 af

Plug-Flow detention time= 189.6 min calculated for 0.355 af (78% of inflow)  
 Center-of-Mass det. time= 111.0 min ( 899.8 - 788.8 )

Volume	Invert	Avail.Storage	Storage Description
#1A	257.25'	0.103 af	<b>73.92'W x 35.64'L x 6.75'H Field A</b> 0.408 af Overall - 0.151 af Embedded = 0.257 af x 40.0% Voids
#2A	258.00'	0.151 af	<b>ADS_StormTech MC-4500 b +Capx 56 Inside #1</b> Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.02'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 56 Chambers in 8 Rows Cap Storage= 39.5 cf x 2 x 8 rows = 632.0 cf
0.254 af Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	257.25'	<b>0.270 in/hr Exfiltration over Surface area</b>
#2	Primary	259.90'	<b>12.0" Round Culvert</b> L= 63.2' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 259.90' / 259.31' S= 0.0093 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Discarded OutFlow** Max=0.02 cfs @ 6.95 hrs HW=257.32' (Free Discharge)  
 ↑ 1=Exfiltration (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=3.03 cfs @ 12.24 hrs HW=261.43' TW=0.00' (Dynamic Tailwater)  
 ↑ 2=Culvert (Inlet Controls 3.03 cfs @ 3.86 fps)

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 59.16% Impervious, Inflow Depth = 4.43" for 25-Year event  
 Inflow = 3.65 cfs @ 12.17 hrs, Volume= 0.383 af  
 Primary = 3.65 cfs @ 12.17 hrs, Volume= 0.383 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Time span=0.00-36.00 hrs, dt=0.05 hrs, 721 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**SubcatchmentP1.1:** Runoff Area=35,781 sf 67.73% Impervious Runoff Depth=9.62"  
Tc=6.0 min CN=85 Runoff=8.52 cfs 0.659 af

**SubcatchmentP1.2:** Runoff Area=9,400 sf 26.54% Impervious Runoff Depth=6.44"  
Tc=6.0 min UI Adjusted CN=62 Runoff=1.60 cfs 0.116 af

**Pond UGS1:** Peak Elev=262.72' Storage=0.223 af Inflow=8.52 cfs 0.659 af  
Discarded=0.02 cfs 0.044 af Primary=4.55 cfs 0.517 af Outflow=4.56 cfs 0.561 af

**Link DP1: Right of Way** Inflow=5.61 cfs 0.632 af  
Primary=5.61 cfs 0.632 af

**Total Runoff Area = 1.037 ac Runoff Volume = 0.774 af Average Runoff Depth = 8.96"**  
**40.84% Pervious = 0.424 ac 59.16% Impervious = 0.614 ac**

**Summary for Subcatchment P1.1:**

Runoff = 8.52 cfs @ 12.09 hrs, Volume= 0.659 af, Depth= 9.62"  
 Routed to Pond UGS1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-Year Rainfall=11.50"

Area (sf)	CN	Description	
3,607	98	Unconnected pavement, HSG A	
5,707	39	>75% Grass cover, Good, HSG A	
3,500	98	Unconnected roofs, HSG C	
17,126	98	Unconnected pavement, HSG C	
5,841	74	>75% Grass cover, Good, HSG C	
35,781	85	Weighted Average	
11,548		32.27% Pervious Area	
24,233		67.73% Impervious Area	
24,233		100.00% Unconnected	
Tc (min)	Length (feet)	Slope (ft/ft)	
Velocity (ft/sec)	Capacity (cfs)	Description	
6.0			<b>Direct Entry, 6-minute minimum</b>

**Summary for Subcatchment P1.2:**

Runoff = 1.60 cfs @ 12.09 hrs, Volume= 0.116 af, Depth= 6.44"  
 Routed to Link DP1 : Right of Way

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100-Year Rainfall=11.50"

Area (sf)	CN	Adj	Description
2,234	98		Unconnected pavement, HSG A
3,476	39		>75% Grass cover, Good, HSG A
261	98		Unconnected pavement, HSG C
3,429	74		>75% Grass cover, Good, HSG C
9,400	67	62	Weighted Average, UI Adjusted
6,905			73.46% Pervious Area
2,495			26.54% Impervious Area
2,495			100.00% Unconnected
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)
Capacity (cfs)	Description		
6.0			<b>Direct Entry, 6-minute minimum</b>

### Summary for Pond UGS1:

Inflow Area = 0.821 ac, 67.73% Impervious, Inflow Depth = 9.62" for 100-Year event  
 Inflow = 8.52 cfs @ 12.09 hrs, Volume= 0.659 af  
 Outflow = 4.56 cfs @ 12.22 hrs, Volume= 0.561 af, Atten= 46%, Lag= 8.1 min  
 Discarded = 0.02 cfs @ 5.50 hrs, Volume= 0.044 af  
 Primary = 4.55 cfs @ 12.22 hrs, Volume= 0.517 af

Routed to Link DP1 : Right of Way

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs  
 Peak Elev= 262.72' @ 12.22 hrs Surf.Area= 0.060 ac Storage= 0.223 af

Plug-Flow detention time= 149.3 min calculated for 0.560 af (85% of inflow)  
 Center-of-Mass det. time= 87.3 min ( 866.1 - 778.9 )

Volume	Invert	Avail.Storage	Storage Description
#1A	257.25'	0.103 af	<b>73.92'W x 35.64'L x 6.75'H Field A</b> 0.408 af Overall - 0.151 af Embedded = 0.257 af x 40.0% Voids
#2A	258.00'	0.151 af	<b>ADS_StormTech MC-4500 b +Capx 56 Inside #1</b> Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.02'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 56 Chambers in 8 Rows Cap Storage= 39.5 cf x 2 x 8 rows = 632.0 cf
0.254 af Total Available Storage			

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	257.25'	<b>0.270 in/hr Exfiltration over Surface area</b>
#2	Primary	259.90'	<b>12.0" Round Culvert</b> L= 63.2' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 259.90' / 259.31' S= 0.0093 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

**Discarded OutFlow** Max=0.02 cfs @ 5.50 hrs HW=257.32' (Free Discharge)  
 ↑ 1=Exfiltration (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=4.52 cfs @ 12.22 hrs HW=262.69' TW=0.00' (Dynamic Tailwater)  
 ↑ 2=Culvert (Inlet Controls 4.52 cfs @ 5.76 fps)

### Summary for Link DP1: Right of Way

Inflow Area = 1.037 ac, 59.16% Impervious, Inflow Depth = 7.32" for 100-Year event  
 Inflow = 5.61 cfs @ 12.16 hrs, Volume= 0.632 af  
 Primary = 5.61 cfs @ 12.16 hrs, Volume= 0.632 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

## **APPENDIX F: STORMWATER CALCULATIONS**

- MA STANDARD #3 – RECHARGE AND DRAWDOWN TIME
- MA STANDARD #4 – WATER QUALITY AND TSS REMOVAL
- NOAA RAINFALL DATA

**Proposed Bank with Drive Thru  
 182 & 186 Hartford Avenue  
 Bellingham, MA  
 Bohler Job Number: MAA240489.00  
 February 5, 2025**

**MA DEP Standard 3: Recharge Volume Calculations**

<b>Required Recharge Volume - A Soils (0.60 in.)</b>	
Existing Site Impervious Area (ac)	0.099
Proposed Site Impervious Area (ac)	0.134
Proposed Increase in Site Impervious Area (ac)	0.035
<b>Recharge Volume Required (cf)</b>	<b>77</b>

<b>Required Recharge Volume - C Soils (0.25 in.)</b>	
Existing Site Impervious Area (ac)	0.132
Proposed Site Impervious Area (ac)	0.479
Proposed Increase in Site Impervious Area (ac)	0.348
<b>Recharge Volume Required (cf)</b>	<b>316</b>

<b>Total Recharge Volume Required (cf)</b>	<b>393</b>
--	------------

<b>Recharge Volume Adjustment Factor</b>	
Impervious Area Directed to Infiltration BMP (ac)	<b>0.556</b>
%Impervious Directed to Infiltration BMP	91%
Adjustment Factor	1.10
<b>Adjusted Total Recharge Volume Required (cf)</b>	<b>433</b>

<b>Provided Recharge Volume*</b>	
UGS1	4,225
<b>Total Recharge Volume Provided (cf)</b>	<b>4,225</b>

**Provided greater than or Equal to Required**

\*Volume provided below lowest outlet in cubic feet (cf)

Prepared By:

**BOHLER //**

352 Turnpike Road  
 Southborough, MA 01772  
 (508) 480-9900

2/5/2025

**Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA  
Bohler Job Number: MAA240489.00  
February 5, 2025**

**MA DEP Standard 3: Drawdown Time Calculations**

---

<b>Drawdown Time - UGS1</b>	
Volume below outlet pipe (Rv) (cf)	4,225
Soil Type	Silt Loam - C
Infiltration rate (K)*	0.27
Bottom Area (sf)	2,635
<b>Drawdown time (Hours)*</b>	<b>71.3</b>

\*Infiltration Rates taken from Rawls Table

\*\*Drawdown time = Rv / (K) x (bottom area)

Prepared By:

**BOHLER //**

352 Turnpike Road  
Southborough, MA 01772  
(508) 480-9900

2/5/2025

**Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA  
Bohler Job Number: MAA240489.00  
February 5, 2025**

**MA DEP Standard 4: Water Quality Volume Calculations**

---

<b>Water Quality Volume Required</b>	
Water Quality Volume runoff (in.)*	1.0
Total Post Development Impervious Area (sf)	26,727
<b>Required Water Quality Volume (cf)</b>	<b>2,227</b>

\*Water Quality volume runoff is equal to 1.0 inches of runoff times the total impervious area of the post development project site.

<b>Water Quality Volume Provided*</b>	
UGS1	4,225
<b>Total Provided Water Quality Volume (cf)</b>	<b>4,225</b>
	<b>Required Water Quality Volume Provided</b>

\*Volume provided below lowest outlet pipe in cubic feet (cf)

Prepared By:

**BOHLER //**

352 Turnpike Road  
Southborough, MA 01772  
(508) 480-9900

2/5/2025

**Proposed Bank with Drive Thru**  
**182 & 186 Hartford Avenue**  
**Bellingham, MA**  
**Bohler Job Number: MAA240489.00**  
**February 5, 2025**  
**1" Water Quality Volume to Flow Rate Calculation Sheet**

**Compute Water Quality Flow with the following Equation**

$$WQF = (qu)(A)(WQV)$$

Site Plan Callout	qu (from 1" - qu Table)	Impervious Area (SF)	Ai (sq/mi)	WQV (inches)	WQF (cfs)
UGS1 Isolator Row	774	24232	0.000869	1	0.67

Water Quality Flow Rate = WQF

Water Quality Volume = WQV\*

Unit peak discharge (csm/in) = qu\*\*

Impervious Area in watershed (square miles) = Ai

\*WQV is expressed in watershed inches (you must use 1.0-inches in all cases with this method and not 0.5-inches)

\*\* calculate the qu based on the time of concentration (see 1" - qu Table)

**Infiltration Basin #1 Isolator row sizing**

Maximum treatment flow rate - MC-4500 Chamber\* 0.277 cfs

Number of chambers in Isolator Row 7

WQF provided by isolator row = 1.94 cfs

\*Per NJCAT Technology Verifaction, Isolator Row Plus, StormTech, LLC, July 2020

Prepared By:

**BOHLER //**

352 Turnpike Road  
Southborough, MA 01772  
(508) 480-9900

**Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA  
Bohler Job Number: MAA240489.00  
February 5, 2025**

**MA DEP Standard 4: TSS Removal Calculation Worksheet**

---

BMP Treatment Train: CB to UGS1

A BMP	B TSS Removal Rate	C Starting TSS Load*	D Amount Removed (B*C)	E Remaining Load (C-D)
Deep-Sump, Hooded Catch Basins	0.25	1.00	0.25	0.75
Underground Infiltration System with Isolator Row	0.80	0.75	0.60	0.15
<b>Total TSS Removal =</b>				<b>85%</b>

\*Equals remaining load from previous BMP (E) which enters BMP

Prepared By:

**BOHLER //**

352 Turnpike Road  
Southborough, MA 01772  
(508) 480-9900

2/5/2025

## Stage-Area-Storage for Pond UGS1:

Elevation (feet)	Surface (acres)	Storage (acre-feet)	Elevation (feet)	Surface (acres)	Storage (acre-feet)
257.25	<b>0.054</b>	0.000	262.45	0.054	0.189
257.35	0.054	0.002	262.55	0.054	0.192
257.45	0.054	0.004	262.65	0.054	0.195
257.55	0.054	0.006	262.75	0.054	0.197
257.65	0.054	0.009	262.85	0.054	0.199
257.75	0.054	0.011	262.95	0.054	0.201
257.85	0.054	0.013	263.05	0.054	0.204
257.95	0.054	0.015	263.15	0.054	0.206
258.05	0.054	0.018	263.25	0.054	0.208
258.15	0.054	0.023	263.35	0.054	0.210
258.25	0.054	0.027	263.45	0.054	0.212
258.35	0.054	0.031	263.55	0.054	0.214
258.45	0.054	0.036	263.65	0.054	0.216
258.55	0.054	0.040	263.75	0.054	0.219
258.65	0.054	0.044	263.85	0.054	0.221
258.75	0.054	0.049	263.95	0.054	<b>0.223</b>
258.85	0.054	0.053			
258.95	0.054	0.057			
259.05	0.054	0.062			
259.15	0.054	0.066			
259.25	0.054	0.070			
259.35	0.054	0.074			
259.45	0.054	0.079			
259.55	0.054	0.083			
259.65	0.054	0.087			
259.75	0.054	0.091			
259.85	0.054	0.095			
259.95	0.054	0.099			
260.05	0.054	0.104			
260.15	0.054	0.108			
260.25	0.054	0.112			
260.35	0.054	0.116			
260.45	0.054	0.120			
260.55	0.054	0.124			
260.65	0.054	0.127			
260.75	0.054	0.131			
260.85	0.054	0.135			
260.95	0.054	0.139			
261.05	0.054	0.143			
261.15	0.054	0.146			
261.25	0.054	0.150			
261.35	0.054	0.154			
261.45	0.054	0.157			
261.55	0.054	0.161			
261.65	0.054	0.164			
261.75	0.054	0.168			
261.85	0.054	0.171			
261.95	0.054	0.174			
262.05	0.054	0.178			
262.15	0.054	0.181			
262.25	0.054	0.184			
262.35	0.054	0.187			

