



ALLEN ENGINEERING & ASSOCIATES, INC.

DRAINAGE ANALYSIS
For
“Blackstone Street Improvements”
Blackstone Street
In
Bellingham, MA 02019



February 14, 2025

Revised:
June 20, 2025
October 31, 2025
January 22, 2026

Prepared For:
Wall Street Development Corp.
P.O. Box 272
Westwood, MA 02090

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Blackstone Street Improvements
Bellingham, MA

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SUMMARY OF EXISTING DRAINAGE CONDITIONS

Site Description:

The project for which this analysis has been prepared comprises 1,900 linear feet of a town-owned gravel road that runs along a varying width ROW in the town of Bellingham, Massachusetts. The existing gravel road slopes down and splits an existing wetland, then slopes back up the hill to a high point, and then slopes back down the hill before entering the Town of Blackstone. Locus contains several wetland systems, which have been identified and delineated by Goddard Consulting LLC and mapped by Allen Engineering & Associates, Inc.

Allen Engineering & Associates, Inc. has reviewed the Soil Survey for Worcester County prepared by the USDA/NRCS and has found the site to contain several soil types having hydrologic soil group (HSG) designations ranging from "A" through "D". The "D" or very slow infiltrating soils appear to be associated with the mapped wetlands, while the remaining areas are located within areas designated as "Canton" and "Ridgebury". On-site test data is appended to this report in section 4.

Hydrology Background:

Allen Engineering & Associates, Inc. has utilized AutoCAD and HydroCAD software to perform this drainage analysis. Autocad was used to generate the existing and proposed drainage plans that can be found in sections 1 and 2 of this report. These plans were used to define such items as subcatchment areas, time of concentration paths and ground cover. Three design evaluation points numbered EV1, EV2 and EV3 in the calculations have been established corresponding to the existing surface runoff collection area. HydroCAD was used to calculate the rates and volume of storm water runoff during various storm events at these evaluation points. These rates are summarized for the existing and proposed site conditions on page iii.

SUMMARY OF PROPOSED DEVELOPMENT DRAINAGE CONDITIONS

Site Description:

The proposed project (“the Project”) consists of improving the 1,900 lf of gravel road. The proposed road is a 22-foot paved travel way with a modified Cape Cod berm. It ends with a 45-foot radius cul-de-sac. A proposed open box culvert and retaining wall will allow for limited wetland impact and hydraulically connect the two existing wetlands.

Stormwater Management:

The proposed drainage system has been designed to meet the Stormwater Management Standards set by the Massachusetts Department of Environmental Protection (DEP). As detailed in the following standards compliance section the total peak flow rate for the constructed site is less than the existing rates for all storm events examined, and the stormwater runoff is treated to the required level and volume by approved Best Management Practices for Total Suspended Solids removal.

Stormwater runoff from the paved roadway will collect in deep sump hooded catch basins and oil grit chambers. The stormwater will then be sent to three different infiltration basins with sediment forebays. The infiltration basins have been designed in accordance with the Massachusetts Stormwater Management Standards to recharge the required volume of runoff as well as provide temporary storage volume necessary to attenuate post-development peak rates to pre-development conditions.

SUMMARY OF HYDROLOGY

Blackstone Street Improvement
Using HydroCAD Software

| | | | |
|-----------|----------------------------------|------------|-----------|
| Job No.: | 00454 | Calced By: | M. Allen |
| Client: | Wall Street Development Corp | Date: | 2/14/2025 |
| Location: | Blackstone Street, Bellingham MA | Revised: | 22-Jan-26 |

TABLE 1: Summary of Peak Rates of Stormwater Runoff

| Evaluation Point | HydroCAD symbols | Existing Conditions Runoff (CFS) | | | | HydroCAD symbols | Proposed Conditions Runoff (CFS) | | | |
|------------------|------------------|----------------------------------|--------------|---------------|---------------|------------------|----------------------------------|--------------|--------------|---------------|
| | | 2-Yr | 10-Yr | 25-Yr | 100-Yr | | 2-Yr | 10-Yr | 25-Yr | 100-Yr |
| Vernal Pool | EV1 | 2.78 | 25.27 | 45.93 | 134.84 | EV1 | 2.78 | 25.27 | 45.93 | 134.84 |
| Wetland | EV2 | 13.86 | 38.09 | 55.49 | 94.20 | EV2 | 5.69 | 8.55 | 16.02 | 59.18 |
| Off Site | EV3 | 0.90 | 4.87 | 8.29 | 16.56 | EV3 | 0.55 | 3.60 | 6.39 | 14.37 |
| TOTAL | | 17.54 | 68.23 | 109.71 | 245.60 | | 9.02 | 37.42 | 68.34 | 208.39 |

Existing Drainage Calculations



DRAINAGE LEGEND

1

1

1

SUBCATCHMENT AREA
REACH
POND
SOIL DELINEATION
TIME OF CONCENTRATION PATH
DRAINAGE AREA LIMIT

LEGEND

EDGE OF WETLAND

EXISTING CONTOUR

EXISTING TREE LINE

NOTES

1. ELEVATIONS REFER TO NAVD 88 VERTICAL DATUM.

2. HORIZONTAL DATUM: NAD 83

3. PARTIAL WETLAND RESOURCES FLAGGED BY GODDARD CONSULTING IN 2024. FLAGS WERE FIELD LOCATED BY ALLEN ENGINEERING & ASSOCIATES IN NOVEMBER 2024.

4. PROPERTY LINE AND TOPOGRAPHIC INFORMATION OBTAINED BY ALLEN ENGINEERING & ASSOCIATES, INC IN AUGUST, 2024.

5. UNDERGROUND UTILITIES SHOWN HAVE BEEN COMPILED FROM AVAILABLE PLANS FROM PUBLIC AND PRIVATE AGENCIES, OR AS INDICATED BY FIELD OBSERVATIONS AND ARE APPROXIMATE ONLY. ADDITIONAL UNDERGROUND UTILITIES OR STRUCTURES NOT SHOWN HEREON MIGHT EXIST. BEFORE ANY EXCAVATION OR CONSTRUCTION CALL DIG-SAFE AT 811.

6. TOPOGRAPHIC INFORMATION OBTAINED FROM LIDAR DATA ISSUED BY NOAA (2021).

OWNER:

Wall Street Development Corp.
P.O. Box 272
Westwood, MA 02090

TITLE:

EXISTING DRAINAGE PLAN
for
Blackstone Street Improvements
Bellingham, MA

SEAL:

PROFESSIONAL ENGINEER

PREPARED BY:

ALLEN ENGINEERING & ASSOCIATES, INC.
Civil Engineers • Surveyors
Land Development Consultants
140 Hartford Avenue East
Hopedale, Ma 01747
(508) 381-3212 • www.allen-ea.com

SCALE:

1" = 150 FEET

0

150

300

450

DATE:

Logpwt{ #: #247

REVISIONS

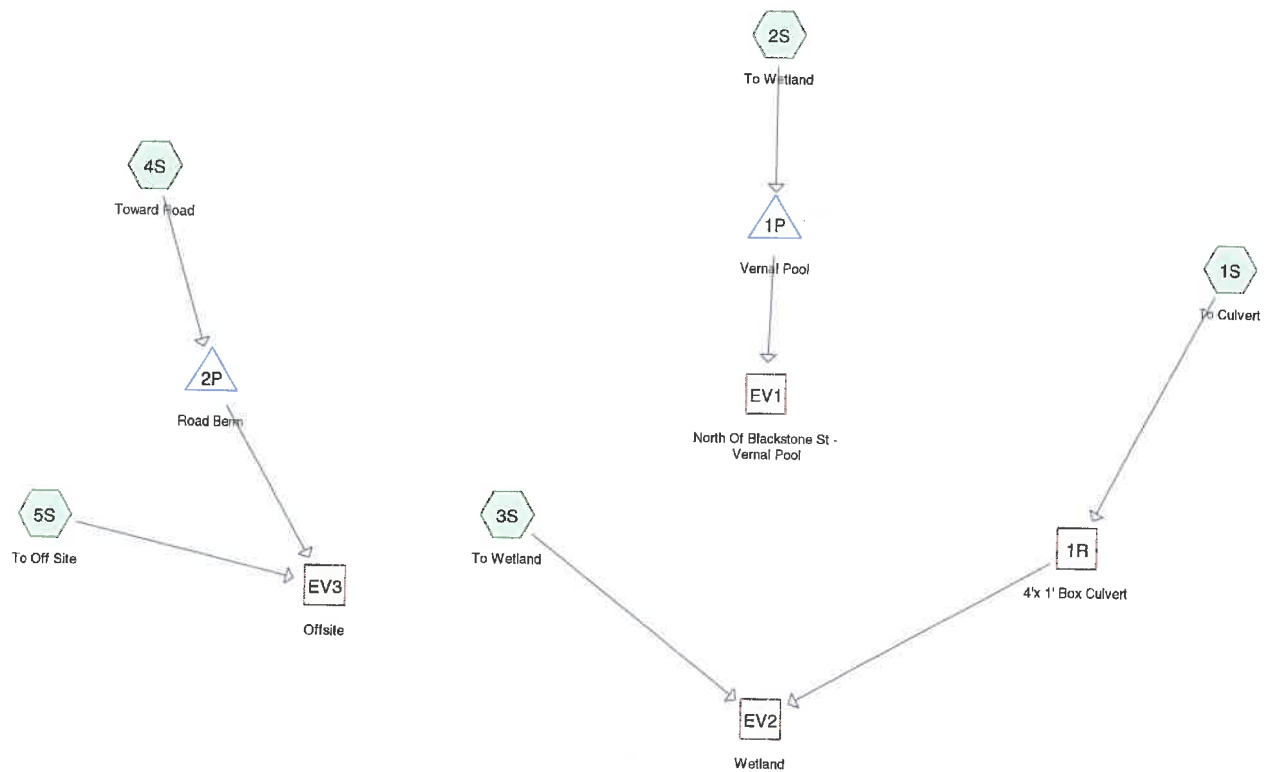
| # | DATE | DESCRIPTION | INIT |
|---|----------|----------------------|------|
| 1 | 6-20-25 | ROADWAY AND DRAINAGE | MEA |
| 2 | 10-31-25 | PER PEER REVIEW | MEA |

JOB NO:

00454

SHEET:

1 of 1



Routing Diagram for 00454-EX - Rev1

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Area Listing (all nodes)

| Area (sq-ft) | CN | Description (subcatchment-numbers) |
|------------------|-----------|--|
| 409,337 | 61 | >75% Grass cover, Good, HSG B (1S, 2S, 3S) |
| 20,077 | 80 | >75% Grass cover, Good, HSG D (2S) |
| 5,248 | 96 | Gravel surface, HSG B (2S, 5S) |
| 7,421 | 96 | Gravel surface, HSG D (2S) |
| 92,735 | 98 | Paved roads w/curbs & sewers, HSG B (1S, 2S) |
| 44,336 | 98 | Roofs, HSG B (1S, 2S, 3S) |
| 3,202 | 98 | Roofs, HSG D (2S) |
| 3,275,715 | 55 | Woods, Good, HSG B (1S, 2S, 3S, 4S, 5S) |
| 793,923 | 77 | Woods, Good, HSG D (2S, 3S) |
| 4,651,994 | 61 | TOTAL AREA |

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00454 - EXISTING Conditions
Type III 24-hr 2 YR Rainfall=3.38"

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Summary for Subcatchment 1S: To Culvert

Runoff = 2.90 cfs @ 12.51 hrs, Volume= 21,338 cf, Depth= 0.52"

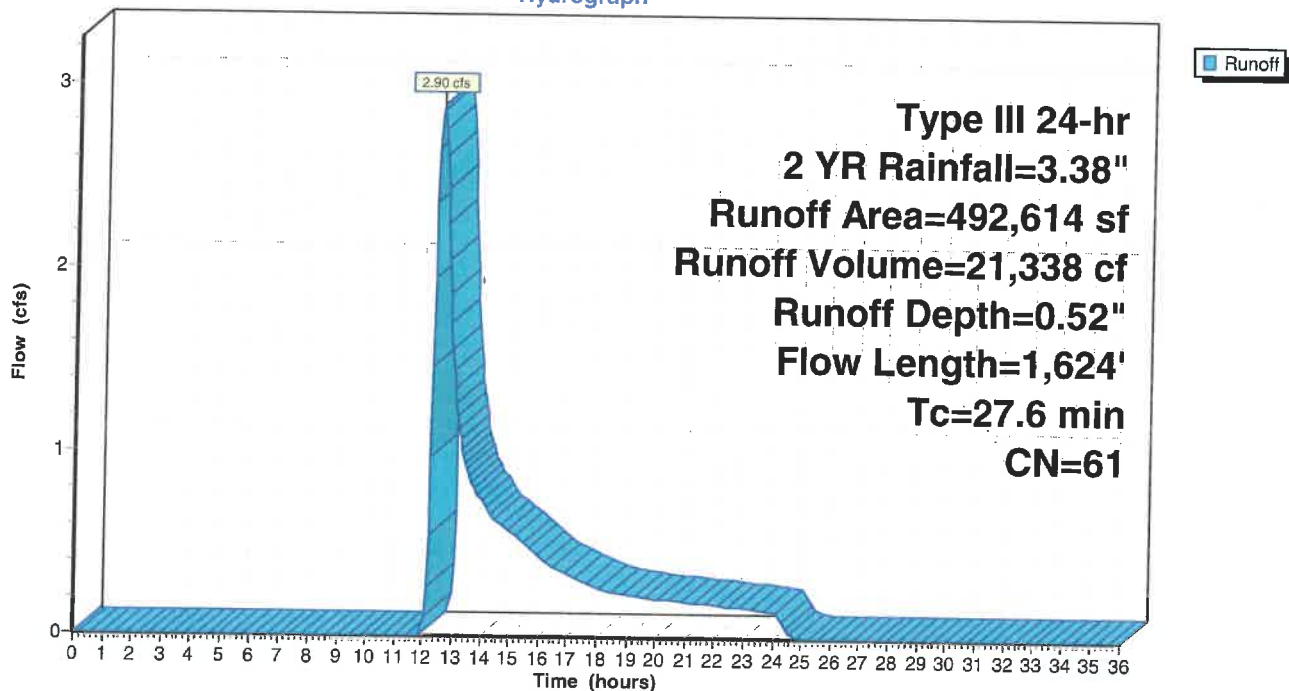
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, |
| | | | | | Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

Subcatchment 1S: To Culvert

Hydrograph



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Summary for Subcatchment 2S: To Wetland

Runoff = 9.98 cfs @ 12.81 hrs, Volume= 101,720 cf, Depth= 0.44"

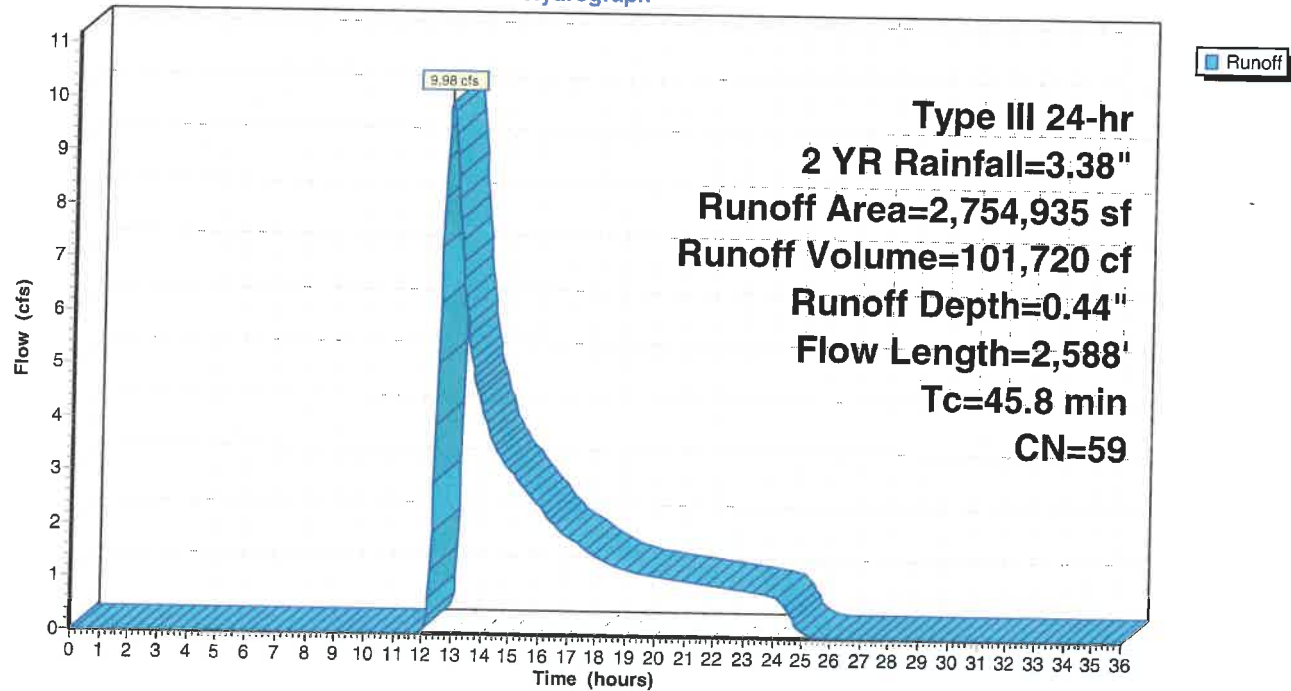
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow |
| | | | | | Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 45.8 | 2,588 | Total | | | |

Subcatchment 2S: To Wetland

Hydrograph



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Summary for Subcatchment 3S: To Wetland

Runoff = 11.02 cfs @ 12.45 hrs, Volume= 64,434 cf, Depth= 0.99"