Storage at 259.90 =  
±0.097 ac-ft = ±4,225 cf



**NOAA Atlas 14, Volume 10, Version 3**  
**Location name: Bellingham, Massachusetts, USA\***  
**Latitude: 42.1142°, Longitude: -71.4724°**

**Elevation: 272 ft\*\***

\* source: ESRI Maps

\*\* source: USGS



## POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

### PF tabular

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.331 (0.257-0.421)	0.399 (0.310-0.508)	0.510 (0.394-0.652)	0.601 (0.463-0.773)	0.727 (0.542-0.978)	0.823 (0.601-1.13)	0.922 (0.654-1.31)	1.03 (0.695-1.51)	1.19 (0.770-1.80)	1.31 (0.831-2.03)
10-min	0.469 (0.365-0.597)	0.565 (0.439-0.720)	0.722 (0.559-0.924)	0.852 (0.655-1.10)	1.03 (0.768-1.39)	1.17 (0.851-1.60)	1.31 (0.926-1.86)	1.46 (0.985-2.14)	1.68 (1.09-2.55)	1.86 (1.18-2.88)
15-min	0.552 (0.429-0.702)	0.664 (0.516-0.847)	0.848 (0.656-1.08)	1.00 (0.770-1.29)	1.21 (0.903-1.63)	1.37 (1.00-1.88)	1.54 (1.09-2.19)	1.72 (1.16-2.52)	1.98 (1.28-3.00)	2.19 (1.39-3.39)
30-min	0.752 (0.585-0.957)	0.907 (0.705-1.16)	1.16 (0.898-1.48)	1.37 (1.06-1.76)	1.66 (1.24-2.24)	1.88 (1.37-2.59)	2.11 (1.50-3.01)	2.36 (1.59-3.45)	2.72 (1.76-4.12)	3.01 (1.90-4.65)
60-min	0.952 (0.741-1.21)	1.15 (0.894-1.47)	1.48 (1.14-1.89)	1.74 (1.34-2.24)	2.11 (1.58-2.84)	2.39 (1.75-3.29)	2.68 (1.90-3.82)	3.00 (2.02-4.39)	3.46 (2.24-5.24)	3.82 (2.42-5.91)
2-hr	1.22 (0.952-1.54)	1.48 (1.16-1.88)	1.92 (1.49-2.44)	2.28 (1.76-2.91)	2.78 (2.08-3.72)	3.15 (2.31-4.31)	3.54 (2.53-5.04)	3.99 (2.70-5.80)	4.65 (3.02-7.00)	5.19 (3.30-7.98)
3-hr	1.41 (1.10-1.77)	1.72 (1.35-2.17)	2.23 (1.74-2.82)	2.65 (2.06-3.38)	3.23 (2.44-4.32)	3.67 (2.71-5.01)	4.13 (2.97-5.88)	4.67 (3.16-6.76)	5.47 (3.56-8.20)	6.14 (3.90-9.39)
6-hr	1.81 (1.43-2.26)	2.21 (1.74-2.77)	2.86 (2.25-3.60)	3.40 (2.66-4.30)	4.15 (3.14-5.51)	4.70 (3.49-6.39)	5.30 (3.83-7.50)	6.00 (4.08-8.63)	7.07 (4.62-10.5)	7.97 (5.09-12.1)
12-hr	2.30 (1.82-2.86)	2.80 (2.22-3.48)	3.61 (2.86-4.51)	4.29 (3.37-5.39)	5.22 (3.98-6.89)	5.91 (4.42-7.99)	6.66 (4.85-9.39)	7.56 (5.15-10.8)	8.92 (5.85-13.2)	10.1 (6.46-15.2)
24-hr	2.76 (2.20-3.41)	3.38 (2.70-4.18)	4.40 (3.50-5.46)	5.25 (4.15-6.55)	6.41 (4.92-8.42)	7.27 (5.47-9.79)	8.20 (6.02-11.5)	9.36 (6.40-13.3)	11.2 (7.33-16.4)	12.7 (8.15-19.0)
2-day	3.13 (2.52-3.84)	3.90 (3.13-4.79)	5.15 (4.12-6.35)	6.19 (4.92-7.67)	7.62 (5.88-9.96)	8.66 (6.57-11.6)	9.82 (7.27-13.8)	11.3 (7.74-15.9)	13.6 (8.97-19.9)	15.6 (10.1-23.3)
3-day	3.42 (2.75-4.17)	4.24 (3.42-5.19)	5.59 (4.49-6.86)	6.71 (5.35-8.28)	8.25 (6.39-10.7)	9.38 (7.13-12.5)	10.6 (7.89-14.9)	12.2 (8.39-17.1)	14.7 (9.71-21.4)	16.9 (10.9-25.0)
4-day	3.68 (2.98-4.49)	4.54 (3.66-5.54)	5.94 (4.78-7.27)	7.10 (5.68-8.74)	8.70 (6.75-11.3)	9.87 (7.52-13.1)	11.2 (8.30-15.5)	12.8 (8.82-17.9)	15.4 (10.2-22.2)	17.6 (11.4-26.0)
7-day	4.42 (3.59-5.36)	5.33 (4.33-6.47)	6.82 (5.51-8.31)	8.06 (6.47-9.86)	9.75 (7.59-12.5)	11.0 (8.39-14.5)	12.4 (9.18-17.0)	14.0 (9.71-19.5)	16.6 (11.0-23.9)	18.8 (12.2-27.5)
10-day	5.13 (4.18-6.20)	6.07 (4.94-7.34)	7.61 (6.17-9.23)	8.88 (7.16-10.8)	10.6 (8.29-13.6)	11.9 (9.11-15.6)	13.3 (9.88-18.2)	15.0 (10.4-20.8)	17.5 (11.6-25.1)	19.6 (12.7-28.6)
20-day	7.23 (5.93-8.68)	8.23 (6.74-9.89)	9.86 (8.05-11.9)	11.2 (9.10-13.6)	13.1 (10.2-16.5)	14.5 (11.1-18.7)	16.0 (11.8-21.3)	17.6 (12.3-24.1)	19.8 (13.3-28.2)	21.7 (14.1-31.4)
30-day	8.98 (7.39-10.7)	10.0 (8.23-12.0)	11.7 (9.58-14.0)	13.1 (10.7-15.8)	15.0 (11.8-18.8)	16.5 (12.6-21.1)	18.0 (13.3-23.8)	19.6 (13.7-26.7)	21.6 (14.5-30.6)	23.2 (15.1-33.6)
45-day	11.1 (9.20-13.3)	12.2 (10.1-14.6)	14.0 (11.5-16.7)	15.4 (12.6-18.6)	17.5 (13.7-21.7)	19.0 (14.6-24.1)	20.6 (15.1-26.8)	22.0 (15.5-29.9)	23.8 (16.0-33.5)	25.2 (16.4-36.2)
60-day	12.9 (10.7-15.4)	14.1 (11.6-16.7)	15.9 (13.1-18.9)	17.4 (14.2-20.8)	19.5 (15.3-24.1)	21.1 (16.2-26.6)	22.7 (16.6-29.3)	24.0 (16.9-32.5)	25.7 (17.3-35.9)	26.7 (17.5-38.3)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

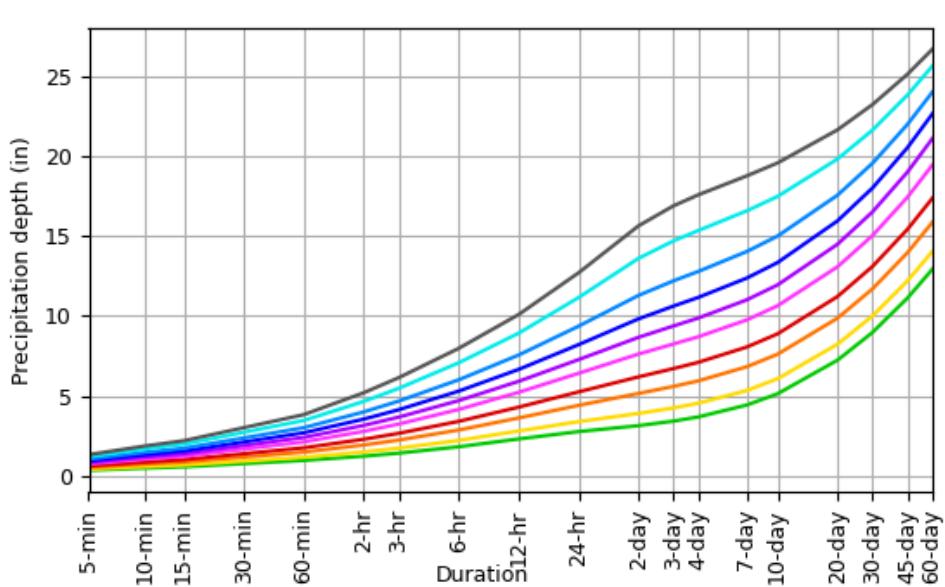
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

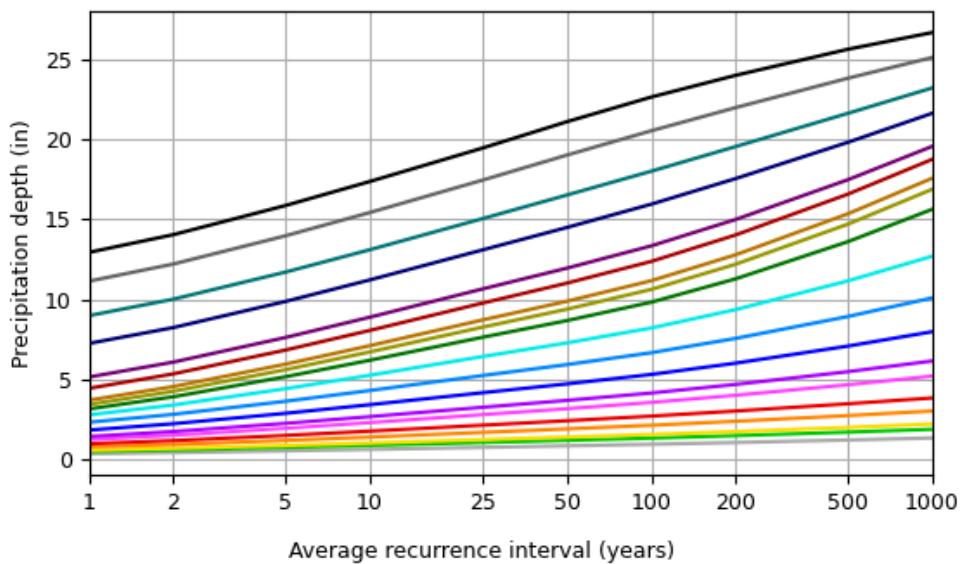
[Back to Top](#)

### PF graphical

PDS-based depth-duration-frequency (DDF) curves  
Latitude: 42.1142°, Longitude: -71.4724°



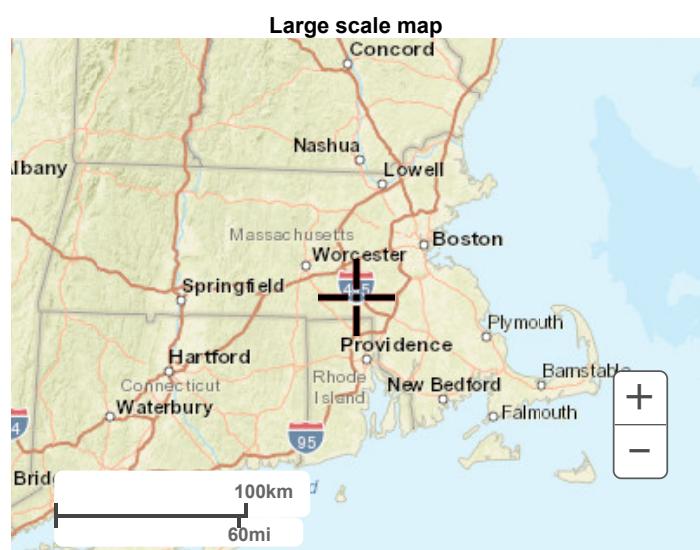
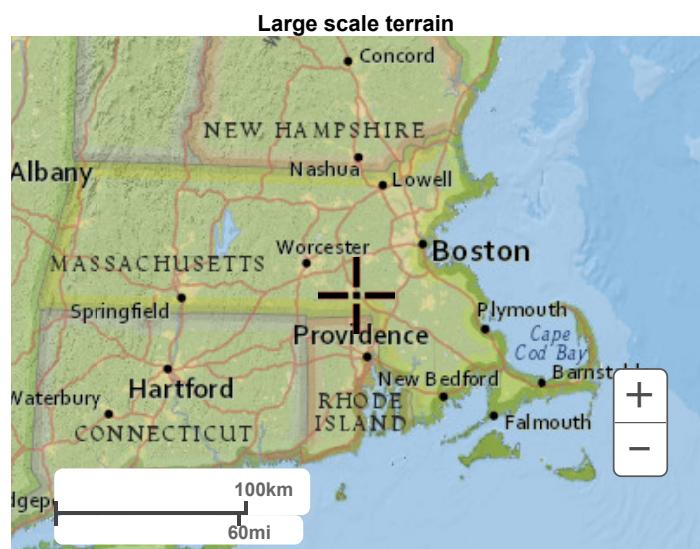
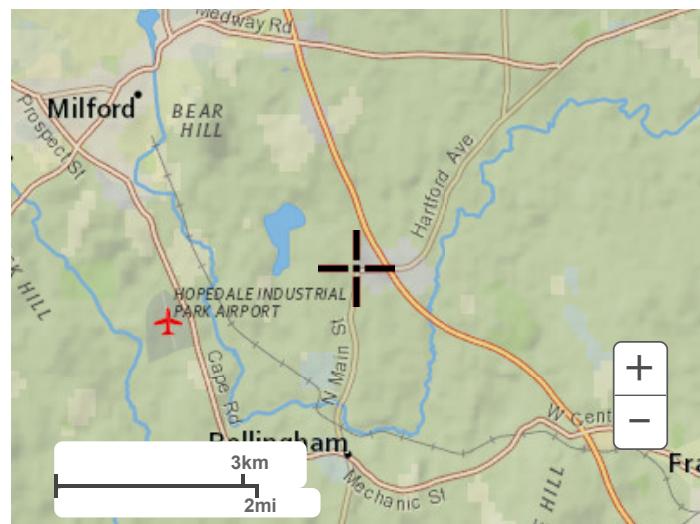
Average recurrence interval (years)
1
2
5
10
25
50
100
200
500
1000



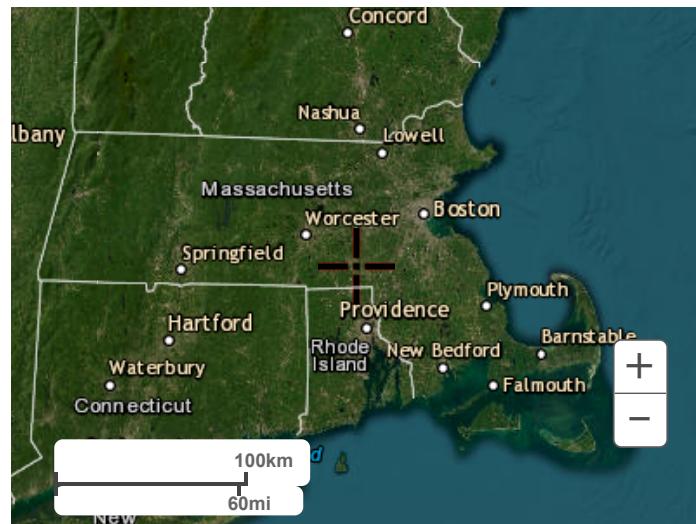
Duration	
5-min	2-day
10-min	3-day
15-min	4-day
30-min	7-day
60-min	10-day
2-hr	10-day
3-hr	20-day
6-hr	30-day
12-hr	45-day
24-hr	60-day

## Maps & aerials

[Small scale terrain](#)



Large scale aerial



[Back to Top](#)

---

[US Department of Commerce](#)  
[National Oceanic and Atmospheric Administration](#)  
[National Weather Service](#)  
[National Water Center](#)  
1325 East West Highway  
Silver Spring, MD 20910  
Questions?: [HDSC.Questions@noaa.gov](mailto:HDSC.Questions@noaa.gov)

[Disclaimer](#)

# **NJCAT TECHNOLOGY VERIFICATION**

**Isolator® Row PLUS**

**StormTech, LLC**

**July 2020**

## Table of Contents

Table of Contents .....	i
List of Figures .....	ii
List of Tables .....	iii
1. Description of Technology .....	1
2. Laboratory Testing .....	2
2.1 Test Setup .....	2
2.2 Test Sediment .....	7
2.3 Sediment Removal Efficiency Testing .....	8
2.4 Sediment Mass Loading Capacity .....	9
3. Supporting Documentation .....	9
4. Testing Results .....	9
4.1 Flow Rate .....	9
4.2 Water Temperature .....	10
4.3 Head .....	10
4.4 Sediment Concentration and Removal Efficiency .....	11
4.5 Sediment Mass Loading .....	18
5. Performance Verification .....	20
6. Design Limitations .....	21
7. Maintenance Plan .....	22
8. Statements .....	23
Specifications .....	29

## **List of Figures**

Figure 1 Schematic of the StormTech Isolator Row PLUS System .....	1
Figure 2 Isolator Row PLUS Detail .....	2
Figure 3 Schematic of the Isolator Row PLUS Test Configuration .....	3
Figure 4 Photograph of Flow Meter .....	4
Figure 5 Photograph of Sediment Delivery Port .....	4
Figure 6 Side View Photograph of Isolator Row PLUS Test Box .....	4
Figure 7 Top View Photograph of Isolator Row PLUS Test Box .....	5
Figure 8 Photograph of Background Sampling Port .....	6
Figure 9 Average Particle Size Distribution of Test Sediment Verified by ECS .....	7
Figure 10 Removal Efficiency vs. Sediment Mass Loading .....	19
Figure 11 Driving Head vs. Sediment Mass Loading .....	20

## List of Tables

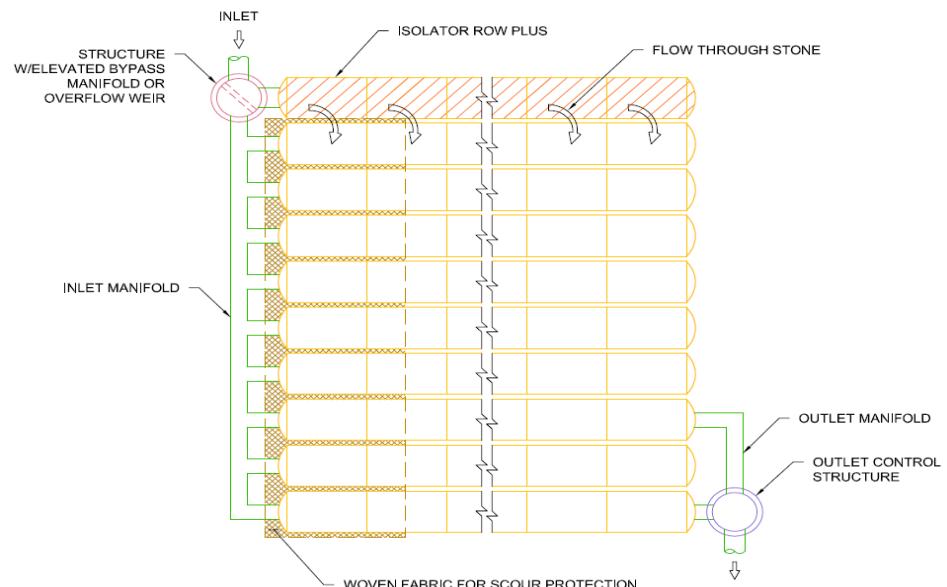
Table 1 Sampling Schedule for the Isolator Row PLUS Tests.....	6
Table 2 Particle Size Distribution of Test Sediment as Analyzed by ECS.....	8
Table 3 Flow Rate and Temperature Summary for All Runs.....	10
Table 4 Sediment Maximum Head (inches) for All Runs.....	11
Table 5 Background TSS Concentrations.....	12
Table 6 Sediment Rate Measurements for Runs 1-10.....	13
Table 7 Sediment Rate Measurements for Runs 11-16.....	14
Table 8 Effluent Sample TSS Concentrations.....	15
Table 9 Drawdown Sample TSS Concentrations.....	16
Table 10 Removal Efficiency Drawdown Losses.....	17
Table 11 Summary of Sediment Concentrations and Removal Efficiency.....	18
Table 12 Sediment Mass Loading Summary.....	19
Table 13 Isolator Row PLUS Model Sizes and New Jersey Treatment Capacities.....	21

## 1. Description of Technology

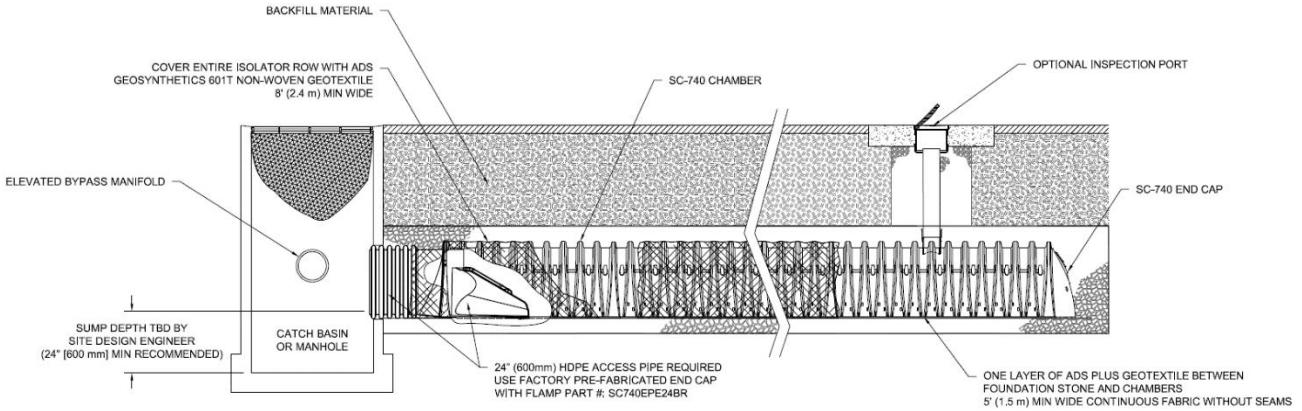
The Isolator® Row PLUS (shown in Figures 1 and 2) is the first row of StormTech chambers that is surrounded with filter fabric and connected to a closely located manhole for easy access. The Isolator Row PLUS provides for settling and filtration of sediment as stormwater rises in the chamber and ultimately passes through the filter fabric. The open-bottom chambers allow stormwater to flow out of the chambers, while sediment is captured in the Isolator Row PLUS.

A single layer of proprietary Advanced Drainage Systems (ADS) PLUS fabric is placed between the angular base stone and the Isolator Row PLUS chamber. The geotextile provides the means for stormwater filtration and provides a durable surface for maintenance operations. A non-woven fabric is placed over the chambers. See link to O&M Manual (pg. 23) for installation pictures.

The Isolator Row PLUS is designed to capture the “first flush” runoff and offers the versatility to be sized on a volume basis or a flow basis. An upstream manhole not only provides access to the Isolator Row PLUS but includes a high/low concept such that stormwater flow rates or volumes that exceed the capacity of the Isolator Row PLUS bypass through a manifold to the other chambers. This is achieved with either an elevated bypass manifold or a high-flow weir. This creates a differential between the Isolator Row PLUS row of chambers and the manifold to the rest of the system, thus allowing for settlement time in the Isolator Row PLUS. After Stormwater flows through the Isolator Row PLUS and into the rest of the StormTech chamber system it is either infiltrated into the soils below or passed at a controlled rate through an outlet manifold and outlet control structure. **Since this technology fits under the infiltration basin BMP in the New Jersey Stormwater BMP Manual, it is not eligible for NJDEP MTD certification.**



**Figure 1 Schematic of the StormTech Isolator Row PLUS System**



**Figure 2 Isolator Row PLUS Detail**

## 2. Laboratory Testing

Beginning in January 2020, two overlapping StormTech SC-740 Isolator Row PLUS commercial size chambers were installed at the BaySaver Laboratory in Mount Airy, Maryland, to evaluate the performance of Isolator Row PLUS on Total Suspended Solid (TSS) removal. Boggs Environmental Consultants (BEC) provided third-party review and oversight of all testing and data collection procedures, in accordance with the *New Jersey Department of Environmental Protection Laboratory Protocol to Assess Total Suspended Solids Removal by a Filtration Manufactured Treatment Device (January 2013)*. All sediment concentration samples were analyzed by Fredericktowne Labs (FTL) using ASTM D3977-97 (2019). All sediment PSD analysis was performed by Environmental Consulting Services (ECS), using the methodology of ASTM D422-63 (2007). Prior to the start of testing, a Quality Assurance Project Plan (QAPP), revision dated January 9, 2020, was submitted to, and approved by the New Jersey Corporation for Advanced Technology (NJCAT).