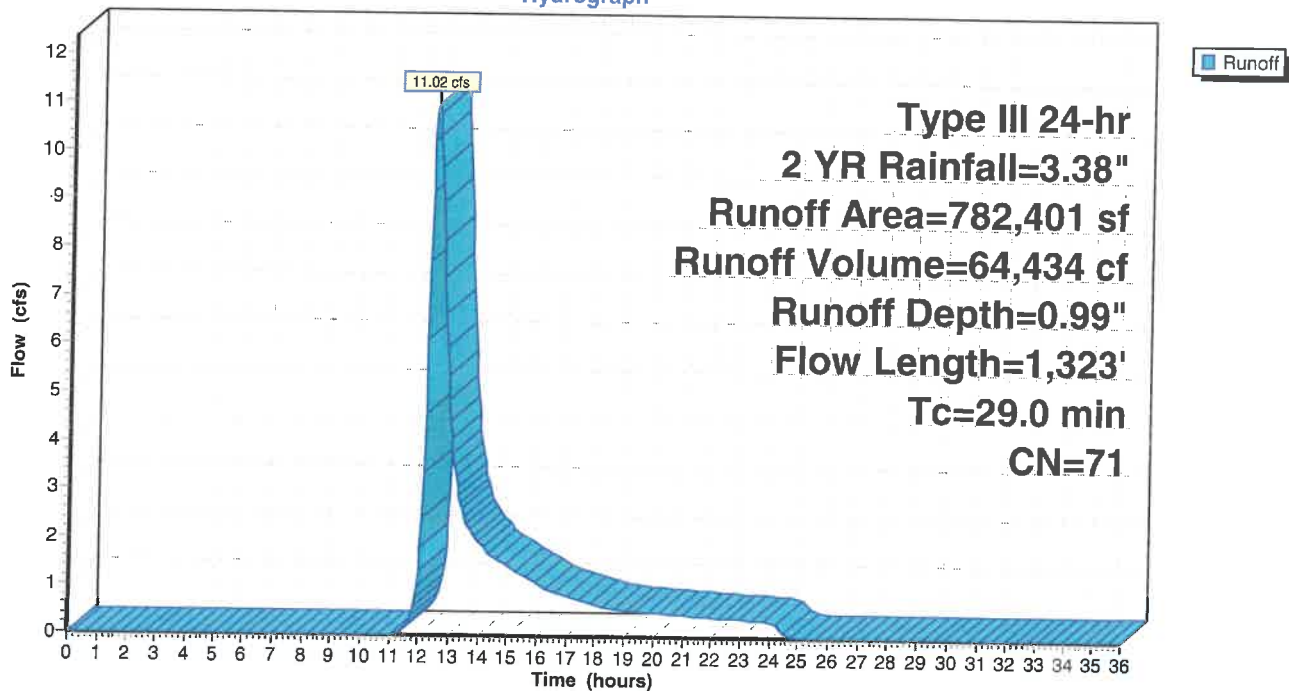
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 205,649 | 55 | Woods, Good, HSG B |
| 557,046 | 77 | Woods, Good, HSG D |
| 4,470 | 98 | Roofs, HSG B |
| 15,236 | 61 | >75% Grass cover, Good, HSG B |
| 782,401 | 71 | Weighted Average |
| 777,931 | | 99.43% Pervious Area |
| 4,470 | | 0.57% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 4.5 | 50 | 0.0350 | 0.19 | | Sheet Flow, Sheet |
| | | | | | Grass: Short n= 0.150 P2= 3.22" |
| 3.1 | 190 | 0.0210 | 1.01 | | Shallow Concentrated Flow, Shallow |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 16.2 | 735 | 0.0230 | 0.76 | | Shallow Concentrated Flow, c-d |
| | | | | | Woodland Kv= 5.0 fps |
| 5.2 | 348 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 29.0 | 1,323 | Total | | | |

Subcatchment 3S: To Wetland

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Type III 24-hr 2 YR Rainfall=3.38"

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Summary for Subcatchment 4S: Toward Road

Runoff = 0.89 cfs @ 12.55 hrs, Volume= 8,569 cf, Depth= 0.31"

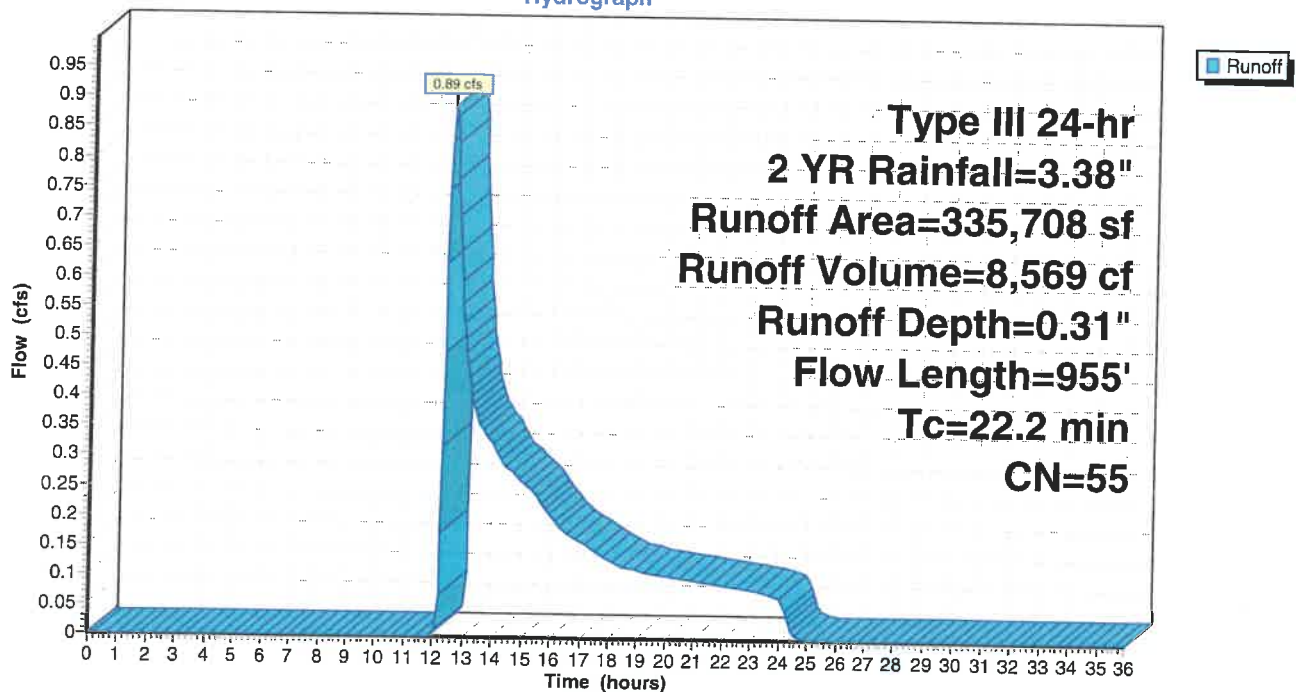
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 335,708 | 55 | Woods, Good, HSG B |
| 335,708 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, |
| 15.1 | 905 | 0.0401 | 1.00 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 955 | Total | | | |

Subcatchment 4S: Toward Road

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Summary for Subcatchment 5S: To Off Site

Runoff = 0.90 cfs @ 12.52 hrs, Volume= 8,075 cf, Depth= 0.34"

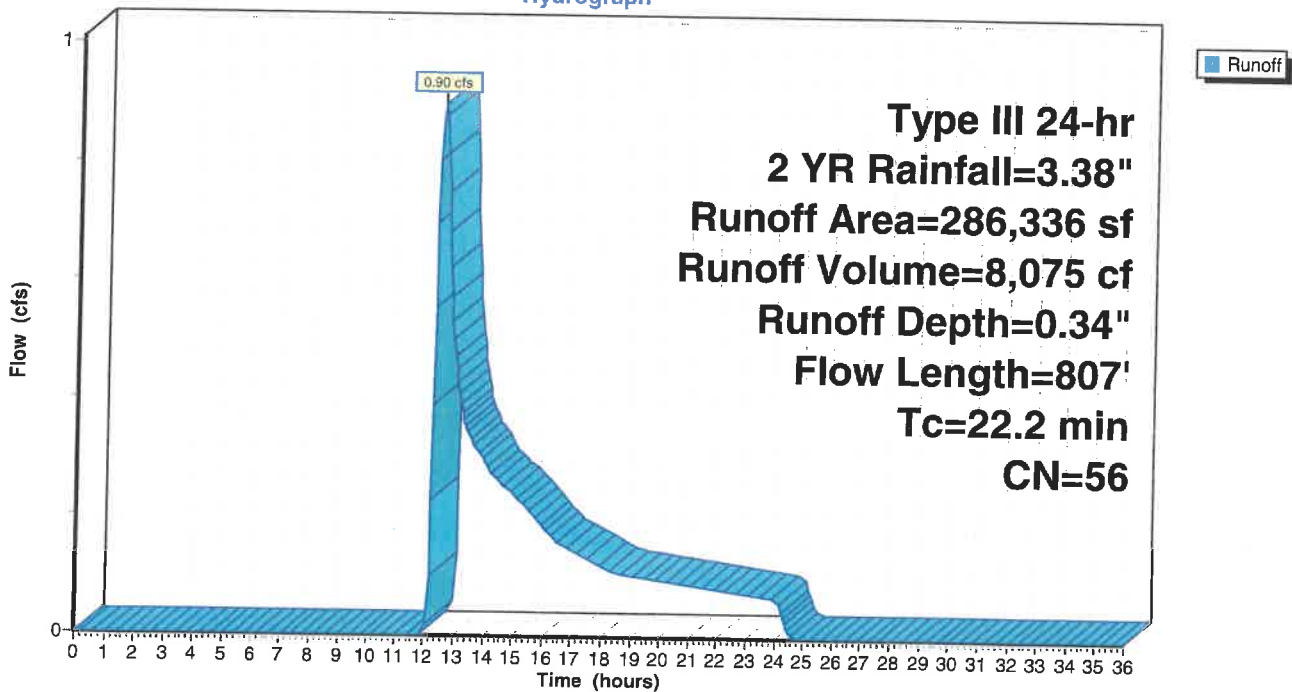
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 4,366 | 96 | Gravel surface, HSG B |
| 281,970 | 55 | Woods, Good, HSG B |
| 286,336 | 56 | Weighted Average |
| 286,336 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 10.3 | 50 | 0.0312 | 0.08 | | Sheet Flow, a-b |
| 11.9 | 757 | 0.0449 | 1.06 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, b-c |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 807 | Total | | | |

Subcatchment 5S: To Off Site

Hydrograph



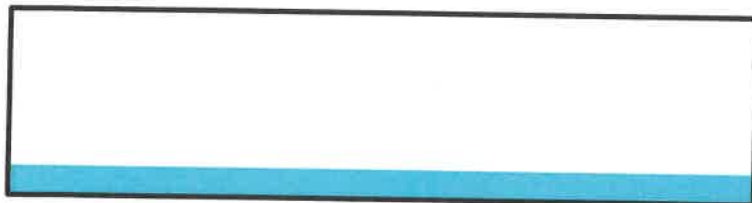
Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth = 0.52" for 2 YR event
 Inflow = 2.90 cfs @ 12.51 hrs, Volume= 21,338 cf
 Outflow = 2.90 cfs @ 12.51 hrs, Volume= 21,338 cf, Atten= 0%, Lag= 0.4 min

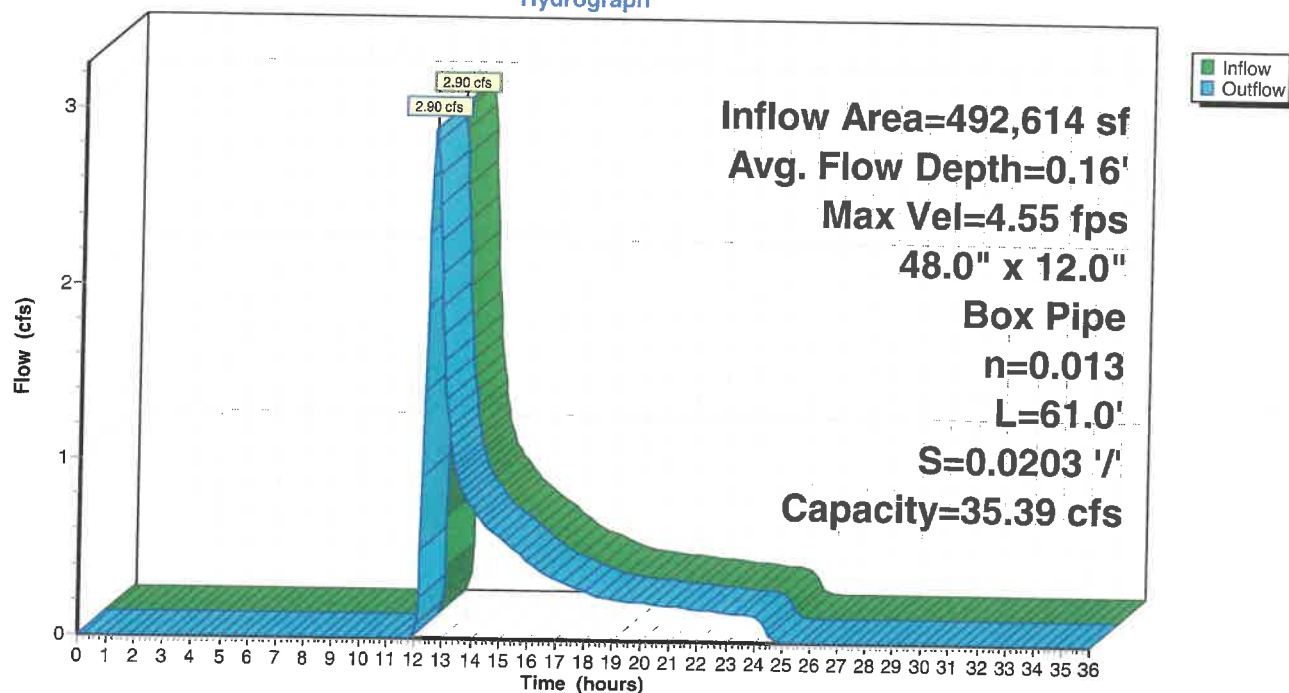
Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Max. Velocity= 4.55 fps, Min. Travel Time= 0.2 min
 Avg. Velocity= 1.95 fps, Avg. Travel Time= 0.5 min

Peak Storage= 39 cf @ 12.51 hrs
 Average Depth at Peak Storage= 0.16', Surface Width= 4.00'
 Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe
 n= 0.013 Concrete, trowel finish
 Length= 61.0' Slope= 0.0203 '/'
 Inlet Invert= 301.34', Outlet Invert= 300.10'

**Reach 1R: 4'x 1' Box Culvert**

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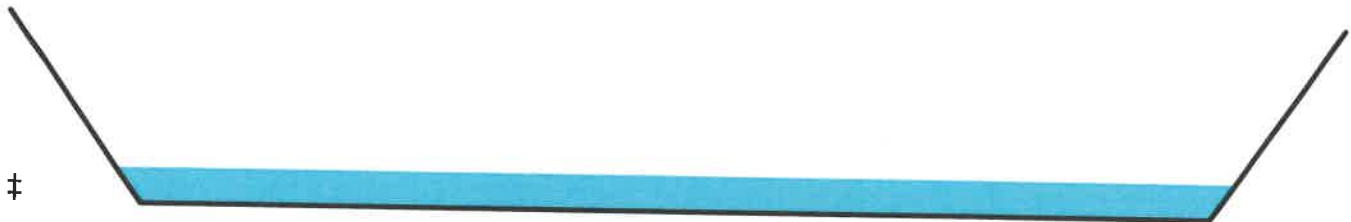
Summary for Reach EV1: North Of Blackstone St - Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 0.27" for 2 YR event
Inflow = 2.78 cfs @ 15.43 hrs, Volume= 62,728 cf
Outflow = 2.78 cfs @ 15.44 hrs, Volume= 62,728 cf, Atten= 0%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.46 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 0.71 fps, Avg. Travel Time= 0.2 min

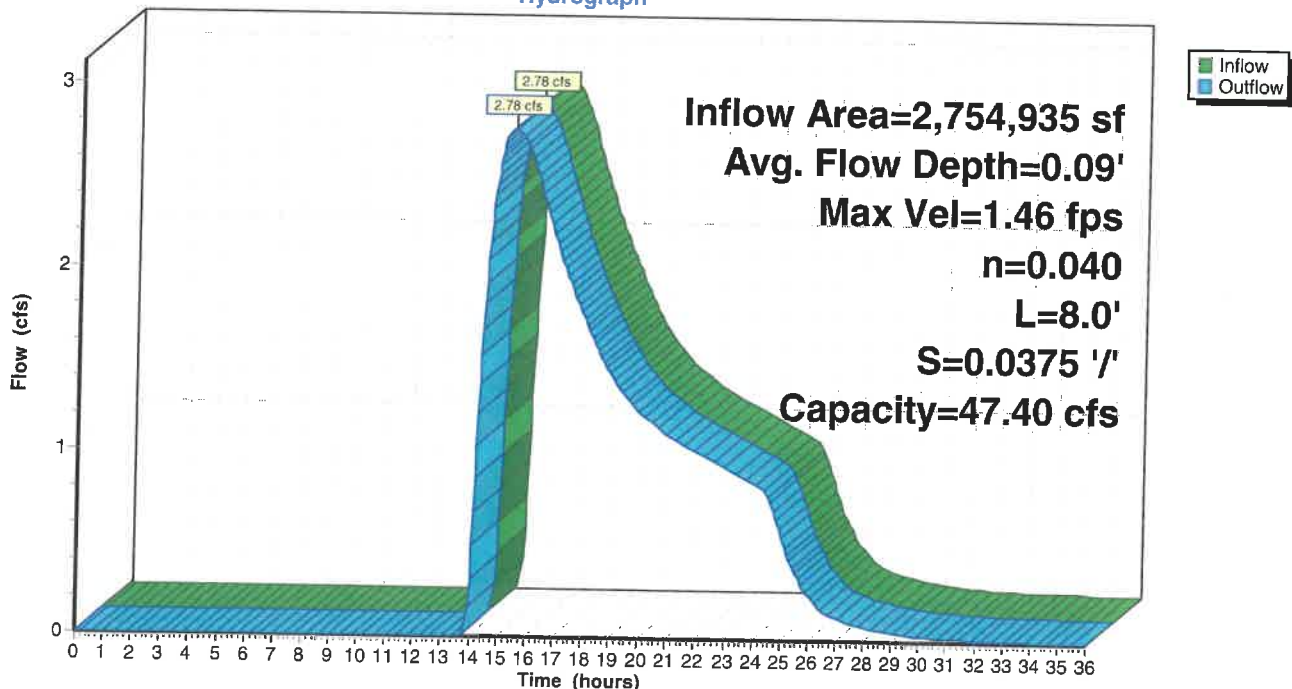
Peak Storage= 15 cf @ 15.44 hrs
Average Depth at Peak Storage= 0.09' , Surface Width= 20.93'
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides
Side Slope Z-value= 5.0 '/' Top Width= 25.00'
Length= 8.0' Slope= 0.0375 '/'
Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: North Of Blackstone St - Vernal Pool