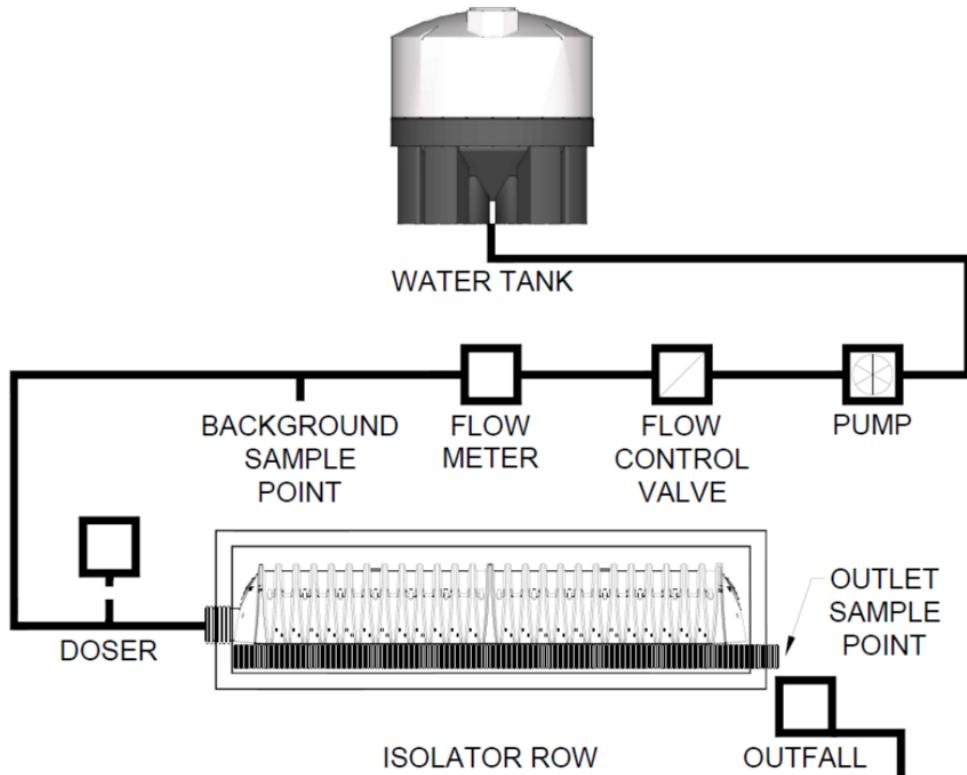
### 2.1 Test Setup

The testing system, shown in **Figure 3**, consisted of a source tank, feed pump, flow control valve, flow meter, background sample port, screw-auger sediment feeder (doser), and an Isolator Row PLUS test system. This verification report only addresses the performance of the Isolator Row PLUS and not the entire StormTech system, since this is the row designed to remove sediment until the system goes into bypass.

#### Testing Procedure

The water source was potable water from the Town of Mount Airy Water & Sewer Department, obtained from an onsite tap, which served as the raw water supply for the testing system. Municipal tap water was used to fill the source tank, and then pumped to the system. Flow rate was controlled to the target of 225 gpm by a flow control valve. An inline flow meter (FloCat MFE electromagnetic flow meter) was used to measure the flow, and a SeaMetrics DL76 data logger (pictured in **Figure 4**) recorded the flow at one-minute intervals. The test sediment was

introduced to the inlet stream via a 12 -inch dosing port teed with a 12-inch influent line (pictured in **Figure 5**) located approximately 4 feet upstream of the system inlet. The dosing rate was controlled by a screw-auger Velodyne Barracuda 1000A volumetric feeder with a 1/2 HP variable speed motor. The dosing rate was set to deliver an amount of sediment that, when mixed with the water from the source tank, would produce influent water with a target test sediment concentration of 200 mg/L.



**Figure 3 Schematic of the Isolator Row PLUS Test Configuration**

The Isolator Row PLUS was installed inside a watertight 16'L x 6'W x 4'H test box (pictured in **Figures 6 and 7**). The Isolator Row PLUS is an arch-shaped stormwater detention/retention sediment collection and filtering device, sealed with end caps, with a 12"-inch inlet pipe welded into the upstream end cap. A ramp apparatus (patent pending) was attached to the inside of the chamber end cap to provide a smooth transition from pipe invert to fabric bottom. It is configured to improve chamber function performance over time by distributing sediment and debris that would otherwise collect at the inlet. It also serves to improve the fluid and solid flow back into the inlet pipe during maintenance and cleaning, and to guide cleaning and inspection equipment back into the inlet pipe when complete.

The chambers were installed on a 10-inch base of washed, angular, crushed stone, (#57, 3/4 inch blue stone) containing an 8-inch perforated underdrain pipe running the length of the test box, penetrating the wall of the downstream end of the test box to the discharge collection point. An ADS non-woven geotextile fabric was placed over the top of the chamber row. The chambers were then backfilled with the washed crushed stone up to the top of the chamber elevation.

Additionally, an opening was cut into the top of one chamber to allow for visual monitoring and head measurement. No bypass or weir was installed upstream of the test box.

The test flow entered the chamber via the influent pipe and flowed across the filter fabric, filling the row. The water then flowed through the filter fabric, driven by hydrostatic head. The treated water exited the test box via the underdrain.



**Figures 4 and 5 Photographs of Flow Meter and Sediment Delivery Port**



**Figure 6 Side View Photograph of Isolator Row PLUS Test Box**



**Figure 7 Top View Photograph of Isolator Row PLUS Test Box**

#### *Test Unit and Scaling Explanation*

The Isolator Row PLUS used in this test was constructed from two (2) overlapping polypropylene open-bottom StormTech SC-740 chambers (one shortened by 5-in. to enable fitting into the test box), two (2) SC-740 end caps, a ramp apparatus and one layer of ADS PLUS geotextile fabric. The chamber floor filtration area (effective filtration treatment area, EFTA) was approximately 54.5 ft<sup>2</sup>. (calculated using an average contact width inside the chamber of 45 in). The target test flow was 225 gpm. The calculated hydraulic loading rate, flow rate/EFTA is 4.13 gpm/ft<sup>2</sup> and the ratio of effective sedimentation treatment area to EFTA is 1.0. Given these data, one can effectively scale the test results for all commercial systems.

#### *Sample Collection*

The grab sampling method was used for all sample collection by sweeping a wide-mouth 1-L plastic bottle through the free-discharge effluent stream, to ensure the full cross section of the flow was sampled. The start time for each run was recorded.

The sampling schedule is provided in **Table 1**. The detention time for the Isolator Row PLUS unit operating at 20 inches hydrostatic head (maximum head tested) is 2.1 minutes. To comply with the NJDEP Filter Protocol, after initiating and stabilizing the flow rate at the MTFR and beginning sediment feed, effluent sampling did not begin until the filtration MTD has been in operation for a minimum of three detention times.

Background water samples were collected upstream of the doser (shown in **Figures 3 and 8**) in correspondence with the odd-numbered effluent samples (i.e., Samples E1, E3, E5 at t = 9, 20, 31 minutes).

**Table 1 Sampling Schedule for the Isolator Row PLUS Tests**

Time (min)	Sample(s)	Time (min)	Sample(s)
0	S1	22	S3
9	E1, BG1	31	E5, BG3
10	E2	32	E6
11	S2	33	Stop Flow
20	E3, BG2	N/A	DDA
21	E4	N/A	DDB

NOTE: S = sediment rate; E = effluent; BG = background; DD = drawdown



**Figure 8 Photograph of Background Sampling Port**

Two evenly-volume-spaced drawdown samples, DDA and DDB, were taken after the flow and sediment feed to the unit had been stopped.

Sediment injection rates were measured using a stopwatch and the mass collected measured on a calibrated scale once at the very beginning of the run and twice more during the run. A fourth sediment rate sample was taken after the run was finished as an internal check but was not included in the calculations for the report. The duration of each run was 33 minutes.

A Chain of Custody (COC) form was used for each test run to record sampling date and time for externally analyzed samples. Copies of these forms were maintained by BaySaver Laboratory and FTL. Sample bottles were labeled to identify the test run number and sample type (e.g., background, effluent), corresponding to the sample identification on the COC form. BEC was present during each test run and witnessed labeling, completion of COC forms, and packaging of

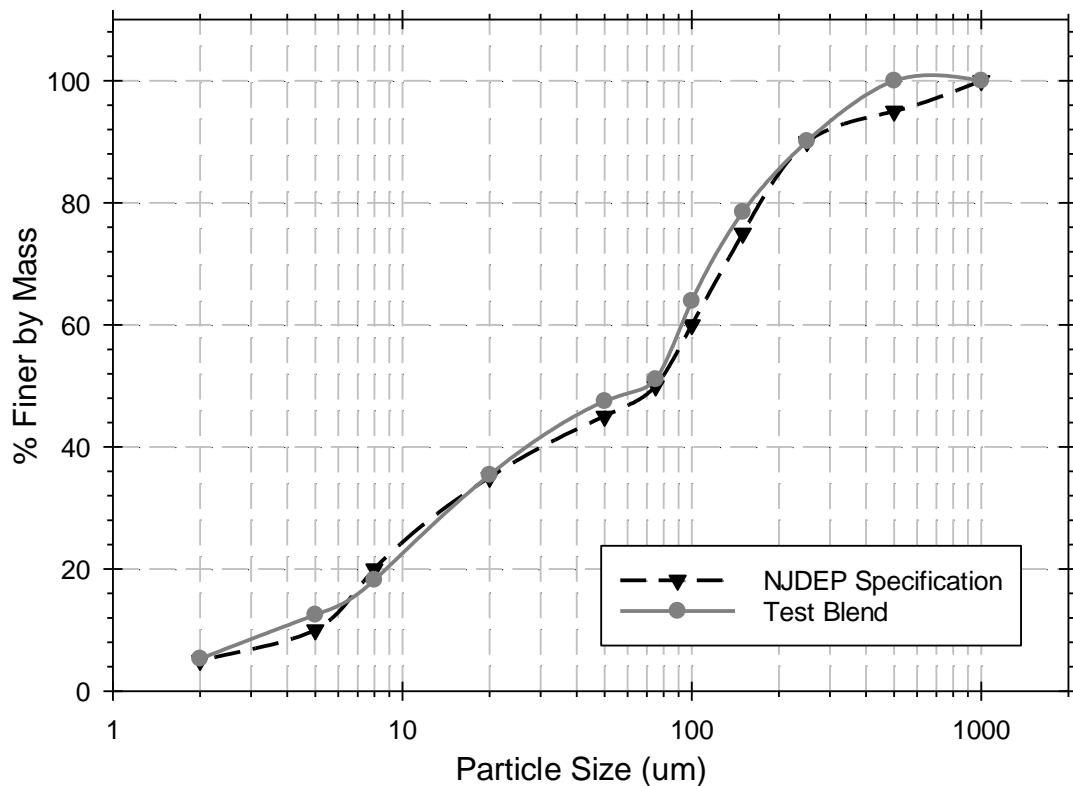
samples for delivery to the external laboratory (FTL). Each person taking or relinquishing possession of the samples was required to sign a COC form before samples changed hands.

#### *Other Instrumentation and Measurement*

Water temperature was recorded every minute by a HOBO data logger placed in the source water tank of the test system. The water level in the Isolator Row PLUS was recorded every 5 minutes by visual observation of a yardstick mounted through the observation port on top of the first chamber. Run and sampling times were measured using a digital timer and a stopwatch, respectively.

#### **2.2 Test Sediment**

The test sediment had the particle size distribution (PSD) presented in **Figure 9**. The test sediment was custom-blended using various commercially available silica sands. The resulting blended sediment met the specification for the NJDEP Filter Protocol. The test sediment was batched, labeled, and stored in covered bins for the duration of this project. Under the supervision of BEC, twenty-one subsamples, taken from various locations within the test sediment containers, were composited. From the composite, three random samples were taken for PSD and moisture content analyses, which were performed by ECS, using the methodology of ASTM method D422-63 (2007).



**Figure 9 Average Particle Size Distribution of Test Sediment Verified by ECS**

The PSD test analysis results are summarized in **Table 2**. ECS results showed that 17-19% of the particles were less than 8  $\mu\text{m}$  and 89-90% of the particles were less than 250  $\mu\text{m}$ . The  $d_{50}$  values (approximately 72  $\mu\text{m}$ ) also indicated that there was no significant difference between the NJDEP target gradation and the ECS-verified gradation of the test sediment. Thus, the blended test sediment was found to meet the NJDEP particle size specification and was acceptable for use. ECS also analyzed the sediment samples for moisture. The average moisture content was 0.1%.

**Table 2 Particle Size Distribution of Test Sediment as Analyzed by ECS**

Particle Size ( $\mu\text{m}$ )	Test Blend % Finer by Mass Analyzed by ECS					NJDEP Specification (minimum % finer)
	NJ Blend A	NJ Blend B	NJ Blend C	Average		
1000	100.0	100.0	100.0	100.0		98
500	100.0	100.0	100.0	100.0		93
250	90.3	89.8	90.2	90.1		88
150	79.3	78.1	78.1	78.5		73
100	66.0	63.2	62.7	63.9		58
75	52.0	50.9	50.3	51.1		50
50	47.5	47.7	47.4	47.5		43
20	35.9	36.0	34.3	35.4		33
8	18.6	18.7	17.4	18.2		18
5	13.0	13.0	11.6	12.5		8
2	5.5	5.4	5.1	5.3		3
$d_{50}$	69 $\mu\text{m}$	72 $\mu\text{m}$	74 $\mu\text{m}$	72 $\mu\text{m}$		75 $\mu\text{m}$

### 2.3 Sediment Removal Efficiency Testing

Sediment removal efficiency testing adhered to the guidelines set forth in Section 5 of the NJDEP Laboratory Protocol for Filtration MTDs. The target flow through the system was 225 gpm, with a target sediment concentration of 200 mg/L. All samples were collected in clean, 1-L wide-mouth bottles. Three background samples were taken at 9, 20 and 31 minutes after the test began to ensure the supply water met the sediment concentration requirement. According to the NJDEP Filter Protocol, these background concentrations cannot exceed a TSS concentration of 20 mg/L.