Hydrograph



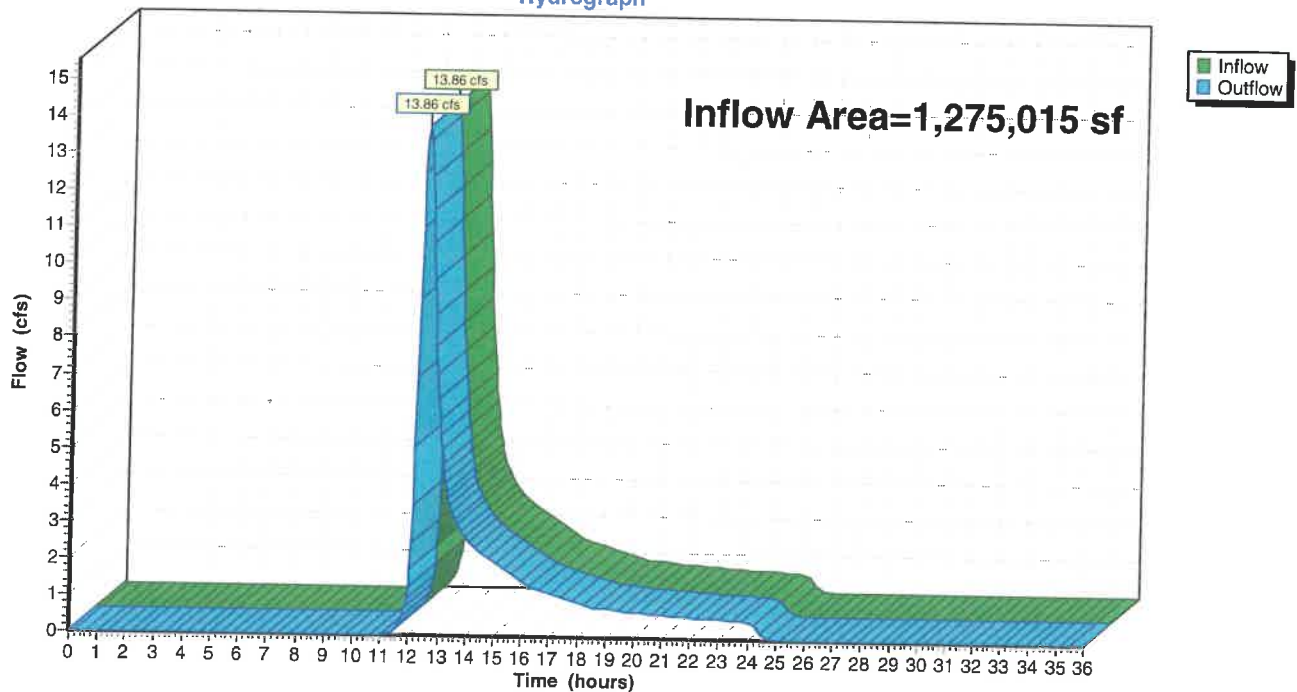
Summary for Reach EV2: Wetland

Inflow Area = 1,275,015 sf, 4.20% Impervious, Inflow Depth = 0.81" for 2 YR event
Inflow = 13.86 cfs @ 12.46 hrs, Volume= 85,772 cf
Outflow = 13.86 cfs @ 12.46 hrs, Volume= 85,772 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV2: Wetland

Hydrograph



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Type III 24-hr 2 YR Rainfall=3.38"

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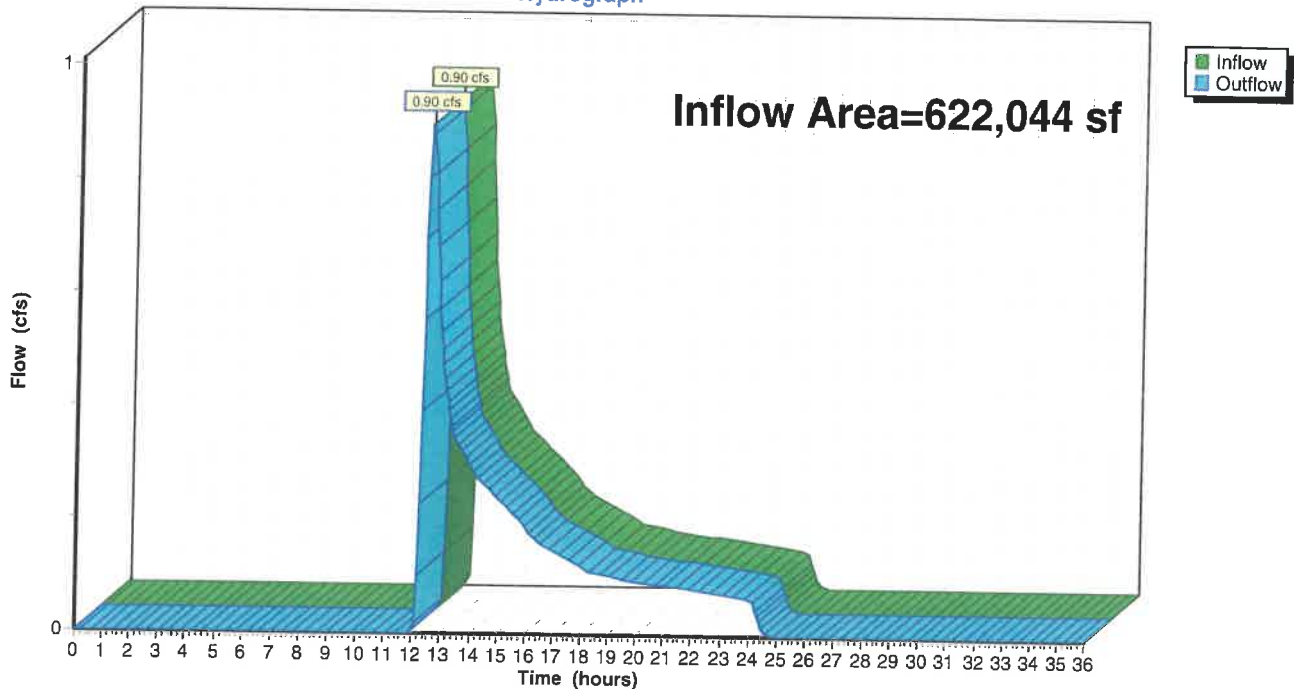
Summary for Reach EV3: Offsite

Inflow Area = 622,044 sf, 0.00% Impervious, Inflow Depth = 0.16" for 2 YR event
Inflow = 0.90 cfs @ 12.52 hrs, Volume= 8,075 cf
Outflow = 0.90 cfs @ 12.52 hrs, Volume= 8,075 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV3: Offsite

Hydrograph



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Type III 24-hr 2 YR Rainfall=3.38"

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Summary for Pond 1P: Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth = 0.44" for 2 YR event
 Inflow = 9.98 cfs @ 12.81 hrs, Volume= 101,720 cf
 Outflow = 2.78 cfs @ 15.43 hrs, Volume= 62,728 cf, Atten= 72%, Lag= 157.3 min
 Primary = 2.78 cfs @ 15.43 hrs, Volume= 62,728 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 294.07' @ 15.43 hrs Surf.Area= 33,327 sf Storage= 47,404 cf

Plug-Flow detention time= 302.0 min calculated for 62,641 cf (62% of inflow)
 Center-of-Mass det. time= 169.8 min (1,125.3 - 955.5)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 291.30' | 143,915 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

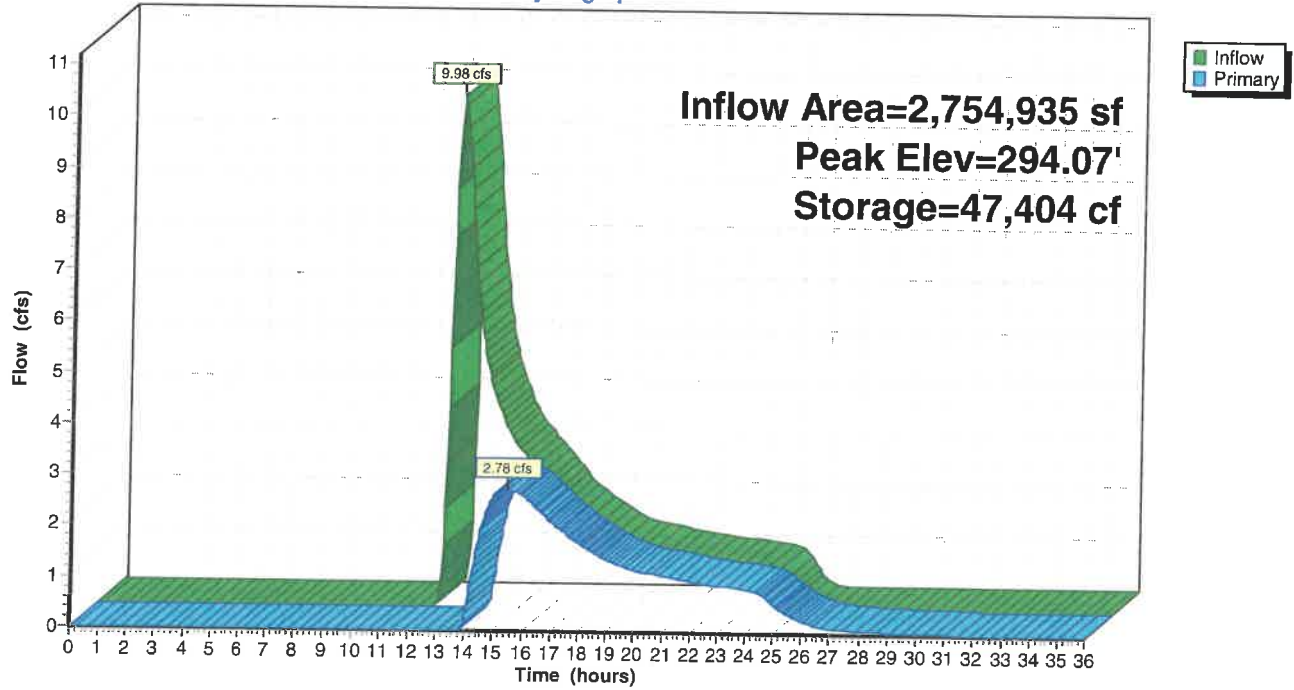
| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=2.77 cfs @ 15.43 hrs HW=294.07' (Free Discharge)
 ↑ **1=Broad-Crested Rectangular Weir** (Weir Controls 2.77 cfs @ 1.30 fps)

Pond 1P: Vernal Pool

Hydrograph



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Type III 24-hr 2 YR Rainfall=3.38"

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Summary for Pond 2P: Road Berm

Inflow Area = 335,708 sf, 0.00% Impervious, Inflow Depth = 0.31" for 2 YR event
 Inflow = 0.89 cfs @ 12.55 hrs, Volume= 8,569 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 310.48' @ 25.30 hrs Surf.Area= 24,353 sf Storage= 8,567 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 310.00' | 256,496 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

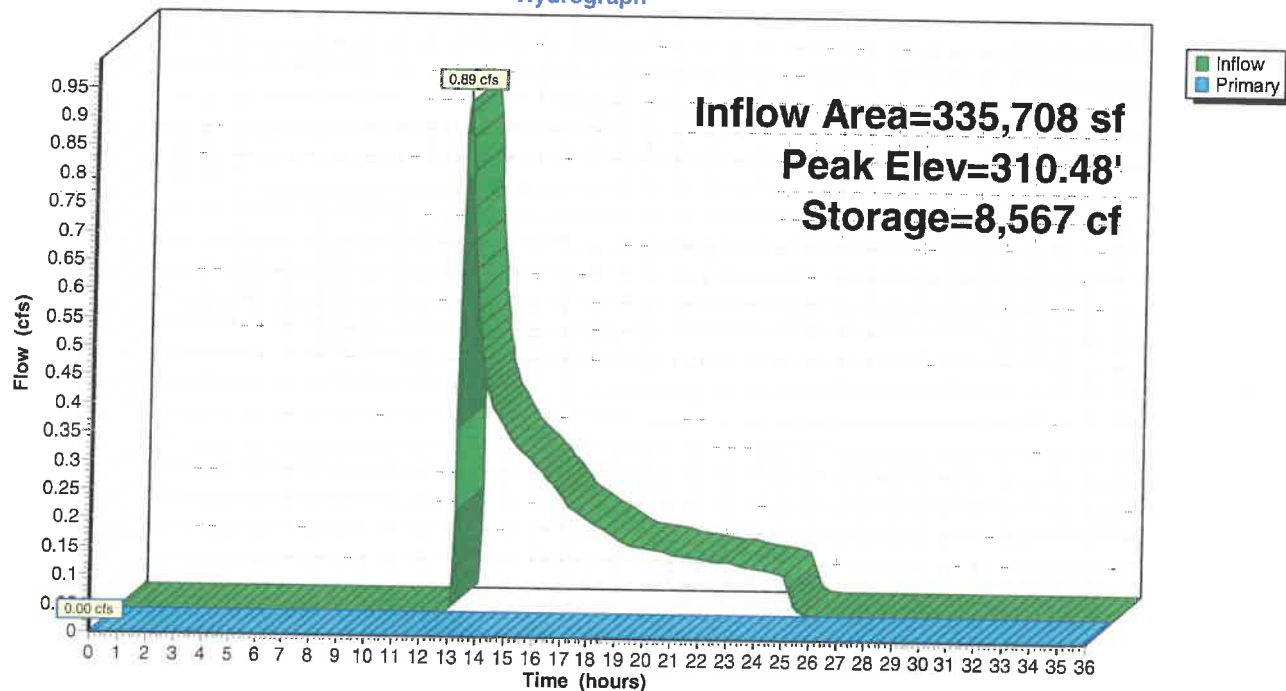
| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 313.50' | 8.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63 |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 2P: Road Berm

Hydrograph



Summary for Subcatchment 1S: To Culvert

Runoff = 10.78 cfs @ 12.43 hrs, Volume= 61,957 cf, Depth= 1.51"

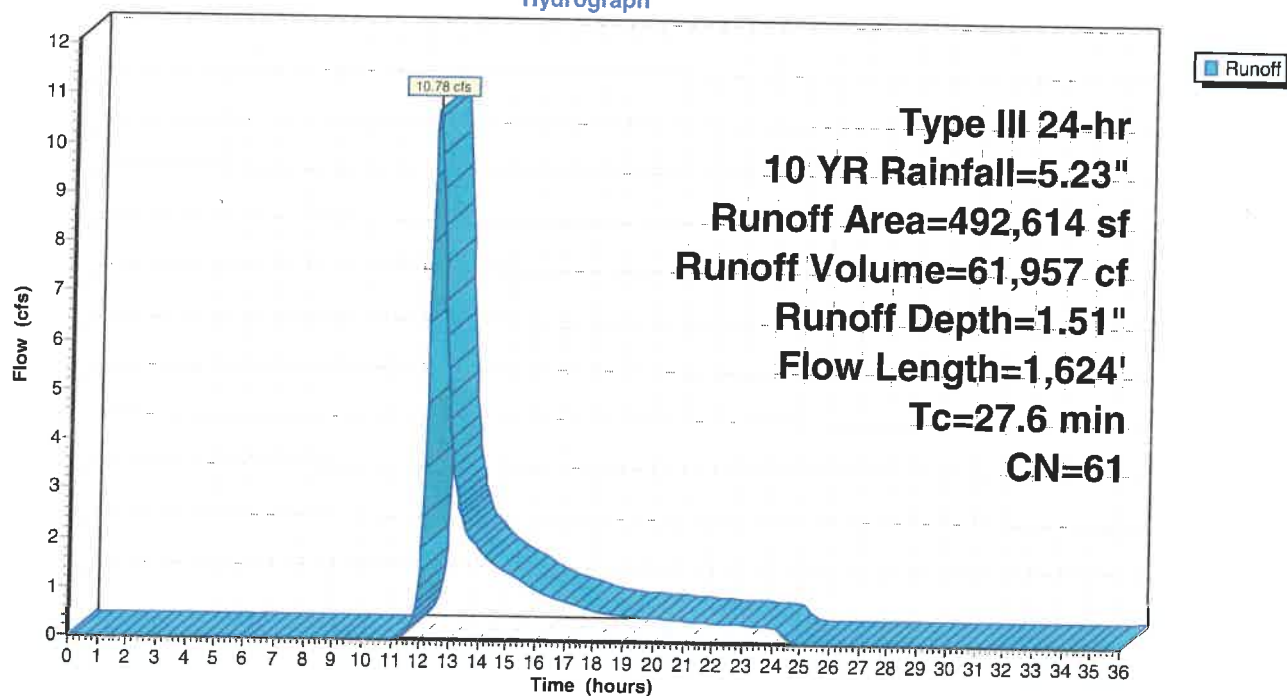
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, |
| | | | | | Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

Subcatchment 1S: To Culvert

Hydrograph



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Type III 24-hr 10 YR Rainfall=5.23"

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Summary for Subcatchment 2S: To Wetland

Runoff = 41.58 cfs @ 12.70 hrs, Volume= 313,788 cf, Depth= 1.37"