The test sediment screw-auger feeder introduced the test sediment into the influent stream to achieve the target influent TSS concentration of 200 mg/L. According to the NJDEP Filter Protocol, this influent concentration must stay within 10% of target, allowing for a 180 mg/L to 220 mg/L influent concentration. The feeder was calibrated prior to each run. In order to confirm sediment feed rates during the test, in accordance with the NJDEP Filter Protocol, three samples of the test sediment were collected from the injection point (**Figure 3**, “Doser”) into a clean one-liter container for verification of sediment feed rate, over an interval timed to the nearest second, with a minimum volume of 0.1 liter or a collection interval not exceeding one minute (whichever came first). The time was measured with a stopwatch. The samples were weighed to the nearest

milligram in the BaySaver Laboratory under the observation of BEC. The sediment feed rate coefficient of variance (COV) for the test sediment samples did not exceed 0.10. The mass from the sediment feed rate measurement samples was subtracted from the total mass introduced to the system when removal efficiency was calculated.

Effluent sampling was performed by the grab sampling method during each run, according to the schedule in **Table 1**. When the test sediment feed was interrupted for test sediment measurements, the next effluent samples were collected after at least three detention times had elapsed. During the drawdown period, two evenly volume-spaced samples were collected after flow and sediment feed had stopped. All sediment concentration samples were analyzed by Fredericktowne Labs (FTL) using ASTM D3977-97 (2019) “Standard Test Methods for Determining Sediment Concentrations in Water Samples.”

## **2.4 Sediment Mass Loading Capacity**

The sediment mass loading capacity testing occurred as a continuation of removal efficiency testing, with the target for influent concentration remaining at 200 mg/L, and all aspects of testing procedures kept the same to ensure consistency throughout. The sediment mass loading capacity of the Isolator Row PLUS is defined per the protocol as the point at which the cumulative mass removal drops below 80.0%. For this testing program, the sediment mass loading testing was stopped prior to that point (after Run 16), because it was incorrectly assumed this criterion was reached. Thus, the mass loading is defined as mass loaded into the unit through the end of Run 16.

## **3. Supporting Documentation**

The Procedure for Obtaining Verification of a Stormwater Manufactured Treatment Device from NJCAT states that copies of the laboratory test reports, all data from performance evaluation test runs, original data, pertinent calculations, and documentation of any maintenance activities that occur during the testing process are to be included in this section. All of this information has been provided to NJCAT and is available upon request. It is not practical to include it in this report.

## **4. Testing Results**

A total of 16 removal efficiency testing runs were completed in accordance with the NJDEP filter protocol. The target flow and influent sediment concentration were 225 gpm and 200 mg/L, respectively. The results from all 16 runs were used to calculate the overall cumulative removal efficiency of the Isolator Row PLUS.

### **4.1 Flow Rate**

Flow was monitored by an inline flow meter (FloCat MFE electromagnetic flow meter) and recorded by a SeaMetrics DL76 data logger every minute during each run. For each run, the flow was maintained within 10% of the target (202.5 – 247.5 gpm). The average flow for all 16 runs was 226.1 gpm. The flow data with coefficient of variance (COV) values for all 16 runs are summarized in **Table 3**.

## 4.2 Water Temperature

Temperatures were recorded every minute by a HOBO water level logger (U20L-04). On average for all runs, the water temperature during testing was 45.7 degrees Fahrenheit, with a maximum of 52.2 degrees Fahrenheit, meeting the NJDEP Filter Protocol requirement to be below 80 degrees Fahrenheit. Data are summarized in **Table 3**.

**Table 3 Flow Rate and Temperature Summary for All Runs**

Run	Max Flow (gpm)	Min Flow (gpm)	Average Flow (gpm)	Flow COV	Flow Compliance (COV < 0.1)	Maximum Temperature (Fahrenheit)	NJDEP Temperature Compliance (< 80 F)
1	232.8	223.9	226.3	0.0078	Y	48.2	Y
2	228.9	218.6	220.8	0.0104	Y	51.5	Y
3	229.4	220.0	227.2	0.0094	Y	44.7	Y
4	230.2	218.7	223.2	0.0138	Y	40.5	Y
5	228.7	216.9	222.2	0.0103	Y	44.7	Y
6	227.6	217.0	224.2	0.0115	Y	46.7	Y
7	229.7	221.9	226.4	0.0092	Y	44.6	Y
8	230.3	222.2	226.8	0.0089	Y	43.5	Y
9	233.2	218.4	225.6	0.0136	Y	45.5	Y
10	232.2	219.7	228.4	0.0126	Y	44.7	Y
11	226.9	219.2	224.1	0.0088	Y	52.4	Y
12	232.2	222.1	226.9	0.0107	Y	48.5	Y
13	234.7	221.2	226.1	0.0109	Y	48.5	Y
14	231.9	223.4	228.7	0.0103	Y	45.6	Y
15	236.8	224.1	231.4	0.0131	Y	52.2	Y
16	232.5	221.3	229.0	0.0137	Y	47.8	Y
Average		226.1				45.7	
Max						52.2	

## 4.3 Head

The head level in the Isolator Row PLUS was recorded to the nearest 1/8 inch every five minutes, through visual observation of a yard stick mounted through the observation port of the first chamber. With each run, after the first several measurements, the head during the run remained the same or increased slightly over that of the previous run. The maximum head reached during all 16 runs was 18.75 inches. Maximum head for each run is summarized in **Table 4**.

**Table 4 Maximum Head (inches) for All Runs**

Run	Maximum Head (inches)	Run	Maximum Head (inches)
1	9.00	9	17.50
2	12.00	10	18.00
3	14.00	11	17.25
4	15.25	12	18.00
5	15.75	13	18.25
6	16.25	14	18.50
7	17.50	15	18.75
8	17.25	16	18.75

#### **4.4 Sediment Concentration and Removal Efficiency**

##### *Background TSS*

Municipal tap water was used as the water source during testing. The background TSS concentration for all runs was well below the 20 mg/L NJDEP Protocol limit. Background TSS concentrations for each run are provided in **Table 5**. The average background TSS concentration for each run was subtracted from the effluent and drawdown concentrations to provide adjusted figures, per the protocol.

##### *Sediment Dosing Rate and Influent TSS*

Influent TSS concentration was calculated by dividing the total mass of sediment added during a given run by the total volume of water flowing through the MTD during the addition of test sediment during that run. The volume of water flowing through the device during the run was calculated by multiplying the average measured flow by the time of sediment addition only. The average influent TSS was 204.2 mg/L, with individual run averages ranging from 195.9 to 216.7 mg/L. All values are within the target range of  $200 \pm 20$  mg/L. **Tables 6 and 7** provide the measured sediment rates for each run, and the resulting calculated influent TSS concentration. In these tables, NJDEP Protocol compliance is defined as a TSS concentration in the range 180 – 220 mg/L and sediment feed rate COV < 0.1.

**Table 5 Background TSS Concentrations**

Run	BG TSS 9 min	BG TSS 20 min	BG TSS 31 min	Average	MDL
	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)
1	0.5	4	2	2.2	1.0
2	1	1	0.5	0.8	1.0
3	1	0.5	0.5	0.7	1.0
4	0.5	0.5	0.5	0.5	1.0
5	0.5	0.5	0.5	0.5	1.0
6	0.5	0.5	0.5	0.5	1.0
7	0.5	0.5	0.5	0.5	1.0
8	0.5	0.5	0.5	0.5	1.0
9	0.5	0.5	0.5	0.5	1.0
10	0.5	0.5	0.5	0.5	1.0
11	0.5	0.5	0.5	0.5	1.0
12	0.5	0.5	0.5	0.5	1.0
13	0.5	0.5	0.5	0.5	1.0
14	0.5	0.5	0.5	0.5	1.0
15	0.5	0.5	0.5	0.5	1.0
16	0.5	0.5	0.5	0.5	1.0

Note: In cases where the measured background TSS concentration was below the Minimum Detection Level (MDL) of 1.0 mg/L, half the MDL was reported for the background concentration.

**Table 6 Sediment Rate Measurements for Runs 1-10**

Run	Run Time (min)	Sediment Weight (g)	Duration (s)	Sediment Feed Rate (g/min)	Influent Water Flow Rate (gpm)	Influent TSS Conc. (mg/L)	NJDEP Compliance
1	0	117.767	39.78	177.6	226.3	202.9	Y
	11	110.674	40.16	165.4			
	22	118.819	40.00	178.2			
	COV			0.0418			
2	0	114.921	39.91	172.8	220.8	198.5	Y
	11	106.158	39.96	159.4			
	22	110.429	40.10	165.2			
	COV			0.0404			
3	0	117.364	39.85	176.7	227.2	206.8	Y
	11	116.700	39.90	175.5			
	22	120.156	39.72	181.5			
	COV			0.0179			
4	0	121.043	39.79	182.5	223.2	216.7	Y
	11	125.058	39.88	188.2			
	22	118.657	39.85	178.7			
	COV			0.0261			
5	0	111.624	40.03	167.3	222.2	215.0	Y
	11	117.883	40.00	176.8			
	22	132.393	39.88	199.2			
	COV			0.0904			
6	0	114.723	39.94	172.3	224.2	206.6	Y
	11	119.043	40.03	178.4			
	22	117.644	40.28	175.2			
	COV			0.0174			
7	0	115.351	40.00	173.0	226.4	198.1	Y
	11	110.196	40.25	164.3			
	22	114.603	40.00	171.9			
	COV			0.0281			
8	0	115.664	39.72	174.7	226.8	201.5	Y
	11	117.915	39.93	177.2			
	22	110.840	39.82	167.0			
	COV			0.0307			
9	0	116.845	39.87	175.8	225.6	205.2	Y
	11	114.135	39.81	172.0			
	22	117.894	39.75	178.0			
	COV			0.0172			
10	0	111.306	39.57	168.8	228.4	203.0	Y
	11	119.680	39.81	180.4			
	22	118.275	39.90	177.9			
	COV			0.0347			

**Table 7 Sediment Rate Measurements for Runs 11-16**

Run #	Run Time (min)	Sediment Weight (g)	Duration (s)	Sediment Feed Rate (g/min)	Influent Water Flow Rate (gpm)	Influent TSS Conc. (mg/L)	NJDEP Compliance
11	0	114.505	39.90	172.2	224.1	207.8	Y
	11	119.160	39.94	179.0			
	22	118.629	40.03	177.8			
	COV			0.0207			
12	0	115.516	39.78	174.2	226.9	208.8	Y
	11	118.805	39.87	178.8			
	22	124.236	40.22	185.3			
	COV			0.0311			
13	0	114.776	39.78	173.1	226.1	198.0	Y
	11	106.924	39.85	161.0			
	22	115.083	39.69	174.0			
	COV			0.0429			
14	0	112.871	39.72	170.5	228.7	199.9	Y
	11	116.869	39.84	176.0			
	22	114.529	39.81	172.6			
	COV			0.0161			
15	0	112.091	39.72	169.3	231.4	195.9	Y
	11	112.200	39.81	169.1			
	22	117.588	39.94	176.6			
	COV			0.0250			
16	0	118.503	39.59	179.6	229.0	202.3	Y
	11	116.834	39.78	176.2			
	22	112.971	39.84	170.1			
	COV			0.0273			

#### *Effluent TSS*

During each run, grab samples were taken of the effluent according to the schedule in **Table 1**, and all TSS analyses were conducted by Fredericktowne Labs. For each run, the average effluent concentration was adjusted by subtracting the average background TSS concentration. The average adjusted effluent TSS concentration during testing was 39 mg/L, with individual run averages ranging from 32.0 to 45.5 mg/L. Effluent and adjusted effluent TSS concentrations for each run are given in **Table 8**.

**Table 8 Effluent Sample TSS Concentrations**

Run	EFF TSS 9 min	EFF TSS 10 min	EFF TSS 20 min	EFF TSS 21 min	EFF TSS 31 min	EFF TSS 32 min	Mean	MDL	Adjusted Effluent TSS
	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)
1	48	48	47	47	48	48	47.7	1.0	45.5
2	32	32	33	32	35	33	32.8	1.0	32.0
3	33	37	37	40	38	38	37.2	1.0	36.5
4	28	31	34	38	32	38	33.5	1.0	33.0
5	40	41	39	33	42	42	39.5	1.0	39.0
6	38	41	39	37	41	44	40.0	1.0	39.5
7	37	40	37	36	37	38	37.5	1.0	37.0
8	38	41	38	40	32	38	37.8	1.0	37.3
9	35	41	36	36	42	41	38.5	1.0	38.0
10	39	44	34	38	37	41	38.8	1.0	38.3
11	35	41	38	38	38	43	38.8	1.0	38.3
12	36	43	36	41	46	47	41.5	1.0	41.0
13	41	46	37	37	42	45	41.3	1.0	40.8
14	44	49	39	42	42	45	43.5	1.0	43.0
15	40	43	41	39	40	45	41.3	1.0	40.8
16	43	45	41	44	45	46	44.0	1.0	43.5

Note: Adjusted effluent TSS concentration is the average effluent TSS concentration minus the average background TSS concentration (Table 5).