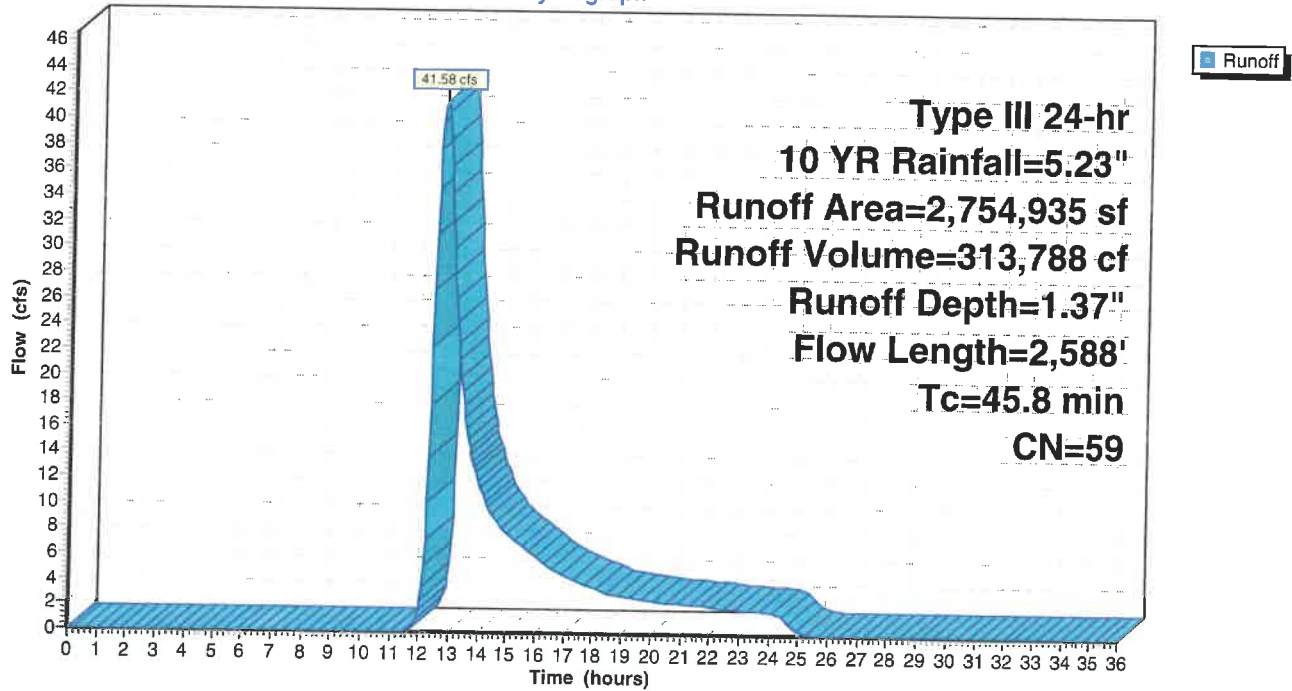
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow |
| | | | | | Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 45.8 | 2,588 | Total | | | |

Subcatchment 2S: To Wetland

Hydrograph



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Type III 24-hr 10 YR Rainfall=5.23"

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Summary for Subcatchment 3S: To Wetland

Runoff = 27.32 cfs @ 12.42 hrs, Volume= 149,430 cf, Depth= 2.29"

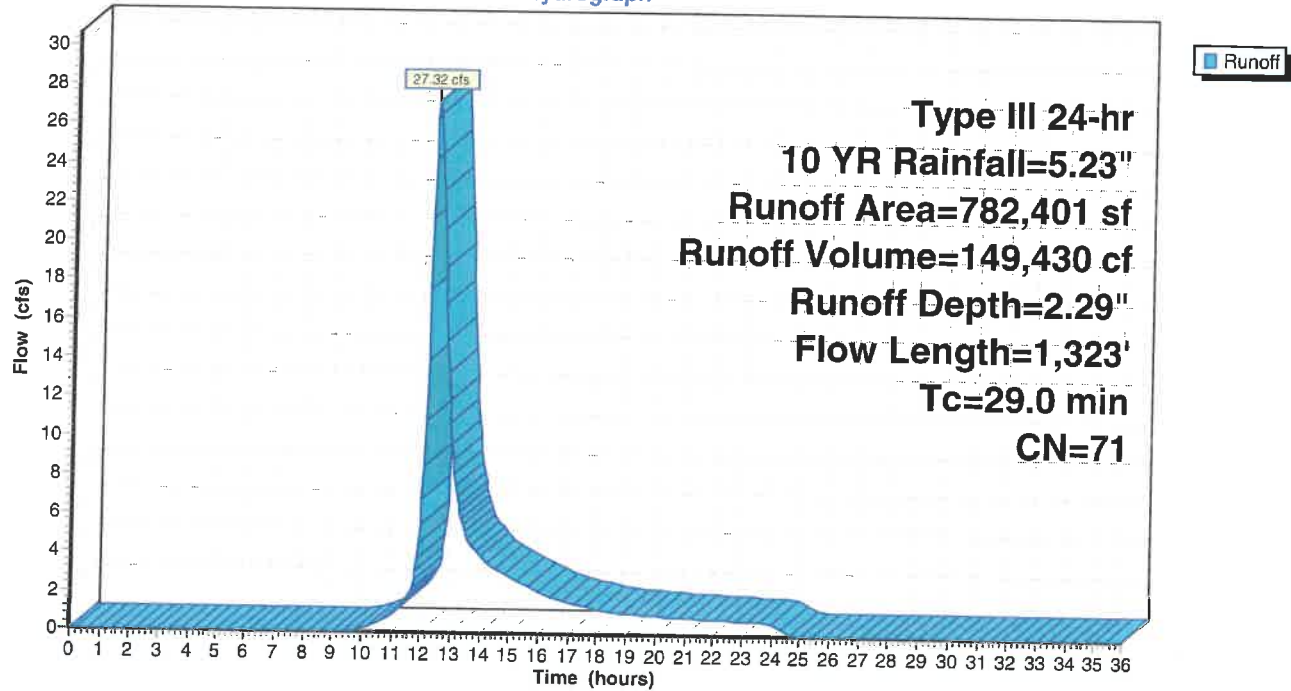
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 205,649 | 55 | Woods, Good, HSG B |
| 557,046 | 77 | Woods, Good, HSG D |
| 4,470 | 98 | Roofs, HSG B |
| 15,236 | 61 | >75% Grass cover, Good, HSG B |
| 782,401 | 71 | Weighted Average |
| 777,931 | | 99.43% Pervious Area |
| 4,470 | | 0.57% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 4.5 | 50 | 0.0350 | 0.19 | | Sheet Flow, Sheet |
| | | | | | Grass: Short n= 0.150 P2= 3.22" |
| 3.1 | 190 | 0.0210 | 1.01 | | Shallow Concentrated Flow, Shallow |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 16.2 | 735 | 0.0230 | 0.76 | | Shallow Concentrated Flow, c-d |
| | | | | | Woodland Kv= 5.0 fps |
| 5.2 | 348 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 29.0 | 1,323 | Total | | | |

Subcatchment 3S: To Wetland

Hydrograph



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Summary for Subcatchment 4S: Toward Road

Runoff = 5.27 cfs @ 12.37 hrs, Volume= 30,681 cf, Depth= 1.10"

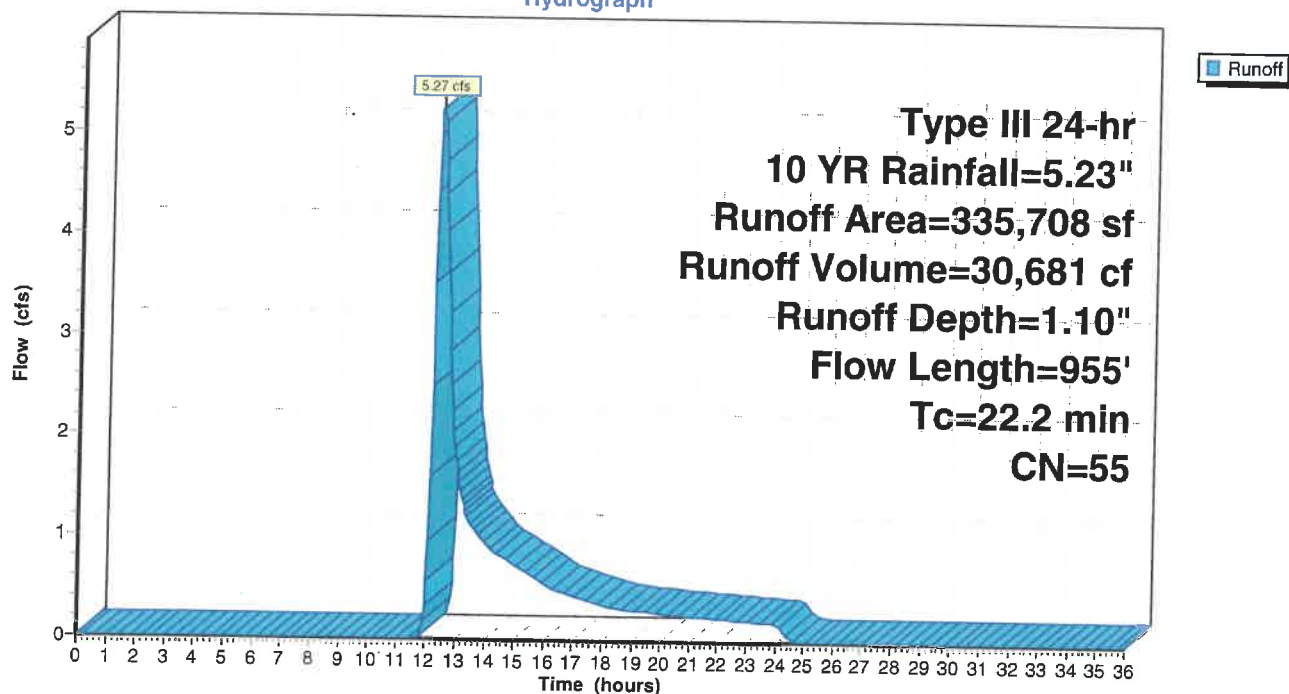
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 335,708 | 55 | Woods, Good, HSG B |
| 335,708 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, |
| 15.1 | 905 | 0.0401 | 1.00 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 955 | Total | | | |

Subcatchment 4S: Toward Road

Hydrograph



Summary for Subcatchment 5S: To Off Site

Runoff = 4.87 cfs @ 12.37 hrs, Volume= 27,735 cf, Depth= 1.16"

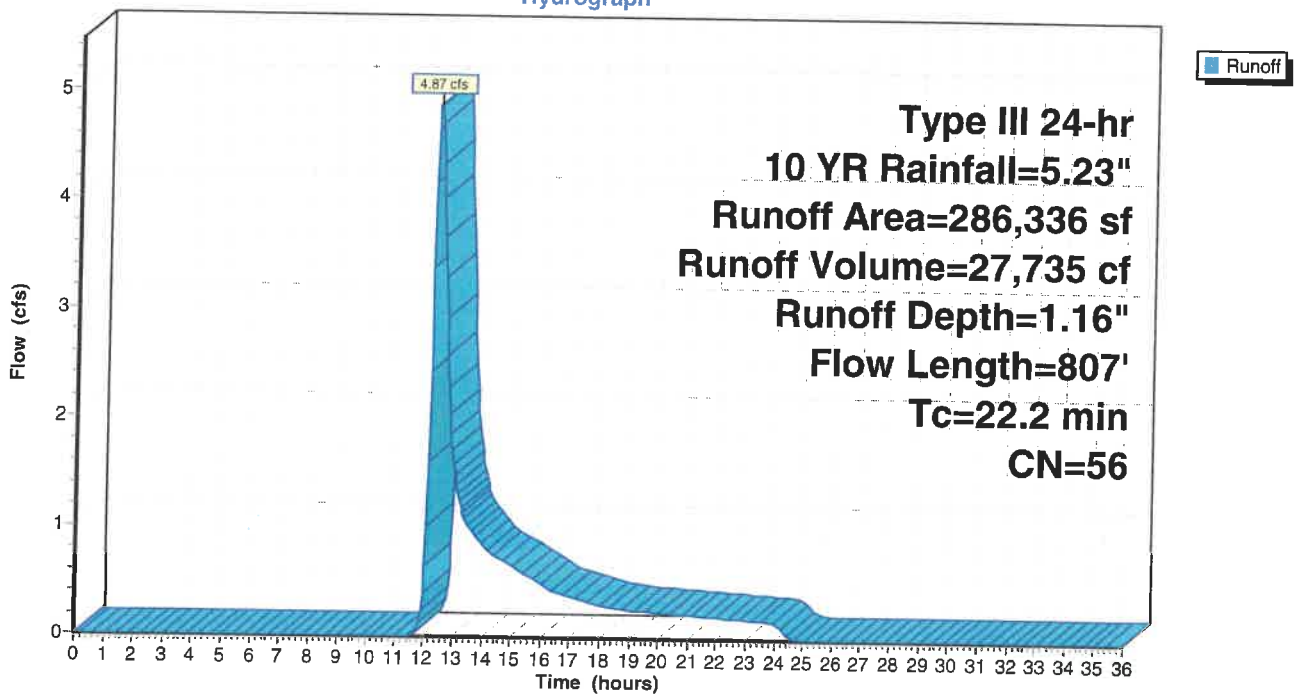
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 4,366 | 96 | Gravel surface, HSG B |
| 281,970 | 55 | Woods, Good, HSG B |
| 286,336 | 56 | Weighted Average |
| 286,336 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 10.3 | 50 | 0.0312 | 0.08 | | Sheet Flow, a-b |
| 11.9 | 757 | 0.0449 | 1.06 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, b-c |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 807 | Total | | | |

Subcatchment 5S: To Off Site

Hydrograph



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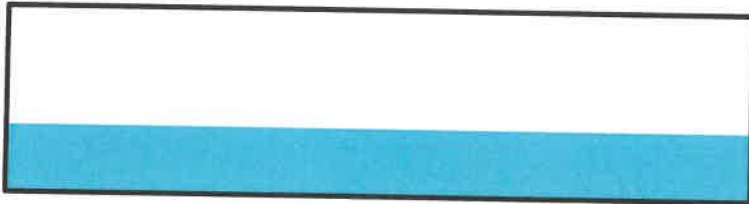
Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth = 1.51" for 10 YR event
Inflow = 10.78 cfs @ 12.43 hrs, Volume= 61,957 cf
Outflow = 10.77 cfs @ 12.43 hrs, Volume= 61,957 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.41 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 2.72 fps, Avg. Travel Time= 0.4 min

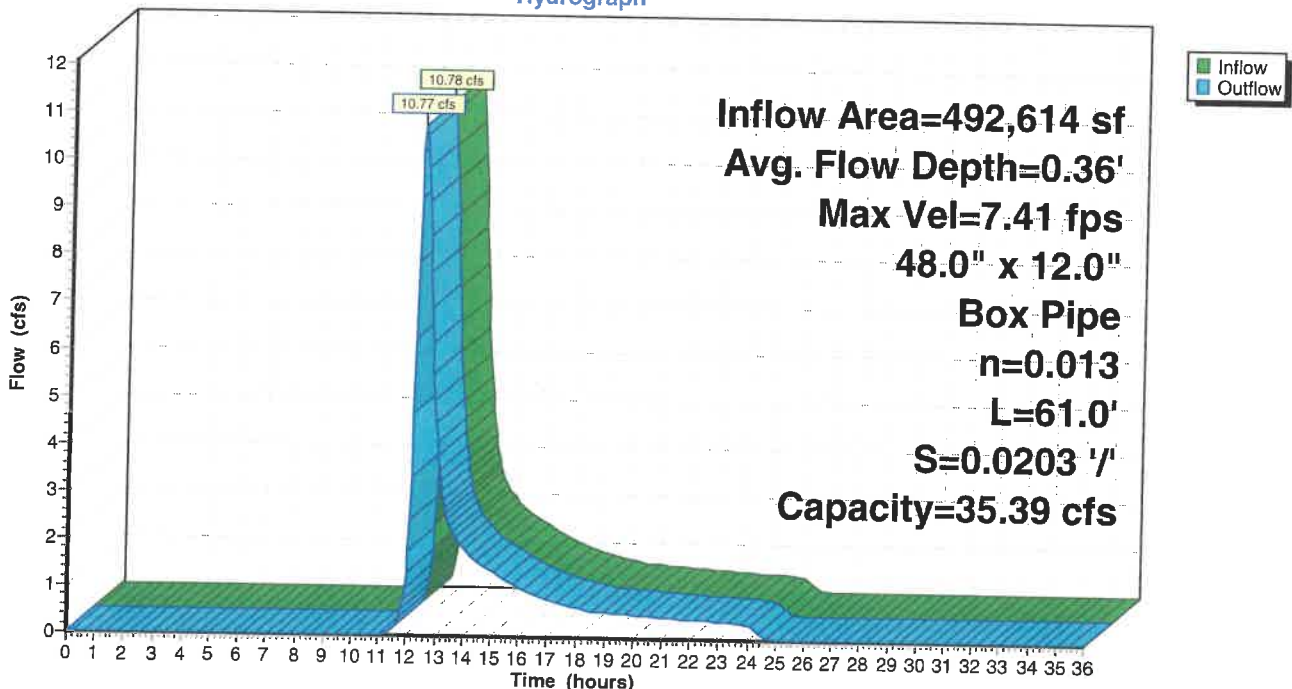
Peak Storage= 89 cf @ 12.43 hrs
Average Depth at Peak Storage= 0.36', Surface Width= 4.00'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe
n= 0.013 Concrete, trowel finish
Length= 61.0' Slope= 0.0203 '/'
Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

Hydrograph



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Summary for Reach EV1: North Of Blackstone St - Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 1.20" for 10 YR event
Inflow = 25.27 cfs @ 13.17 hrs, Volume= 274,792 cf
Outflow = 25.27 cfs @ 13.17 hrs, Volume= 274,791 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.36 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 1.12 fps, Avg. Travel Time= 0.1 min

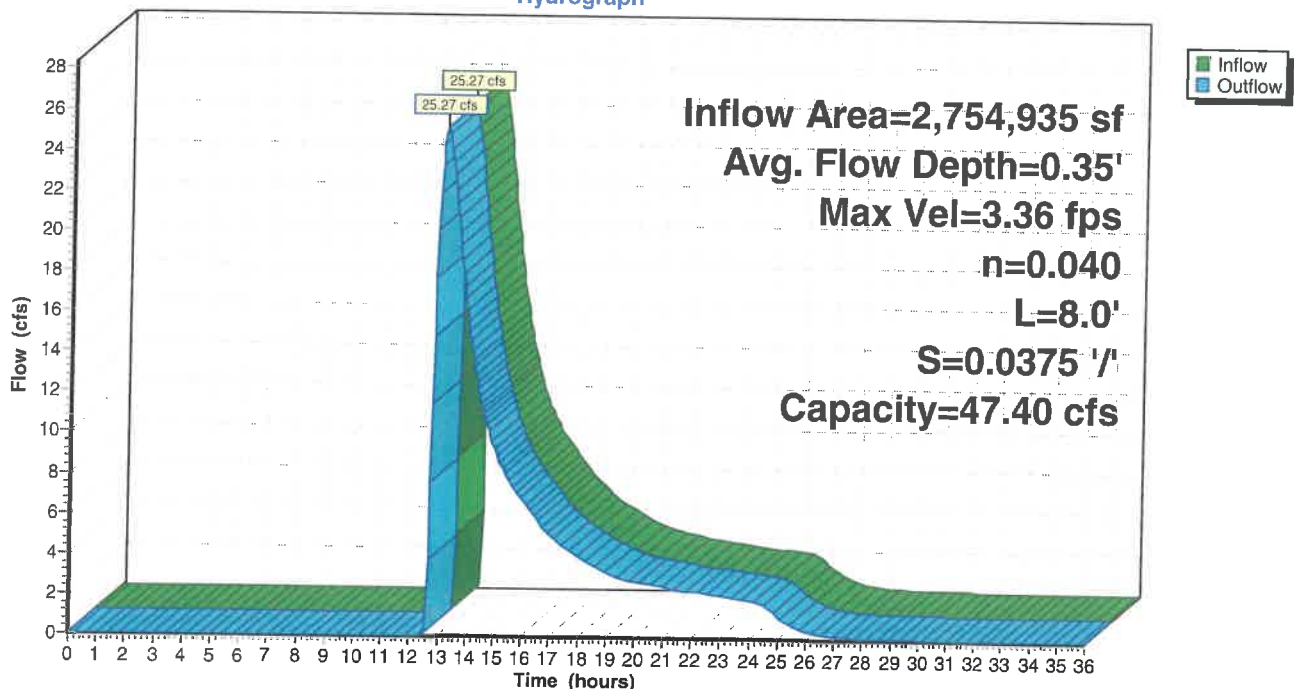
Peak Storage= 60 cf @ 13.17 hrs
Average Depth at Peak Storage= 0.35', Surface Width= 23.46'
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides
Side Slope Z-value= 5.0 '/' Top Width= 25.00'
Length= 8.0' Slope= 0.0375 '/'
Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: North Of Blackstone St - Vernal Pool

Hydrograph



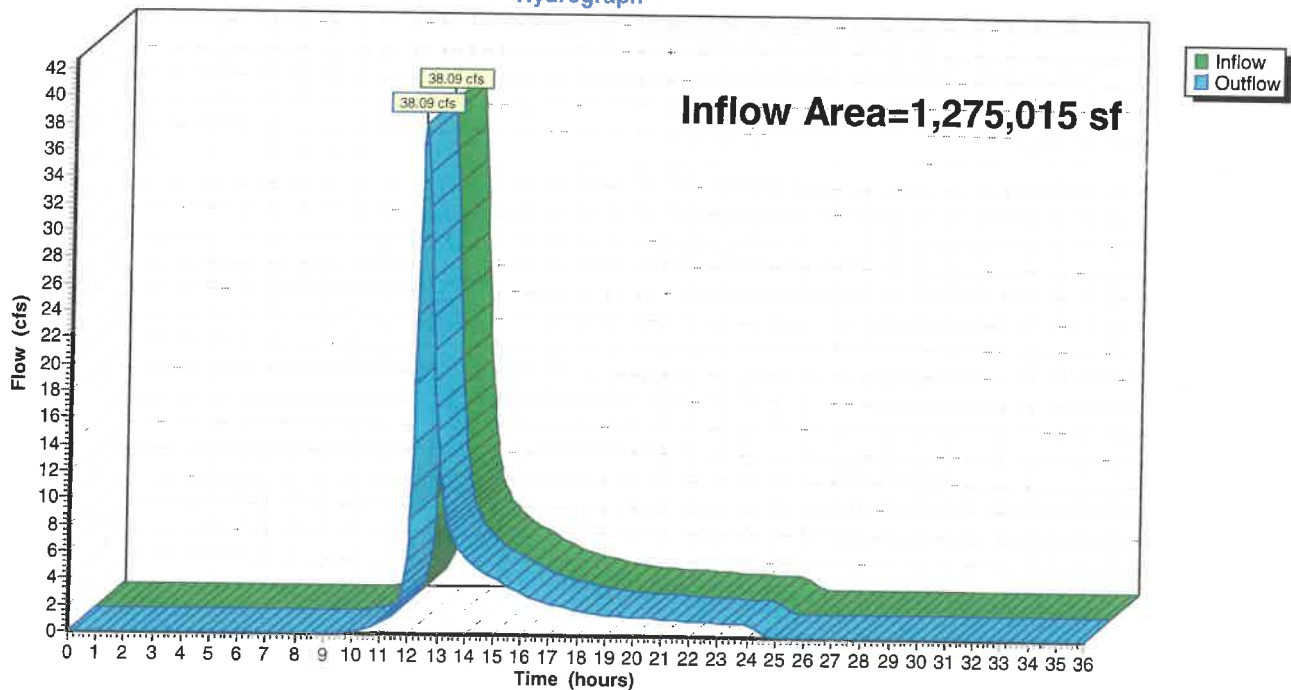
Summary for Reach EV2: Wetland

Inflow Area = 1,275,015 sf, 4.20% Impervious, Inflow Depth = 1.99" for 10 YR event
Inflow = 38.09 cfs @ 12.42 hrs, Volume= 211,387 cf
Outflow = 38.09 cfs @ 12.42 hrs, Volume= 211,387 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV2: Wetland

Hydrograph



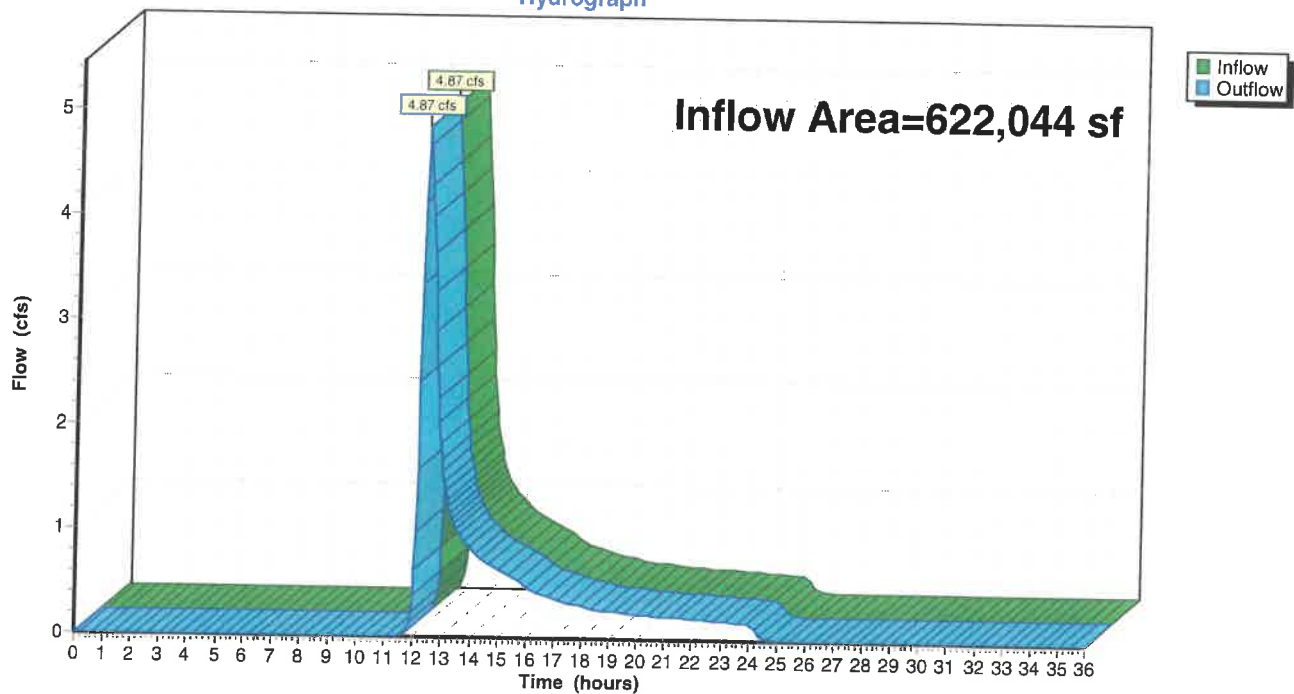
Summary for Reach EV3: Offsite

Inflow Area = 622,044 sf, 0.00% Impervious, Inflow Depth = 0.54" for 10 YR event
Inflow = 4.87 cfs @ 12.37 hrs, Volume= 27,735 cf
Outflow = 4.87 cfs @ 12.37 hrs, Volume= 27,735 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV3: Offsite

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Summary for Pond 1P: Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth = 1.37" for 10 YR event
 Inflow = 41.58 cfs @ 12.70 hrs, Volume= 313,788 cf
 Outflow = 25.27 cfs @ 13.17 hrs, Volume= 274,792 cf, Atten= 39%, Lag= 28.3 min
 Primary = 25.27 cfs @ 13.17 hrs, Volume= 274,792 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 294.91' @ 13.17 hrs Surf.Area= 45,996 sf Storage= 81,001 cf

Plug-Flow detention time= 120.7 min calculated for 274,792 cf (88% of inflow)
 Center-of-Mass det. time= 62.4 min (974.5 - 912.0)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 291.30' | 143,915 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

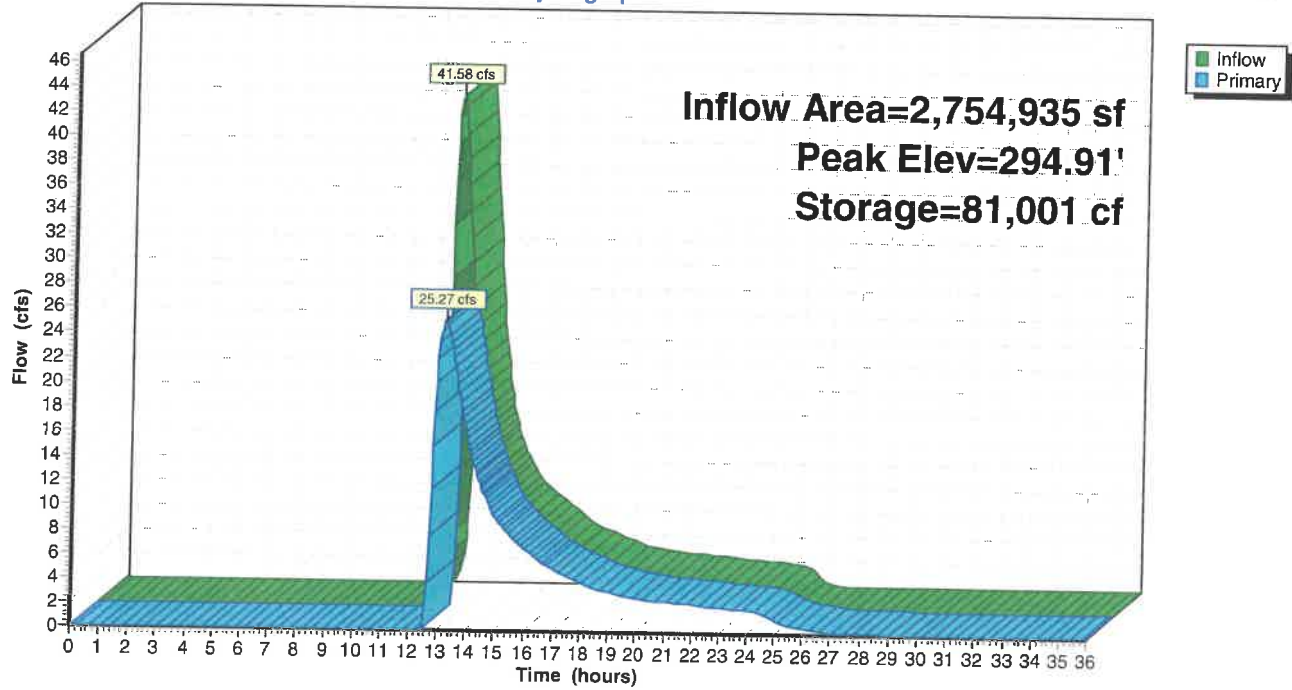
| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=25.23 cfs @ 13.17 hrs HW=294.91' (Free Discharge)
 ↗ **1=Broad-Crested Rectangular Weir** (Weir Controls 25.23 cfs @ 2.83 fps)

Pond 1P: Vernal Pool

Hydrograph



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Type III 24-hr 10 YR Rainfall=5.23"

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Summary for Pond 2P: Road Berm

Inflow Area = 335,708 sf, 0.00% Impervious, Inflow Depth = 1.10" for 10 YR event
 Inflow = 5.27 cfs @ 12.37 hrs, Volume= 30,681 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 311.15' @ 25.30 hrs Surf.Area= 42,547 sf Storage= 30,676 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 310.00' | 256,496 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

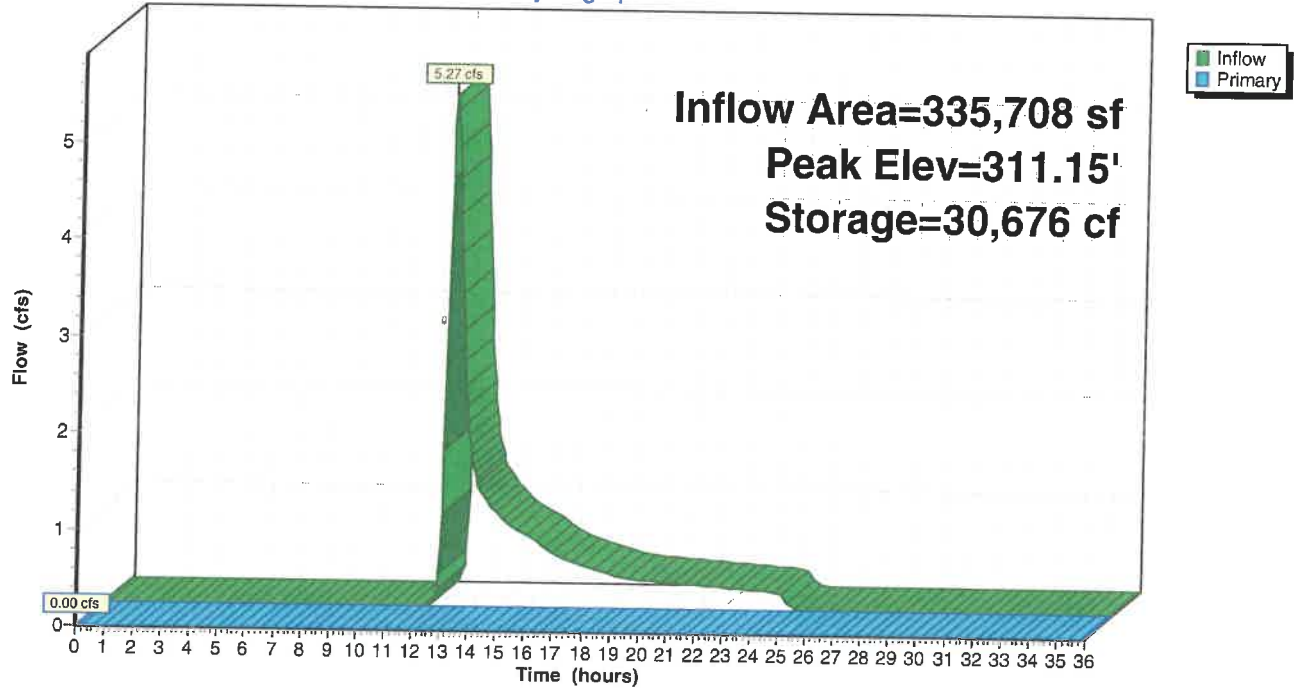
| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 313.50' | 8.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63 |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)
 ↑ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 2P: Road Berm

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 1S: To Culvert

Runoff = 16.88 cfs @ 12.41 hrs, Volume= 92,937 cf, Depth= 2.26"

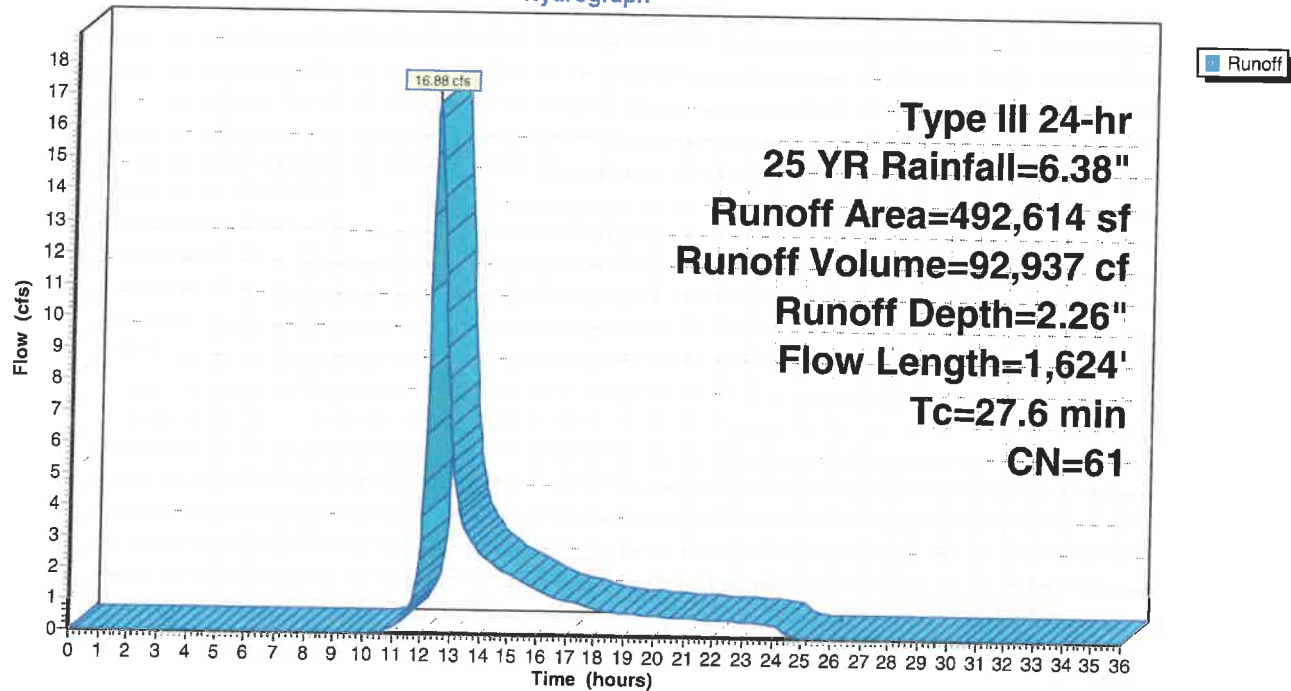
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, |
| | | | | | Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