#### *Drawdown TSS*

According to the NJDEP Filter Protocol, the amount of sediment that leaves the filter during the drawdown period must be accounted for and documented. During each run, two evenly volume-spaced grab samples were taken of the drawdown, and all TSS analyses were conducted by Fredericktowne Labs. For each run, the average drawdown concentration was adjusted by subtracting the average background TSS concentration (**Table 9**).

**Table 9 Drawdown Sample TSS Concentrations**

Run	DDA	DDB	Average	MDL	Adjusted Drawdown TSS
	(mg/L)	(mg/L)	(mg/L)	(mg/L)	(mg/L)
1	62	11	36.5	1.0	34.3
2	39	16	27.5	1.0	26.7
3	42	14	28.0	1.0	27.3
4	41	18	29.5	1.0	29.0
5	42	16	29.0	1.0	28.5
6	45	17	31.0	1.0	30.5
7	44	16	30.0	1.0	29.5
8	48	17	32.5	1.0	32.0
9	42	18	30.0	1.0	29.5
10	45	17	31.0	1.0	30.5
11	43	17	30.0	1.0	29.5
12	44	16	30.0	1.0	29.5
13	46	18	32.0	1.0	31.5
14	50	18	34.0	1.0	33.5
15	47	17	32.0	1.0	31.5
16	48	15	31.5	1.0	31.0

Note: Adjusted drawdown TSS concentration is the average drawdown TSS concentration minus the average background TSS concentration (Table 5).

In order to estimate the volume of water during drawdown, under observation by BEC, the unit was filled prior to all testing with clean water and the drawdown volume as a function of time was calculated from the height of the flow stream in the effluent pipe as a function of time. Total drawdown volume was estimated at 268.6 gal at an operating head of 2.5 inches. This volume was used to determine the volume of the void space of the gravel bed, which was then used, along with the dimensions of the Isolator Row PLUS chambers, to calculate the drawdown volume for incremental head levels above 2.5 inches. Adjusted average drawdown TSS concentrations and drawdown losses are given in **Table 10**.

**Table 10 Drawdown Losses**

Run	Head Level at End of Run (in)	Drawdown Volume (gal)	Average Adjusted Drawdown TSS Conc. (mg/L)	Total Sediment Lost During Drawdown (g)
1	9.00	285.2	34.3	37.1
2	12.00	354.2	26.7	35.7
3	14.00	403.3	27.3	41.7
4	15.25	432.8	29.0	47.5
5	15.75	443.9	28.5	47.9
6	16.25	454.2	30.5	52.4
7	17.50	476.0	29.5	53.2
8	17.00	468.2	32.0	56.7
9	17.25	472.3	29.5	52.7
10	17.75	476.0	30.5	55.0
11	17.25	472.3	29.5	52.7
12	17.5	476.0	29.5	53.2
13	18.00	482.4	31.5	57.5
14	18.25	484.9	33.5	61.5
15	18.50	486.8	31.5	58.1
16	18.25	484.9	31.0	56.9

#### *Removal Efficiency Calculation*

Removal efficiency was calculated using the following equation from the NJDEP Filter Protocol:

$$\text{Removal Efficiency (\%)} = \frac{\left( \frac{\text{Average Influent TSS Concentration} \times \text{Total Volume of Test Water}}{\text{Total Volume of Effluent Water}} \right) - \left( \frac{\text{Adjusted Effluent TSS Concentration} \times \text{Total Volume of Effluent Water}}{\text{Total Volume of Drawdown Water}} \right)}{\text{Average Influent TSS Concentration} \times \text{Total Volume of Test Water}} \times 100$$

For each run, sediment concentrations of background, influent, effluent, and drawdown, as well as the calculated removal efficiency, are summarized in **Table 11**. As shown in this summary table, the Isolator Row PLUS demonstrated a cumulative sediment removal efficiency of 81.2% over the course of 16 test runs.

**Table 11 Removal Efficiency Results**

Run	Average Influent TSS (mg/L)	Influent Water Volume (gal)	Adjusted Average Effluent TSS (mg/L)	Effluent Water Volume (gal)	Adjusted Average Drain Down TSS (mg/L)	Drain Down Water Volume (gal)	Single Run Removal Efficiency (%)	Mass of Captured Sediment (g)	Cumulative Removal Efficiency (%)
1	203	7166	46	6881	34	285	77.8	4282	77.8
2	199	6993	32	6639	27	354	84.0	4415	80.8
3	207	7197	37	6793	27	403	82.6	4654	81.4
4	217	7068	33	6635	29	433	84.9	4923	82.3
5	215	7037	39	6593	29	444	82.2	4705	82.3
6	207	7097	40	6643	31	454	81.2	4504	82.1
7	198	7169	37	6693	30	476	81.6	4386	82.0
8	201	7184	37	6716	32	468	81.6	4473	82.0
9	205	7147	38	6675	30	472	81.8	4539	82.0
10	203	7235	38	6759	31	476	81.4	4523	81.9
11	208	7096	38	6624	30	472	81.8	4567	81.9
12	209	7185	41	6709	30	476	80.7	4584	81.8
13	198	7162	41	6680	32	482	79.7	4277	81.6
14	200	7242	43	6757	34	485	78.8	4318	81.4
15	196	7329	41	6842	32	487	79.5	4320	81.3
16	202	7254	44	6769	31	485	78.9	4384	81.2
Ave.	204.2	7160	39	6713	31	447	81.2	4491	N/A
<b>Cumulative Mass Removed (g)</b>							71854		
<b>Cumulative Mass Removed (lb)</b>							158.4		
<b>Total Mass Loaded (lb)</b>							195.2		
<b>Cumulative Removal Efficiency (%)</b>							81.2		

#### 4.5 Sediment Mass Loading

Sediment mass loading for each run was approximately 12.2 lbs on average. These data are summarized in **Table 12**.

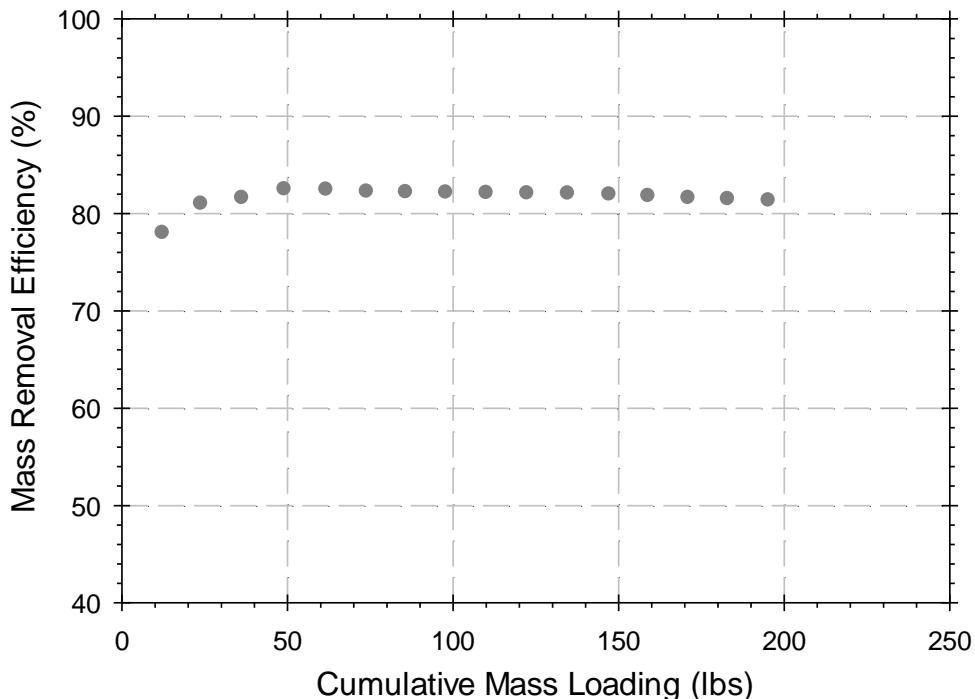
Sediment mass loading was calculated from the summation of the total sediment mass added during dosing in each run.

**Table 12 Sediment Mass Loading Summary**

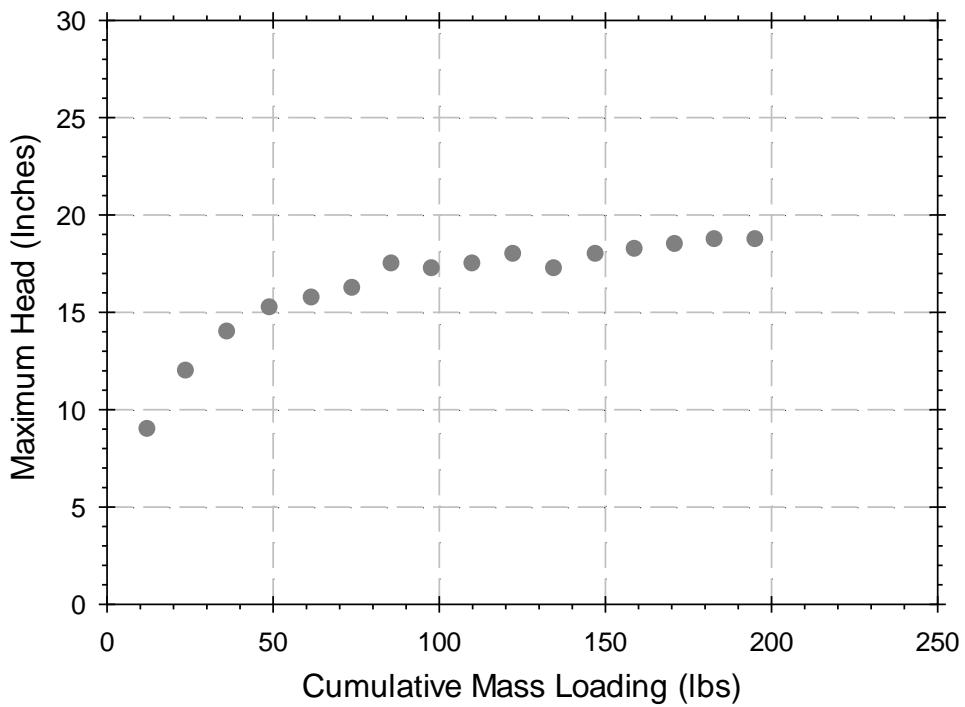
Run	Sediment Loading (lbs)	Cumulative Sediment Loading (lbs)	Run	Sediment Loading (lbs)	Cumulative Sediment Loading (lbs)
1	12.1	12.1	9	12.2	110.0
2	11.6	23.7	10	12.3	122.2
3	12.4	36.1	11	12.3	134.5
4	12.8	48.9	12	12.5	147.0
5	12.6	61.5	13	11.8	158.9
6	12.2	73.8	14	12.1	170.9
7	11.9	85.6	15	12.0	182.9
8	12.1	97.7	16	12.2	195.2

Overall, a total of 195.2 lbs of sediment was loaded into the Isolator Row PLUS over the course of the 16 runs. Total captured mass over the 16 runs was 158.4 lbs (**Table 11**).

The relationship between removal efficiency and sediment mass loading is shown in **Figure 10**. The relationship between driving head and sediment mass loading is shown in **Figure 11**.



**Figure 10 Removal Efficiency vs. Sediment Mass Loading**



**Figure 11 Driving Head vs. Sediment Mass Loading**

## 5. Performance Verification

The Isolator Row PLUS used in this test, constructed from two (2) overlapping StormTech SC-740 chambers and one layer of ADS PLUS fabric, demonstrated a cumulative mass TSS removal efficiency of 81.2% and a sediment mass loading capacity of 3.58 lb./ft<sup>2</sup> (mass capture capacity of 2.91 lb./ft<sup>2</sup>) of geotextile fabric filtration area when operated with a driving head < 20 inches at a hydraulic loading rate of 4.13 gpm/ft<sup>2</sup> of geotextile fabric filtration area. The MTFR's and maximum allowable drainage area for other StormTech Isolator Row PLUS models are shown in **Table 13**.

**Table 13 Isolator Row PLUS System Model Sizes and New Jersey Treatment Capacities**

Model	Surface Loading Rate (gpm/ft <sup>2</sup> )	Effective Filtration Treatment Area (ft <sup>2</sup> )	MTFR (cfs) <sup>1</sup>	Mass Loading Capacity (lbs)	Mass Capture Capacity (lbs)	Drainage Area (acres)
	Single Chamber	Single Chamber		Single Chamber	Single Chamber	Single Chamber
<b>StormTech SC-160</b>	4.13	11.45	0.105	41.0	33.4	0.06
<b>StormTech SC-310</b>	4.13	17.7	0.163	63.4	51.6	0.09
<b>StormTech SC-740</b>	4.13	27.8	0.256	99.6	81.0	0.14
<b>StormTech DC-780</b>	4.13	27.8	0.256	99.6	81.0	0.14
<b>StormTech MC-3500</b>	4.13	42.9	0.395	153.7	125.0	0.21
<b>StormTech MC-4500</b>	4.13	30.1	0.277	107.8	87.7	0.15

1. Based on 4.13 gpm/ft<sup>2</sup> of effective filtration treatment area.  
 2. Drainage Area is based on the equation in the NJDEP Filter Protocol wherein drainage area is calculated by dividing the pounds of mass captured by 600 lb/acre.

## 6. Design Limitations

### *Maximum Flow Rate*

The StormTech Isolator Row PLUS unit has an MTFR of 0.501 cfs (225 gpm) and an effective filtration treatment area (EFTA) of 54.5 ft<sup>2</sup> (loading rate 4.13 gpm/ft<sup>2</sup>).

### *Slope*

The StormTech Isolator Row PLUS is recommended for installation with little to no slope to ensure proper, consistent operation. Steep slopes should be reviewed by ADS/StormTech Engineering support.

### *Allowable Head Loss*

There is an operational head loss associated with the StormTech Isolator Row PLUS. The head loss will increase over time due to the sediment loading to the system. Site-specific treatment flow rates, peak flow rates, pipe diameter, and pipe slopes should be evaluated to ensure there is appropriate head for the system to function properly.

### *Sediment Load Capacity*

Based on laboratory testing results, the StormTech Isolator Row PLUS unit has a mass loading capacity of 195.2 lbs. while operating at a sediment removal efficiency of 81.2%; the total sediment load captured by the tested Isolator Row PLUS is 158.4 lbs.

### *Pre-treatment Requirements*

The StormTech Isolator Row PLUS unit does not require additional pre-treatment.

### *Configurations*

The StormTech Isolator Row PLUS is available in multiple configurations. The length and size can be adjusted to meet project specific design volumes or flow rates.

### *Structure Load Limitations*

The StormTech Isolator Row PLUS, as part of the overall chamber system, is designed to meet the full scope of design requirements of the American Society of Testing Materials (ASTM) International specification F2787 “Standard Practice for Structural Design of Thermoplastic Corrugated Wall Stormwater Collection Chambers” and produced to the requirements of the ASTM F2418 “Standard Specification for Polypropylene (PP) Corrugated Stormwater Collection Chambers”. The StormTech chambers provide the full AASHTO safety factors for live loads and permanent earth loads. The ASTM F 2787 standard provides specific guidance on how to design thermoplastic chambers in accordance with AASHTO Section 12.12. of the AASHTO LRFD Bridge Design Specifications. ASTM F 2787 requires that the safety factors included in the AASHTO guidance are achieved as a prerequisite to meeting ASTM F 2418. The three standards provide both the assurance of product quality and safe structural design.

## **7. Maintenance Plan**

The frequency of Inspection and Maintenance varies by location. A routine inspection schedule needs to be established for each individual location, based upon site-specific variables. The type of land use (i.e. industrial, commercial, public, residential), anticipated pollutant load, percent imperviousness, climate, rainfall data, etc., all play a critical role in determining the actual frequency of inspection and maintenance practices.

The Isolator Row PLUS may also be part of a treatment train. By treating stormwater prior to entry into the chamber system, the service life can be extended and pollutants such as hydrocarbons can be captured.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row PLUS chamber should be inspected every 6 months for the first year of operation. For subsequent years, the inspection schedule should be adjusted based upon previous observation of sediment deposition.

The Isolator Row PLUS incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the Isolator Row PLUS from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If, upon visual inspection, it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row PLUS, clean-out should be performed.

The Isolator Row PLUS was designed to reduce the cost of periodic maintenance. By “isolating” sediment to just one row of the StormTech system, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high-pressure water nozzle to propel itself down the Isolator Row PLUS while scouring and suspending sediment. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency.

Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear-facing jets with an effective spread of at least 45” are best. Most JetVac reels have 400 feet of hose, allowing maintenance of an Isolator Row PLUS up to 50 chambers long. The JetVac process should only be performed on StormTech Isolator Rows PLUS that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.

Complete details of the design, operation, and maintenance of the Isolator Row PLUS can be found in the StormTech O&M Manual, available online at:

[https://www.stormtech.com/download\\_files/pdf/11081-stormtech-isolator-row-plus-manual-07-20.pdf](https://www.stormtech.com/download_files/pdf/11081-stormtech-isolator-row-plus-manual-07-20.pdf)

## **8. Statements**

The attached pages include signed statements from the manufacturer (Advanced Drainage Systems, Inc.), the third-party environmental consulting firm (Boggs Environmental Consultants, Inc.), and NJCAT. These statements are included as a requirement for the verification process.



June 26<sup>th</sup>, 2020

Dr. Richard S. Magee, Sc.D., P.E., BCEE  
NJCAT  
Center for Environmental Systems  
Steven Institute of Technology  
Castle Point on Hudson  
Hoboken, NJ 07030-0000

Dr. Magee,

Advanced Drainage Systems is pleased to provide this letter as our statement certifying that the protocol, "New Jersey Department of Environmental Protection Laboratory Protocol to Assess Total Suspended Solids Removal by a filtration Manufactured Treatment Device" (NJDEP Filter Protocol, January 25, 2013), was strictly followed while testing our StormTech Isolator® Row PLUS. The testing was performed at BaySaver Laboratories, located in Mount Airy, MD. All data pertaining to the StormTech Isolator Row PLUS NJDEP Protocol test is included in the Verification Report.

Respectfully,

Greg Spires, PE  
General Manager - StormTech  
Advanced Drainage Systems  
614.325.0032  
[greg.spires@ads-pipe.com](mailto:greg.spires@ads-pipe.com)



**BOGGS**  
ENVIRONMENTAL CONSULTANTS

Middletown, MD & Morgantown, WV

Administrative Office:  
200 W Main Street      Office (301) 694-5607  
Middletown, Maryland 21769      Fax (301) 694-9799

June 25, 2020

StormTech  
Advanced Drainage Systems, Inc.  
520 Cromwell Avenue  
Rocky Hill, CT 06067  
[gregory.spires@ads-pipe.com](mailto:gregory.spires@ads-pipe.com)

ATTENTION      Greg Spires, PE  
General Manager, StormTech  
Advanced Drainage Systems, Inc.

REFERENCE:      Third Party Review of Testing Procedures of the Isolator® Row PLUS at the  
BaySaver Laboratory  
1207 Park Ridge Drive  
Mount Airy, MD 21771

**BOGGS ENVIRONMENTAL CONSULTANTS, INC.** (BEC) provided Third Party Review services for the testing of the Isolator® Row PLUS to evaluate if the required testing meets certification standards established by the New Jersey Department of Environmental Protection (NJDEP).

#### LABORATORY TESTING PROCEDURES & METHODOLOGIES

The following two procedures and testing requirements were followed during the testing process of the Isolator® Row PLUS:

- *New Jersey Department of Environmental Protection, Laboratory Protocol to Assess Total Suspended Solids Removal by a Filtration Manufactured Treatment Device, dated January 25, 2013.*
- *QAPP for Isolator® Row PLUS, New Jersey Department of Environmental Protection Testing, prepared by StormTech (a subsidiary of Advanced Drainage Systems, Inc.), Revision dated January 9, 2020.*

#### ONSITE THIRD-PARTY OBSERVATION OF TESTING PROCEDURES

BEC was present at the BaySaver Laboratory, at 1207 Park Ridge Drive, in Mount Airy, MD 21771, to observe the following testing of the Isolator® Row PLUS:

- The mixing and establishment of a sediment blend that included manufactured sands that when delivered to the feed water would result in influent Total Suspended Solids (TSS) concentrations within the established range of approximately 200 mg/L and a particle size distribution specified and approved by NJDEP;
- BEC assisted in the establishment of a Procedure Checklist to be used on each run to verify and document the following: Verify that pumps and measurement devices are turned on and functioning; Verification that the correct measurements of dry sediments are added to the doser and feed stream; Document that, background effluent, and duplicate samples are collected at established intervals during the run; and, Recording of periodic flow rates and head measurements during each run;
- Observation of Runs 1 through 16 from January 14, 2020 to February 12, 2020 and verified that that sediment, background, effluent samples were collected during each 33-minute run, and that drawdown samples were collected after the end of each run.
- After sampling was completed for each run, BEC was present for the downloading of flow data as well as sediment feed rates to verify that calculated sediment feed rates met NJDEP protocols for testing. BEC also verified that that sample containers were properly labeled and chain of custodies were filled and were boxed and sealed for delivery to Fredericktowne Labs for analysis of Total Suspended Solids (TSS).

**ENVIRONMENTAL SCIENCE, ENGINEERING & INDUSTRIAL HYGIENE SERVICES**



Third Party Review of  
Isolator® Row PLUS Testing Procedures  
June 25, 2020  
Page 2 of 2

#### THIRD-PARTY VERIFICATION & OPINIONS

Based on observations during the runs and the reported TSS analytical results, BEC verified the following:

- That the testing of the Isolator® Row PLUS at the BaySaver Laboratory was conducted in accordance with the *New Jersey Department of Environmental Protection, Laboratory Protocol to Assess Total Suspended Solids Removal by a Filtration Manufactured Treatment Device, dated January 25, 2013* and procedures established in Advanced Drainage Systems, Inc.'s *QAPP for Isolator® Row PLUS, New Jersey Department of Environmental Protection Testing*, prepared by StormTech (a subsidiary of Advanced Drainage Systems), Revision dated January 9, 2020.
- The report titled *NJCAT Technology Verification, of Isolator® Row PLUS*, prepared by StormTech, dated June 2020, used applicable NJCAT protocol and accurately reflects the testing observed by BEC.

BEC has no financial conflict of interest, as defined in the *Procedure for Obtaining Verification of a Stormwater Manufactured Treatment Device from New Jersey Corporation of Advanced Technology* (NJEP 2013).

Should you have any questions, contact our office at your earliest convenience.

Sincerely,  
BOGGS ENVIRONMENTAL CONSULTANTS, INC.

A handwritten signature in blue ink that reads "William R. Warfel".

William R. Warfel  
Principal Environmental Scientist



**Center for Environmental Systems  
Stevens Institute of Technology  
One Castle Point  
Hoboken, NJ 07030-0000**

May 1, 2020

George F. Ives III, P.E.  
StormTech, LLC  
520 Cromwell Ave  
Rocky Hill, CT 06067

Dear Mr. Ives,

Based on my review, evaluation and assessment of the testing conducted on the StormTech , LLC Isolator Row PLUS at the BaySaver Laboratory (Storm Tech, LLC and BaySaver Technologies, LLC are subsidiaries of Advanced Drainage Systems, Inc.), under the independent third-party oversight of Boggs Environmental Consultants (BEC), Inc., the test protocol requirements contained in the “New Jersey Department of Environmental Protection Laboratory Protocol to Assess Total Suspended Solids Removal by a Filtration Manufactured Treatment Device” (NJDEP Filter Protocol, January 2013) were met or exceeded. Specifically:

*Test Sediment Feed*

The test blend was custom-blended using various commercially available silica sands under the oversight of BEC. The particle size distribution was independently analyzed by Environmental Consulting Services (ECS), using the methodology of ASTM method D422-63. The blended silica met the specification within tolerance as described in Section 5B of the NJDEP filter protocol and was acceptable for use.

*Removal Efficiency Testing*

Sixteen (16) removal efficiency testing runs were completed in accordance with the NJDEP filter protocol. The target flow rate was 225 gpm and the influent sediment concentration was 200 mg/L. The average flow rate for all 16 runs was 226.1, with a coefficient of variation (COV) below the flow compliance (COV) < 0.1 for all the runs. Likewise, for all runs the sediment feed rate COV was below the < 0.03 protocol limit. The Isolator Row PLUS demonstrated a cumulative sediment removal efficiency of 81.2% over the course of the 16 test runs.

### *Sediment Mass Loading Capacity*

Mass loading capacity testing was conducted concurrently with removal efficiency testing. The Isolator Row PLUS has a mass loading capture capacity of 158.4 lbs (2.91 lbs/ft<sup>2</sup> of filtration area).

No maintenance was performed on the test system during the entire testing program.

### *Scour Testing*

No scour testing was performed. Hence the Isolator Row PLUS is verified for off-line installation only.

Sincerely,

A handwritten signature in blue ink that reads "Richard S. Magee". The signature is fluid and cursive, with "Richard" and "S." being more stylized and "Magee" being more clearly legible.

Richard S. Magee, Sc.D., P.E., BCEE

## Specifications

### *Introduction*

- Manufacturer – StormTech, LLC, 520 Cromwell Ave, Rocky Hill, CT 06067
- Website: <http://www.StormTech.com>. Phone: 888-892-2694
- MTD – StormTech Isolator Row PLUS verified models are shown in **Table 13**
- TSS Removal Rate – 81.2%
- Off-line installation

### *Detailed Specification*

- NJDEP sizing tables and physical dimensions of StormTech Isolator Row PLUS verified models are shown in **Table 13**. These sizing tables are valid for NJ following NJDEP Water Quality Design Storm Event of 1.25" in 2 hours (NJAC 7:8-5.5(a)).
- Maximum inflow drainage area
  - The maximum inflow drainage area is governed by the maximum treatment flow rate of each model as presented in **Table 13**.
- Driving head will vary for a given Isolator Row PLUS model based on the site-specific configuration. The maximum head without bypass is 36", but the minimum head varies depending on the flow rate through the unit. Design support is given by StormTech for each project, and site-specific drawings (cut sheets) will be provided that show pipe inverts, finish surface elevation, and peak treatment and maximum flow rates through the unit.
- The drawdown flow exits via the underdrain. A clean filter draws down in approximately 20 minutes.