Subcatchment 1S: To Culvert

Hydrograph



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Summary for Subcatchment 2S: To Wetland

Runoff = 66.94 cfs @ 12.68 hrs, Volume= 478,830 cf, Depth= 2.09"

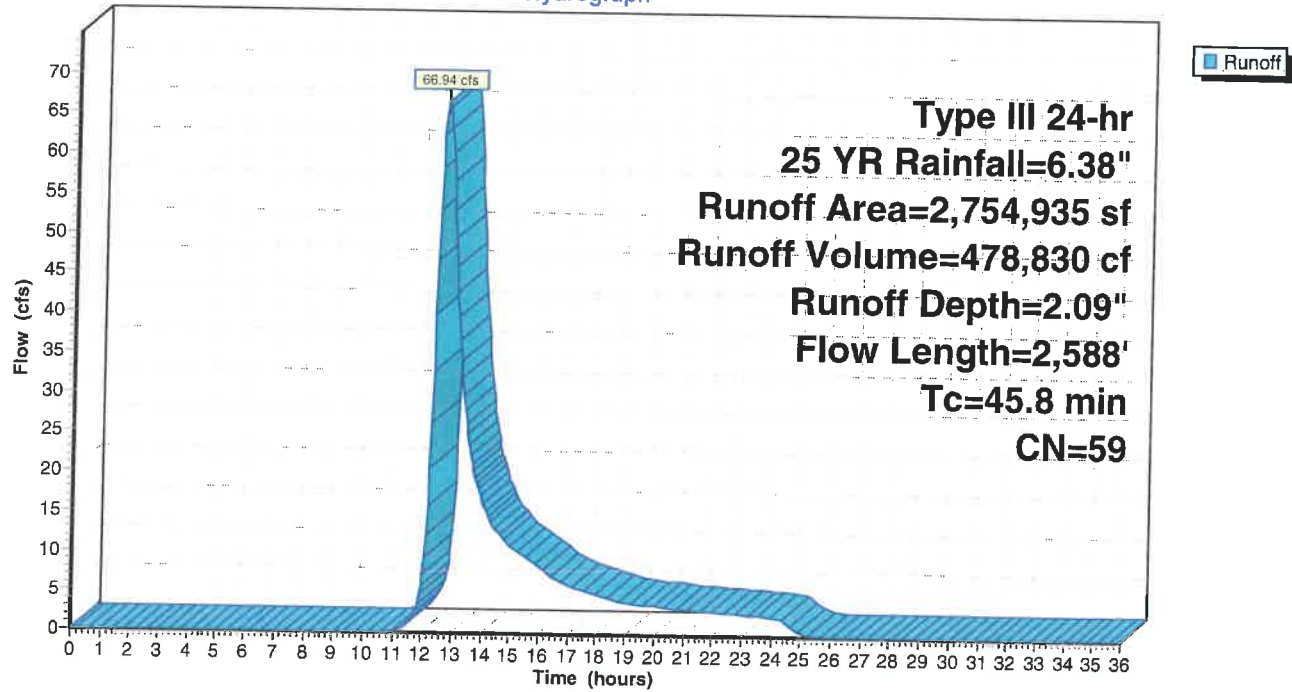
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow |
| | | | | | Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 45.8 | 2,588 | Total | | | |

Subcatchment 2S: To Wetland

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 3S: To Wetland

Runoff = 38.63 cfs @ 12.41 hrs, Volume= 209,152 cf, Depth= 3.21"

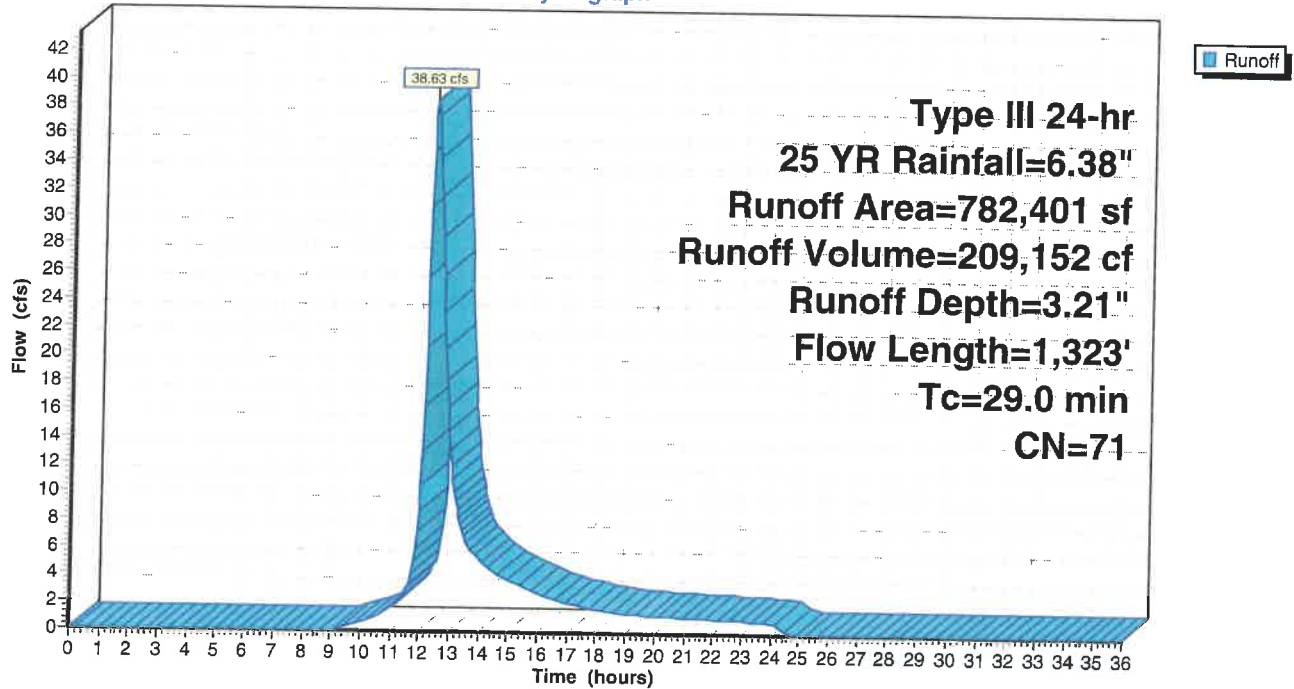
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 205,649 | 55 | Woods, Good, HSG B |
| 557,046 | 77 | Woods, Good, HSG D |
| 4,470 | 98 | Roofs, HSG B |
| 15,236 | 61 | >75% Grass cover, Good, HSG B |
| 782,401 | 71 | Weighted Average |
| 777,931 | | 99.43% Pervious Area |
| 4,470 | | 0.57% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 4.5 | 50 | 0.0350 | 0.19 | | Sheet Flow, Sheet |
| | | | | | Grass: Short n= 0.150 P2= 3.22" |
| 3.1 | 190 | 0.0210 | 1.01 | | Shallow Concentrated Flow, Shallow |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 16.2 | 735 | 0.0230 | 0.76 | | Shallow Concentrated Flow, c-d |
| | | | | | Woodland Kv= 5.0 fps |
| 5.2 | 348 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 29.0 | 1,323 | Total | | | |

Subcatchment 3S: To Wetland

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 4S: Toward Road

Runoff = 9.15 cfs @ 12.35 hrs, Volume= 48,703 cf, Depth= 1.74"

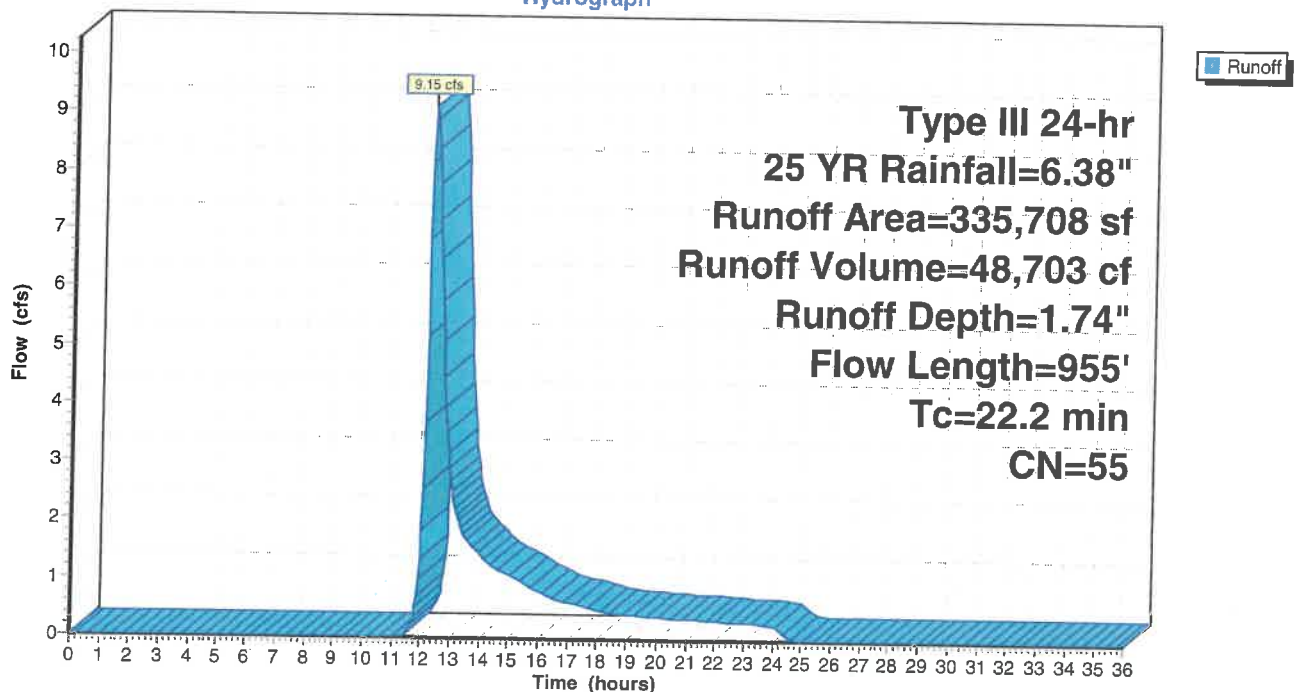
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 335,708 | 55 | Woods, Good, HSG B |
| 335,708 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, |
| 15.1 | 905 | 0.0401 | 1.00 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 955 | Total | | | |

Subcatchment 4S: Toward Road

Hydrograph



Summary for Subcatchment 5S: To Off Site

Runoff = 8.29 cfs @ 12.34 hrs, Volume= 43,561 cf, Depth= 1.83"

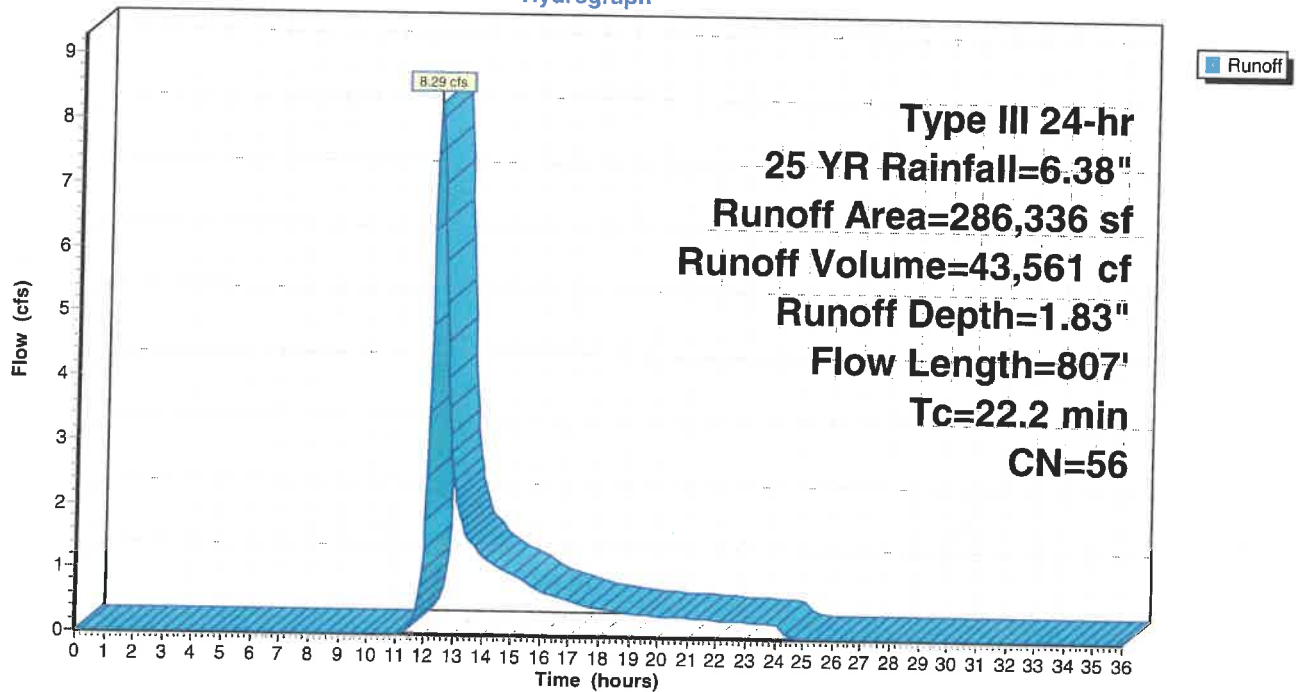
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 4,366 | 96 | Gravel surface, HSG B |
| 281,970 | 55 | Woods, Good, HSG B |
| 286,336 | 56 | Weighted Average |
| 286,336 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 10.3 | 50 | 0.0312 | 0.08 | | Sheet Flow, a-b |
| 11.9 | 757 | 0.0449 | 1.06 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, b-c |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 807 | Total | | | |

Subcatchment 5S: To Off Site

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Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth = 2.26" for 25 YR event
Inflow = 16.88 cfs @ 12.41 hrs, Volume= 92,937 cf
Outflow = 16.87 cfs @ 12.42 hrs, Volume= 92,937 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 8.70 fps, Min. Travel Time= 0.1 min

Avg. Velocity= 3.06 fps, Avg. Travel Time= 0.3 min

Peak Storage= 118 cf @ 12.41 hrs

Average Depth at Peak Storage= 0.48', Surface Width= 4.00'

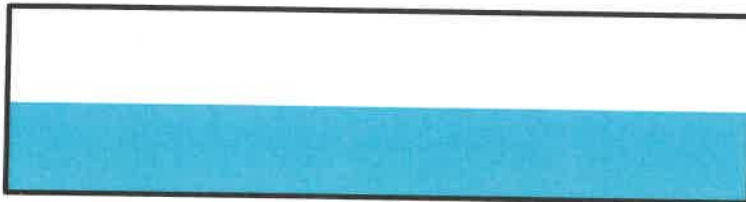
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe

n= 0.013 Concrete, trowel finish

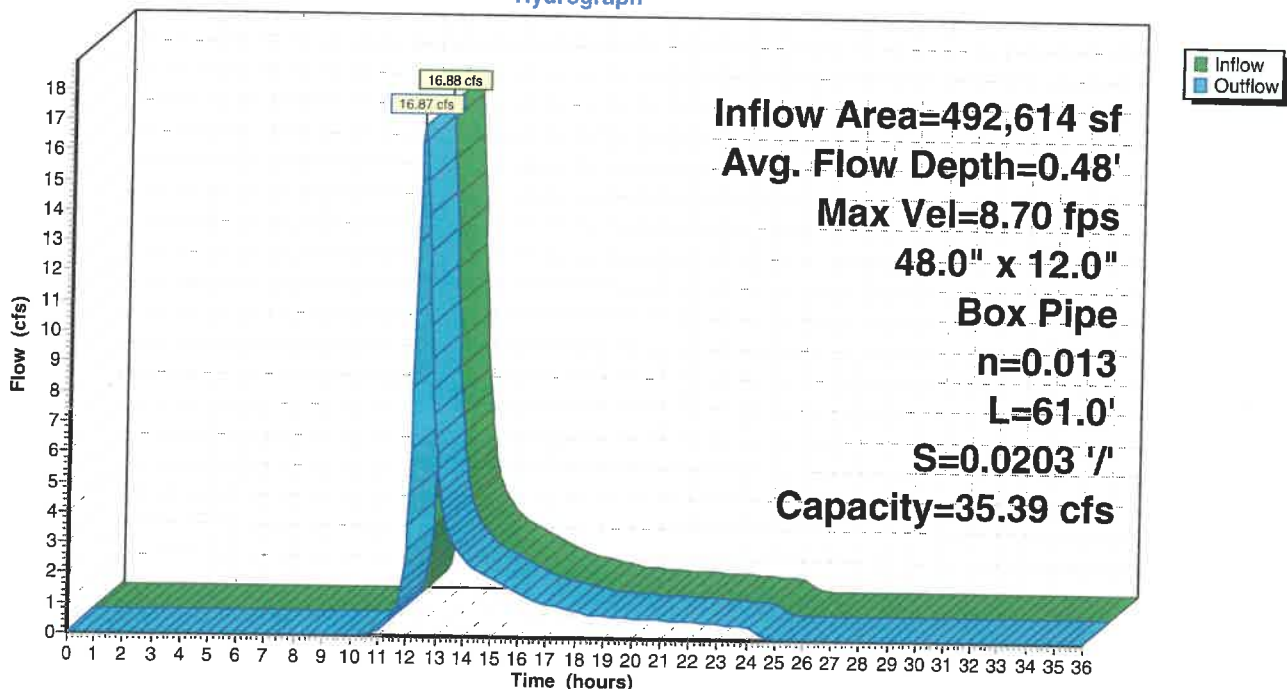
Length= 61.0' Slope= 0.0203 '/'

Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

Hydrograph



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Summary for Reach EV1: North Of Blackstone St - Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth = 1.92" for 25 YR event
Inflow = 45.97 cfs @ 13.05 hrs, Volume= 439,831 cf
Outflow = 45.93 cfs @ 13.06 hrs, Volume= 439,831 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.17 fps, Min. Travel Time= 0.0 min

Avg. Velocity= 1.29 fps, Avg. Travel Time= 0.1 min

Peak Storage= 88 cf @ 13.06 hrs

Average Depth at Peak Storage= 0.49' , Surface Width= 24.91'

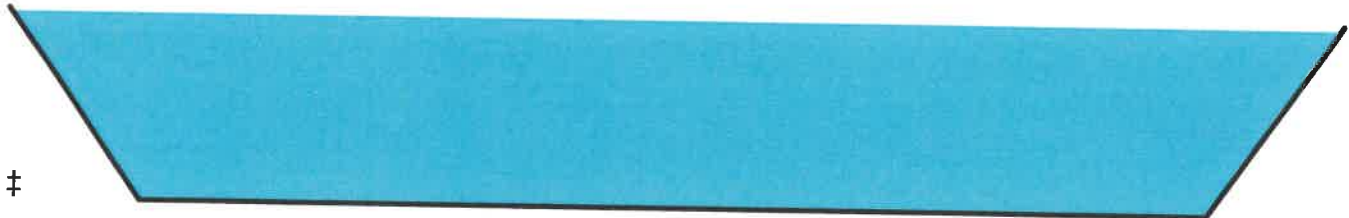
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 ' / ' Top Width= 25.00'

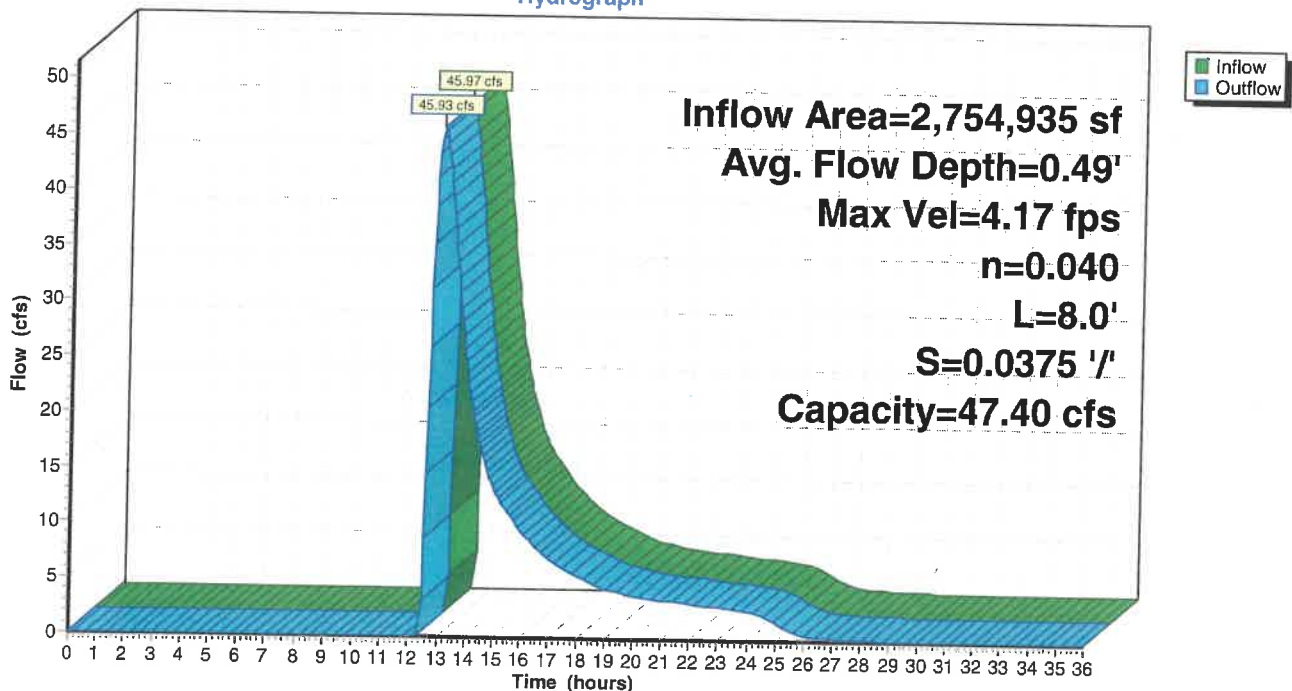
Length= 8.0' Slope= 0.0375 ' / '

Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: North Of Blackstone St - Vernal Pool

Hydrograph



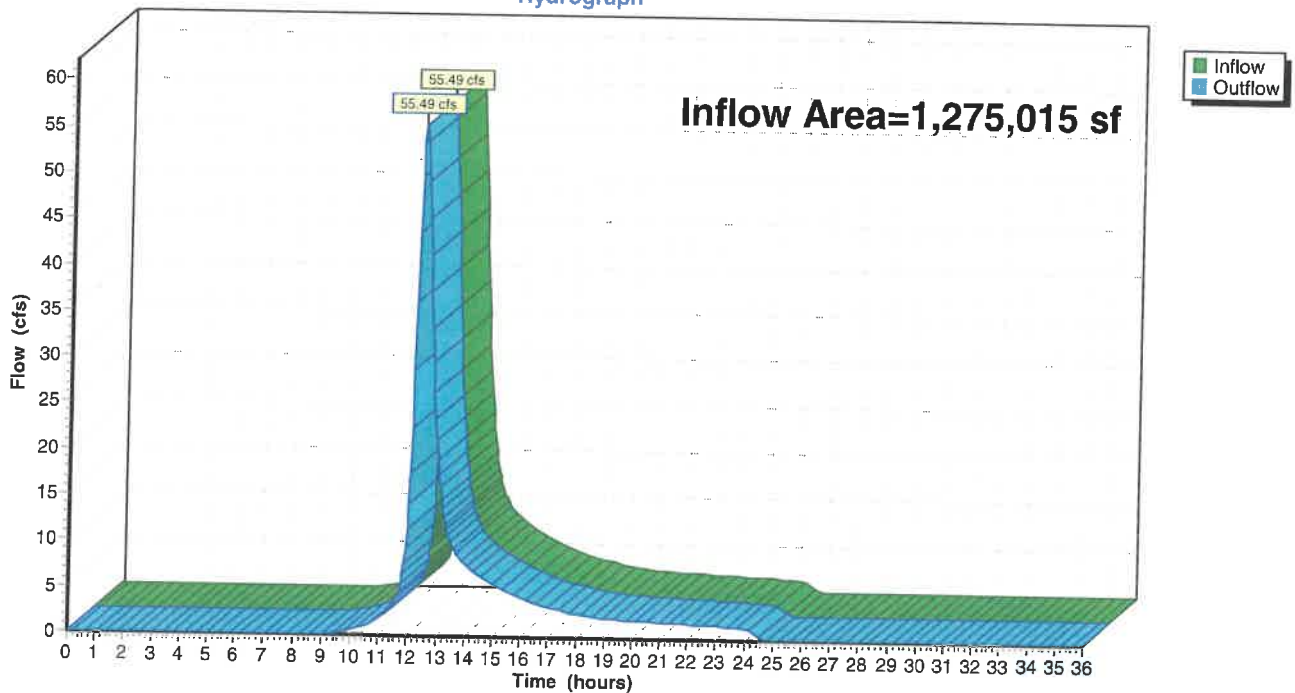
Summary for Reach EV2: Wetland

Inflow Area = 1,275,015 sf, 4.20% Impervious, Inflow Depth = 2.84" for 25 YR event
Inflow = 55.49 cfs @ 12.41 hrs, Volume= 302,089 cf
Outflow = 55.49 cfs @ 12.41 hrs, Volume= 302,089 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV2: Wetland

Hydrograph



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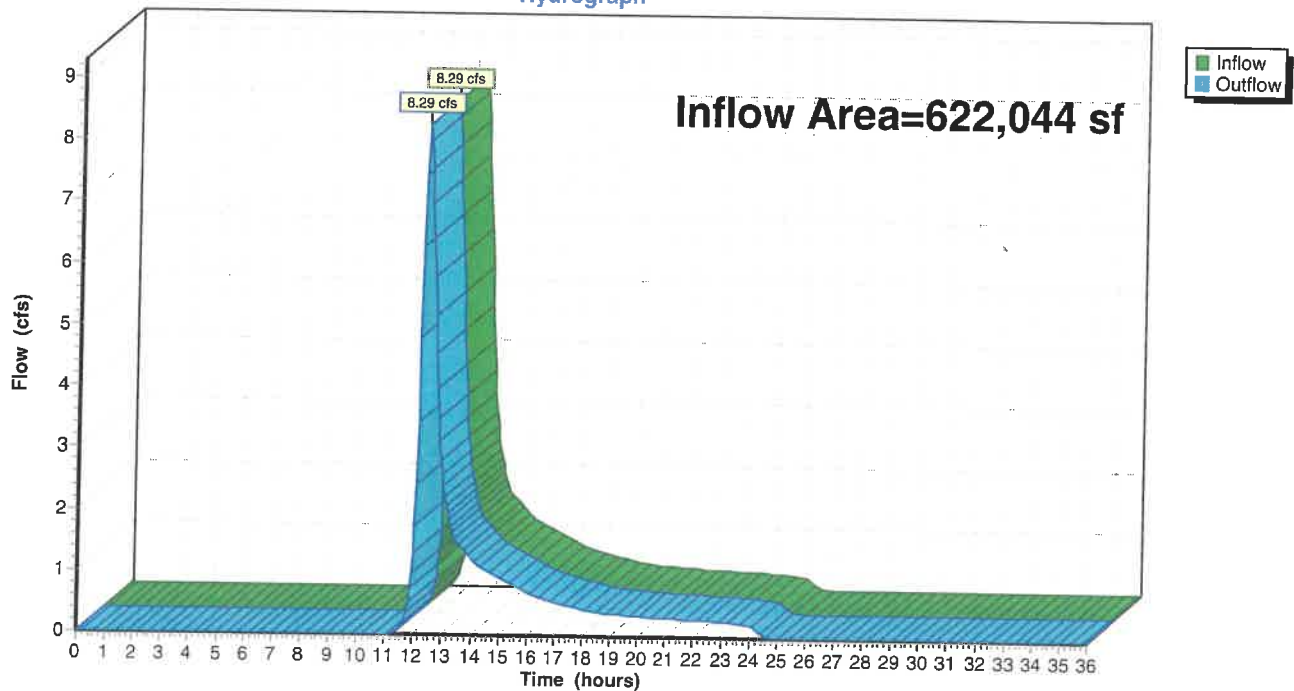
Summary for Reach EV3: Offsite

Inflow Area = 622,044 sf, 0.00% Impervious, Inflow Depth = 0.84" for 25 YR event
Inflow = 8.29 cfs @ 12.34 hrs, Volume= 43,561 cf
Outflow = 8.29 cfs @ 12.34 hrs, Volume= 43,561 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV3: Offsite

Hydrograph



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Summary for Pond 1P: Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth = 2.09" for 25 YR event
 Inflow = 66.94 cfs @ 12.68 hrs, Volume= 478,830 cf
 Outflow = 45.97 cfs @ 13.05 hrs, Volume= 439,831 cf, Atten= 31%, Lag= 22.6 min
 Primary = 45.97 cfs @ 13.05 hrs, Volume= 439,831 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 295.48' @ 13.05 hrs Surf.Area= 58,432 sf Storage= 110,347 cf

Plug-Flow detention time= 89.0 min calculated for 439,221 cf (92% of inflow)
 Center-of-Mass det. time= 48.7 min (947.2 - 898.5)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 291.30' | 143,915 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

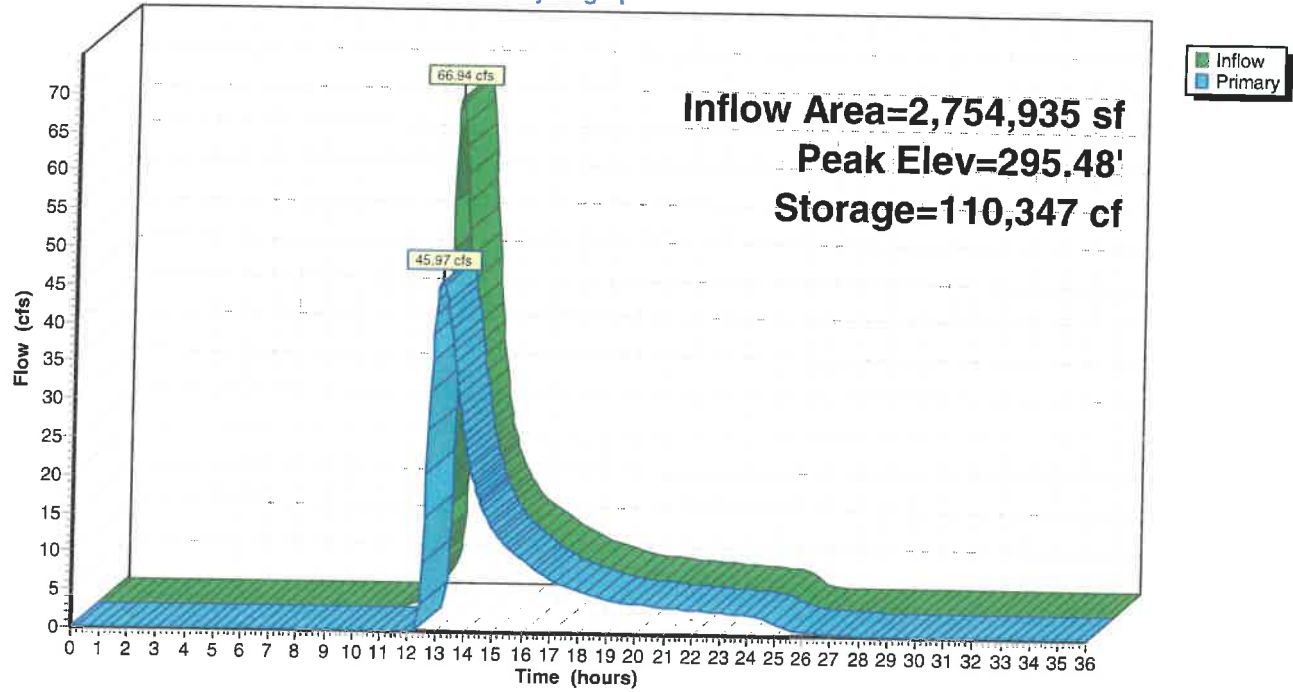
| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=45.95 cfs @ 13.05 hrs HW=295.48' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 45.95 cfs @ 3.42 fps)

Pond 1P: Vernal Pool

Hydrograph



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Summary for Pond 2P: Road Berm

Inflow Area = 335,708 sf, 0.00% Impervious, Inflow Depth = 1.74" for 25 YR event
 Inflow = 9.15 cfs @ 12.35 hrs, Volume= 48,703 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 311.52' @ 25.30 hrs Surf.Area= 52,940 sf Storage= 48,702 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 310.00' | 256,496 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

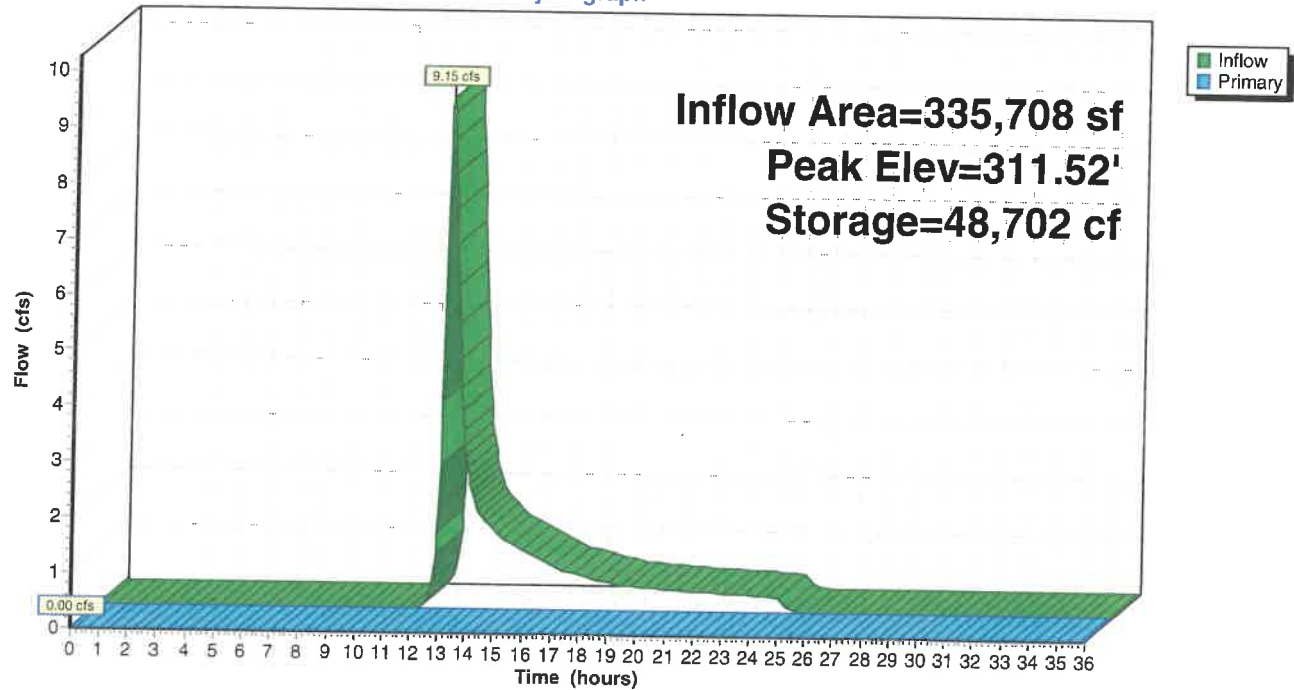
| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 313.50' | 8.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63 |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)