## **APPENDIX G: OPERATION AND MAINTENANCE**

- STORMWATER OPERATION AND MAINTENANCE PLAN
- INSPECTION REPORT
- INSPECTION AND MAINTENANCE LOG FORM
- LONG-TERM POLLUTION PREVENTION PLAN
- ILICIT DISCHARGE STATEMENT
- SPILL PREVENTION
- PROPOSED OPERATION AND MAINTENANCE MAP
- MANUFACTURER'S INSPECTION AND MAINTENANCE MANUALS

# **STORMWATER OPERATION AND MAINTENANCE PLAN**

***Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA***

## **RESPONSIBLE PARTY DURING CONSTRUCTION:**

***Rte. 85 Realty Corp.  
32 Hastings Street, P.O. Box 444  
Mendon, MA***

## **RESPONSIBLE PARTY POST CONSTRUCTION:**

***Rte. 85 Realty Corp.  
32 Hastings Street, P.O. Box 444  
Mendon, MA***

### **Construction Phase**

During the construction phase, all erosion control devices and measures shall be maintained in accordance with the final record plans, local/state approvals and conditions, the EPA Construction General Permit and the Stormwater Pollution Prevention Plan (SWPPP) if applicable. Additionally, the maintenance of all erosion / siltation control measures during construction shall be the responsibility of the general contractor. Contact information of the OWNER and CONTRACTOR shall be listed in the SWPPP for this site. The SWPPP also includes information regarding construction period allowable and illicit discharges, housekeeping and emergency response procedures. Upon proper notice to the property owner, the Town/City or its authorized designee shall be allowed to enter the property at a reasonable time and in a reasonable manner for the purposes of inspection.

### **Post Development Controls**

Once construction is completed, the post development stormwater controls are to be operated and maintained in compliance with the following permanent procedures (note that the continued implementation of these procedures shall be the responsibility of the Owner or its assignee):

1. Parking lots: Sweep at least four (4) times per year and on a more frequent basis depending on sanding operations. Swept areas shall include all parking, drive aisles, and access aisles. All resulting sweepings shall be collected and properly disposed of offsite in accordance with MADEP and other applicable requirements.

Approximate Maintenance Budget: \$1,000/year

2. Catch basins, manholes and piping: Inspect four (4) times per year and at the end of foliage and snow-removal seasons. These features shall be cleaned four (4) times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the catch basin or underground system. Accumulated sediment and hydrocarbons present must be removed and properly disposed of off-site in accordance with MADEP and other applicable requirements.

Approximate Maintenance Budget: \$500/year per structure.

3. Underground Infiltration Basins: Preventative maintenance after every major storm event during the first three (3) months of operation and at least twice per year thereafter. Inspect structure and pretreatment BMP to ensure proper operation after every major storm event (generally equal or greater to 3.0 inches in 24 hours) for the first three months. The outlet of the basin, if any, shall be inspected for erosion and sedimentation, and riprap shall be promptly repaired in the case of erosion. Sediment collecting in the bottom of the basin shall be inspected twice annually, and removal shall commence any time the sediment reaches a depth of six inches anywhere in the basin. Any sediment removed shall be disposed of in accordance with MADEP and other applicable requirements.

Approximate Maintenance Budget: Cleaning - \$1,000/year, Inspection - \$200/year

All components of the stormwater system will be accessible by the owner or their assignee.

**STORMWATER MANAGEMENT SYSTEM**  
**POST-CONSTRUCTION INSPECTION REPORT**

**LOCATION:**

***Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA***

**RESPONSIBLE PARTY:**

***Rte. 85 Realty Corp.  
32 Hastings Street, P.O. Box 444  
Mendon, MA***

NAME OF INSPECTOR:	INSPECTION DATE:
Note Condition of the Following (sediment depth, debris, standing water, damage, etc.):	
Catch Basins / Manholes / Piping:	
Underground Infiltration Basin:	
Isolator Row:	
Other:	

Note Recommended Actions to be taken on the Following (sediment and/or debris removal, repairs, etc.):

Catch Basins / Manholes / Piping:

Underground Infiltration Basin:

Isolator Row:

Other:

Comments:

## STORMWATER INSPECTION AND MAINTENANCE LOG FORM

## ***Proposed Bank with Drive Thru***

## **182 & 186 Hartford Avenue - Bellingham, MA**

# **LONG-TERM POLLUTION PREVENTION PLAN**

***Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA***

## **RESPONSIBLE PARTY DURING CONSTRUCTION:**

***Rte. 85 Realty Corp.  
32 Hastings Street, P.O. Box 444  
Mendon, MA***

## **RESPONSIBLE PARTY POST CONSTRUCTION:**

***Rte. 85 Realty Corp.  
32 Hastings Street, P.O. Box 444  
Mendon, MA***

For this site, the Long-Term Pollution Prevention Plan will consist of the following:

- The property owner shall be responsible for “good housekeeping” including proper periodic maintenance of building and pavement areas, curbing, landscaping, etc.
- Proper storage and removal of solid waste (dumpsters).
- Sweeping of parking lots, drive aisles and access aisles a minimum of four times per year with a commercial cleaning unit. Any sediment removed shall be disposed of in accordance with applicable local and state requirements.
- Regular inspections and maintenance of Stormwater Management System as noted in the “O&M Plan”.
- Snow removal shall be the responsibility of the property owner. Snow shall not be plowed, dumped and/or placed in forebays, infiltration basins or similar stormwater controls. Salting and/or sanding of pavement / walkway areas during winter conditions shall only be done in accordance with all state/local requirements and approvals.
- No outdoor maintenance or washing of vehicles allowed.
- Trash and other debris shall be removed from all areas of the site at least twice yearly.
- Reseed any bare areas as soon as they occur. Erosion control measures shall be installed in these areas to prevent deposits of sediment from entering the drainage system.

- Grass shall be maintained at a minimum blade height of two to three inches and only 1/3 of the plant height shall be removed at a time. Clippings shall not be disposed of within stormwater management areas or adjacent resource areas.
- Plants shall be pruned as necessary.
- Snow piles shall be located adjacent to or on pervious surfaces in upland areas. This will allow snow melt water to filter into the soil, leaving behind sand and debris which can be removed in the springtime.
- In no case shall snow be disposed of or stored in resource areas (wetlands, floodplain, streams, or other water bodies).
- In no case shall snow be disposed of or stored in the detention basins, infiltration basins or bioretention areas.
- If necessary, stockpiled snow will be removed from the Site and disposed of at an off-site location in accordance with all local, state and federal regulations.
- The amount of sand and deicing chemicals shall be kept at the minimum amount required to provide safe pedestrian and vehicle travel.
- Deicing chemicals are recommended as a pretreatment to storm events to minimize the amount of applied sand.
- Sand and deicing chemicals should be stockpiled under covered storage facilities that prevent precipitation and adjacent runoff from coming in contact with the deicing materials. Stockpile areas shall be located outside resource areas.
- The primary agents used for deicing at parking lots, sidewalks and the access roads shall consist of salt alternatives such as calcium carbonate (CaCO<sub>3</sub>) or potassium chloride (KCl) or sodium chloride.
- Deliveries shall be monitored by owner or owner's representative to ensure proper delivery and, in the event that a spillage occurs, it shall be contained and cleaned up immediately in accordance with the spill prevention program for the project.
- Recycle materials whenever possible. Provide separate containers for recycle materials. Recycling products will be removed by a certified waste hauler.

## **OPERATION AND MAINTENANCE TRAINING PROGRAM**

The Owner will coordinate an annual in-house training session to discuss the Operations and Maintenance Plan, the Long-Term Pollution Prevention Plan, and the Spill Prevention Plan and response procedures. Annual training will include the following:

### Discuss the Operations and Maintenance Plan:

- Explain the general operations of the stormwater management system and its BMPs
- Identify potential sources of stormwater pollution and measures / methods of reducing or eliminating that pollution
- Emphasize good housekeeping measures

### Discuss the Spill Prevention and Response Procedures:

- Explain the process in the event of a spill
- Identify potential sources of spills and procedures for cleanup and /or reporting and notification
- Complete a yearly inventory or Materials Safety Data sheets of all tenants and confirm that no potentially harmful chemicals are in use.

## **ILLICIT DISCHARGE STATEMENT**

Certain types of non-stormwater discharges are allowed under the U.S. Environmental Protection Agency Construction General Permit. These types of discharges will be allowed under the conditions that no pollutants will be allowed to come in contact with the water prior to or after its discharge. The control measures which have been outlined previously in this LTPPP will be strictly followed to ensure that no contamination of these non-storm water discharges takes place. Any existing illicit discharges, if discovered during the course of the work, will be reported to MassDEP and the local DPW, as applicable, to be addressed in accordance with their respective policies. No illicit discharges will be allowed in conjunction with the proposed improvements.

Duly Acknowledged:

---

Name & Title

Date

## **SPILL PREVENTION AND RESPONSE PROCEDURES (POST CONSTRUCTION)**

In order to prevent or minimize the potential for a spill of Hazardous Substances or Oil or come into contact with stormwater, the following steps will be implemented:

1. All Hazardous Substances or Oil (such as pesticides, petroleum products, fertilizers, detergents, acids, paints, paint solvents, cleaning solvents, etc.) will be stored in a secure location, with their lids on, preferably under cover, when not in use.
2. The minimum practical quantity of all such materials will be kept on site.
3. A spill control and containment kit (containing, for example, absorbent materials, acid neutralizing powder, brooms, dust pans, mops, rags, gloves, goggles, plastic and metal trash containers, etc.) will be provided on site.
4. Manufacturer's recommended methods for spill cleanup will be clearly posted and site personnel will be trained regarding these procedures and the location of the information and cleanup supplies.
5. It is the OWNER's responsibility to ensure that all Hazardous Waste on site is disposed of properly by a licensed hazardous material disposal company. The OWNER is responsible for not exceeding Hazardous Waste storage requirements mandated by the EPA or state and local authorities.

In the event of a spill of Hazardous Substances or Oil, the following procedures should be followed:

1. All measures should be taken to contain and abate the spill and to prevent the discharge of the Hazardous Substance or Oil to stormwater or off-site. (The spill area should be kept well ventilated and personnel should wear appropriate protective clothing to prevent injury from contact with the Hazardous Substances.)
2. For spills of less than five (5) gallons of material, proceed with source control and containment, clean-up with absorbent materials or other applicable means unless an imminent hazard or other circumstances dictate that the spill should be treated by a professional emergency response contractor.
3. For spills greater than five (5) gallons of material immediately contact the MADEP at the toll-free 24-hour statewide emergency number: **1-888-304-1133**, the local fire department (**9-1-1**) and an approved emergency response contractor. Provide information on the type of material spilled, the location of the spill, the quantity spilled, and the time of the spill to the emergency response contractor or coordinator, and proceed with prevention, containment and/or clean-up if so desired. (Use the form provided, or similar).
4. If there is a Reportable Quantity (RQ) release, then the National Response Center should be notified immediately at (800) 424-8802; within 14 days a report should be submitted to the EPA regional office describing the release, the date and circumstances of the release and the steps taken to prevent another release. This Pollution Prevention Plan should be updated to reflect any such steps or actions taken and measures to prevent the same from reoccurring.

## **SPILL PREVENTION CONTROL AND COUNTERMEASURE FORM**

*Proposed Bank with Drive Thru  
182 & 186 Hartford Avenue  
Bellingham, MA*

Where a release containing a hazardous substance occurs, the following steps shall be taken by the facility manager and/or supervisor:

1. Immediately notify The Bellingham Fire Department (at **9-1-1**)
2. All measures must be taken to contain and abate the spill and to prevent the discharge of the pollutant(s) to off-site locations, receiving waters, wetlands and/or resource areas.
3. Notify the Bellingham Board of Health at (508) 966-5820 and the Bellingham Conservation Commission at (508) 657-2858.
4. Provide documentation from licensed contractor showing disposal and cleanup procedures were completed as well as details on chemicals that were spilled to the Bellingham Board of Health and Conservation Commission.

Date of spill: \_\_\_\_\_ Time: \_\_\_\_\_ Reported By: \_\_\_\_\_

Weather Conditions:

Cause of Spill: \_\_\_\_\_  
\_\_\_\_\_

Measures Taken to Clean up Spill: \_\_\_\_\_  
\_\_\_\_\_

Type of equipment: \_\_\_\_\_ Make: \_\_\_\_\_ Size: \_\_\_\_\_

License or S/N: \_\_\_\_\_

Location and Method of Disposal \_\_\_\_\_  
\_\_\_\_\_

Procedures, method, and precautions instituted to prevent a similar occurrence from recurring: \_\_\_\_\_  
\_\_\_\_\_

Additional Contact Numbers:

- DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) EMERGENCY  
PHONE: 1-888-304-1133
- NATIONAL RESPONSE CENTER PHONE: (800) 424-8802
- U.S. ENVIRONMENTAL PROTECTION AGENCY PHONE: (888) 372-7341