↑ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 2P: Road Berm

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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Subcatchment 1S: To Culvert

Runoff = 30.99 cfs @ 12.40 hrs, Volume= 165,275 cf, Depth= 4.03"

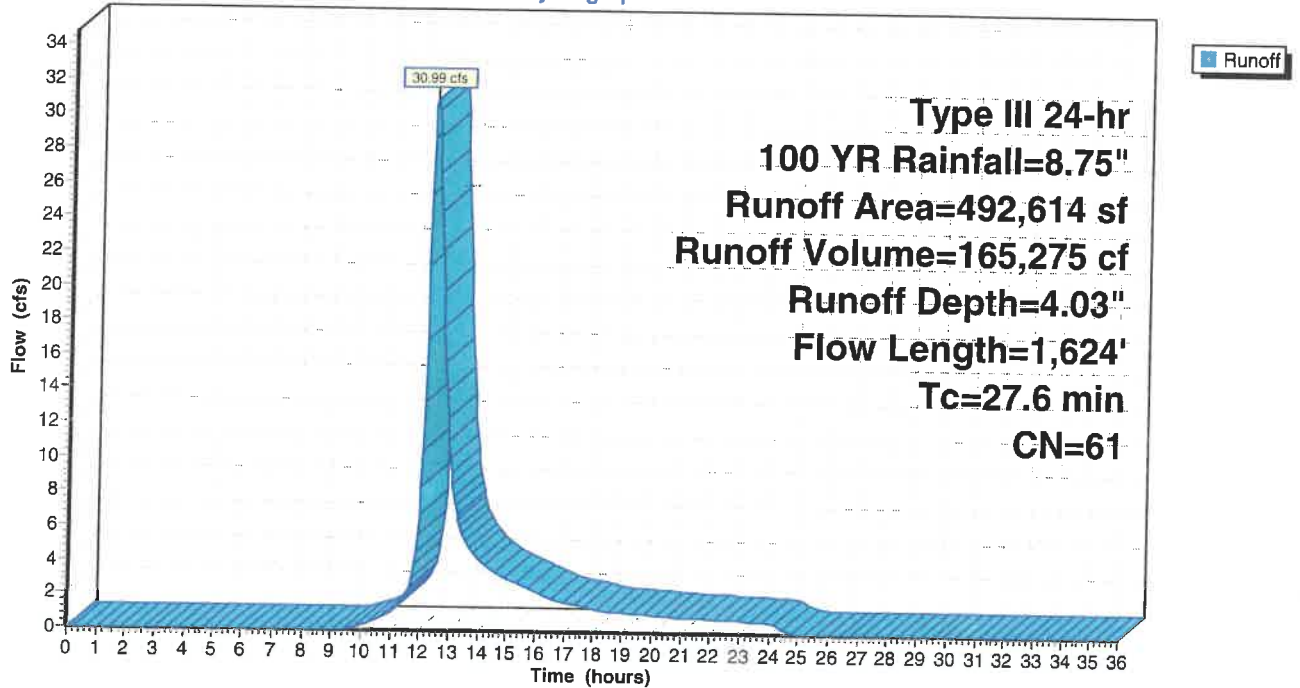
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, |
| | | | | | Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

Subcatchment 1S: To Culvert

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Summary for Subcatchment 2S: To Wetland

Runoff = 126.66 cfs @ 12.65 hrs, Volume= 869,134 cf, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow |
| | | | | | Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 45.8 | 2,588 | Total | | | |

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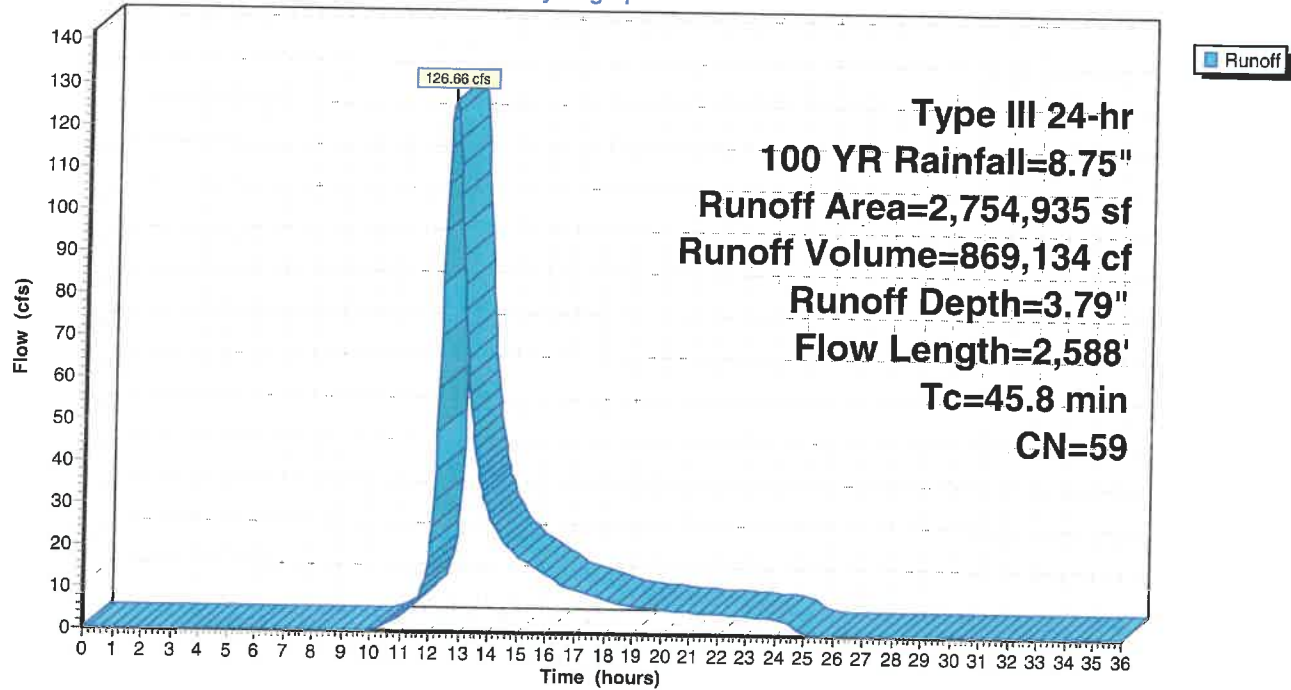
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Subcatchment 2S: To Wetland

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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Subcatchment 3S: To Wetland

Runoff = 63.23 cfs @ 12.40 hrs, Volume= 341,441 cf, Depth= 5.24"

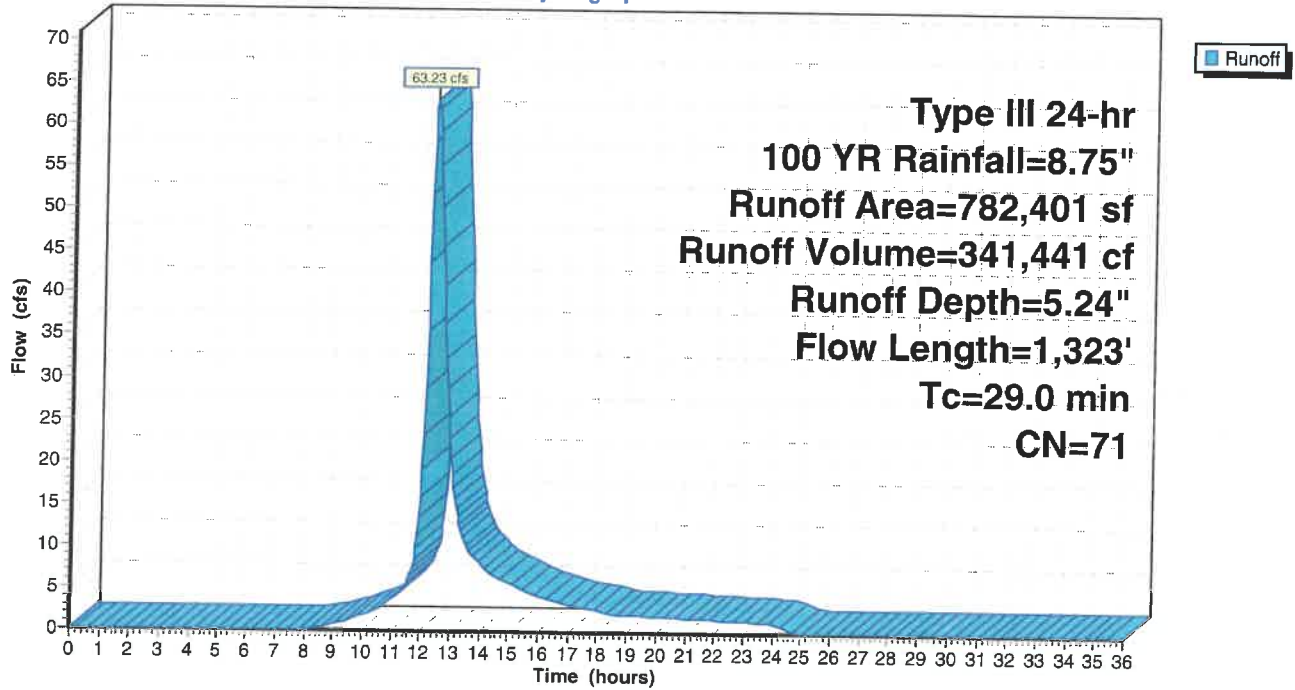
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 205,649 | 55 | Woods, Good, HSG B |
| 557,046 | 77 | Woods, Good, HSG D |
| 4,470 | 98 | Roofs, HSG B |
| 15,236 | 61 | >75% Grass cover, Good, HSG B |
| 782,401 | 71 | Weighted Average |
| 777,931 | | 99.43% Pervious Area |
| 4,470 | | 0.57% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 4.5 | 50 | 0.0350 | 0.19 | | Sheet Flow, Sheet |
| | | | | | Grass: Short n= 0.150 P2= 3.22" |
| 3.1 | 190 | 0.0210 | 1.01 | | Shallow Concentrated Flow, Shallow |
| | | | | | Short Grass Pasture Kv= 7.0 fps |
| 16.2 | 735 | 0.0230 | 0.76 | | Shallow Concentrated Flow, c-d |
| | | | | | Woodland Kv= 5.0 fps |
| 5.2 | 348 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream |
| | | | | | Area= 1.2 sf Perim= 3.5' r= 0.34' |
| | | | | | n= 0.070 Medium-dense brush |
| 29.0 | 1,323 | Total | | | |

Subcatchment 3S: To Wetland

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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Subcatchment 4S: Toward Road

Runoff = 18.61 cfs @ 12.33 hrs, Volume= 92,555 cf, Depth= 3.31"

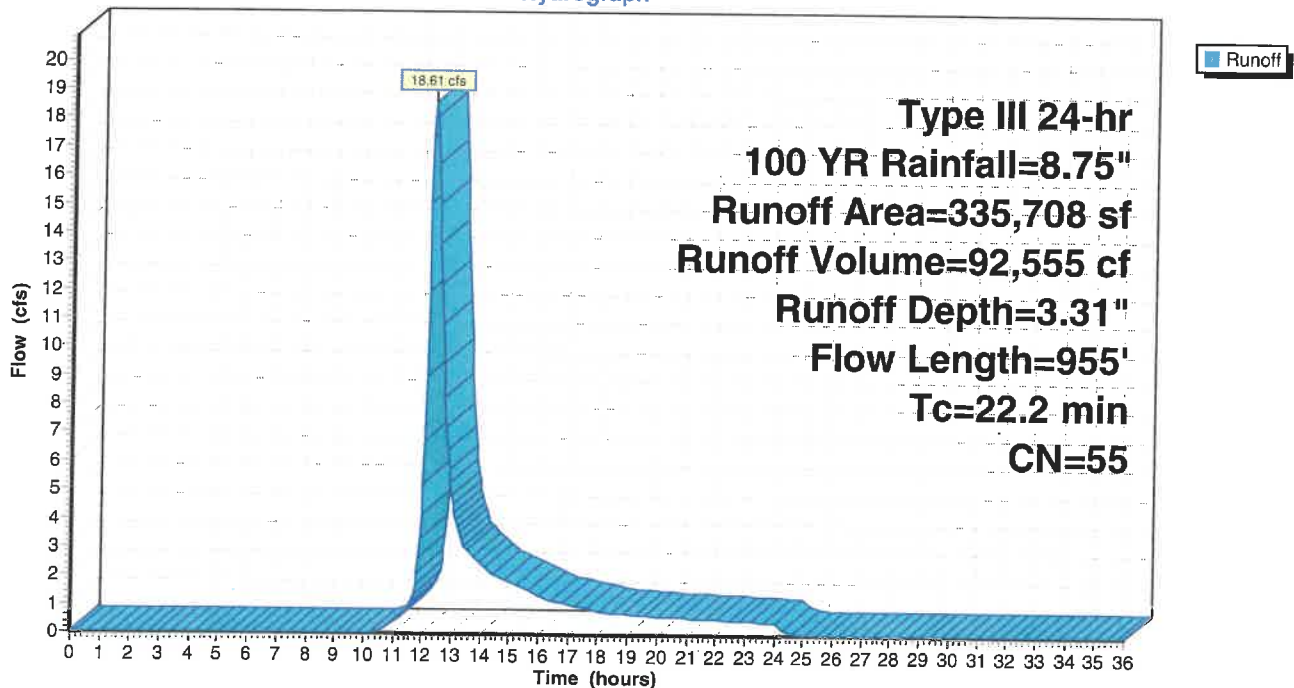
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 335,708 | 55 | Woods, Good, HSG B |
| 335,708 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, |
| 15.1 | 905 | 0.0401 | 1.00 | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| | | | | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 955 | Total | | | |

Subcatchment 4S: Toward Road

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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Subcatchment 5S: To Off Site

Runoff = 16.56 cfs @ 12.32 hrs, Volume= 81,780 cf, Depth= 3.43"

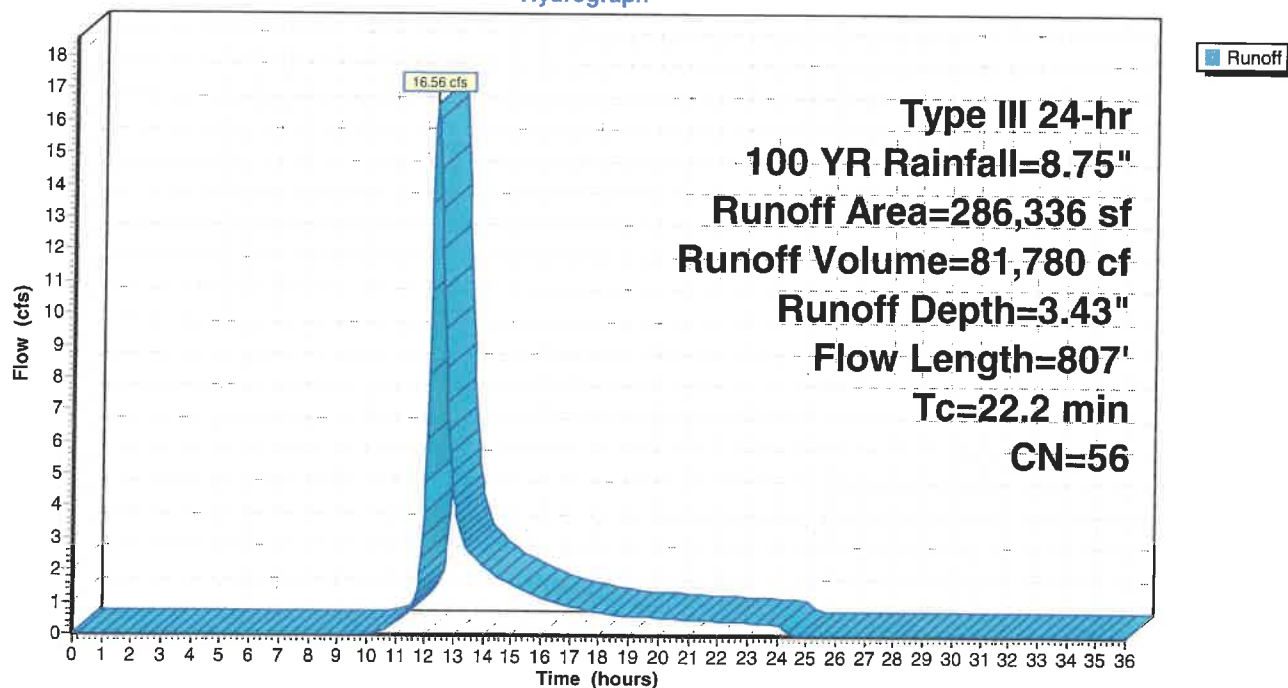
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 4,366 | 96 | Gravel surface, HSG B |
| 281,970 | 55 | Woods, Good, HSG B |
| 286,336 | 56 | Weighted Average |
| 286,336 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 10.3 | 50 | 0.0312 | 0.08 | | Sheet Flow, a-b |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 11.9 | 757 | 0.0449 | 1.06 | | Shallow Concentrated Flow, b-c |
| | | | | | Woodland Kv= 5.0 fps |
| 22.2 | 807 | Total | | | |

Subcatchment 5S: To Off Site

Hydrograph



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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth = 4.03" for 100 YR event
Inflow = 30.99 cfs @ 12.40 hrs, Volume= 165,275 cf
Outflow = 30.97 cfs @ 12.40 hrs, Volume= 165,275 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 10.70 fps, Min. Travel Time= 0.1 min

Avg. Velocity= 3.64 fps, Avg. Travel Time= 0.3 min

Peak Storage= 177 cf @ 12.40 hrs

Average Depth at Peak Storage= 0.72', Surface Width= 4.00'

Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe

n= 0.013 Concrete, trowel finish

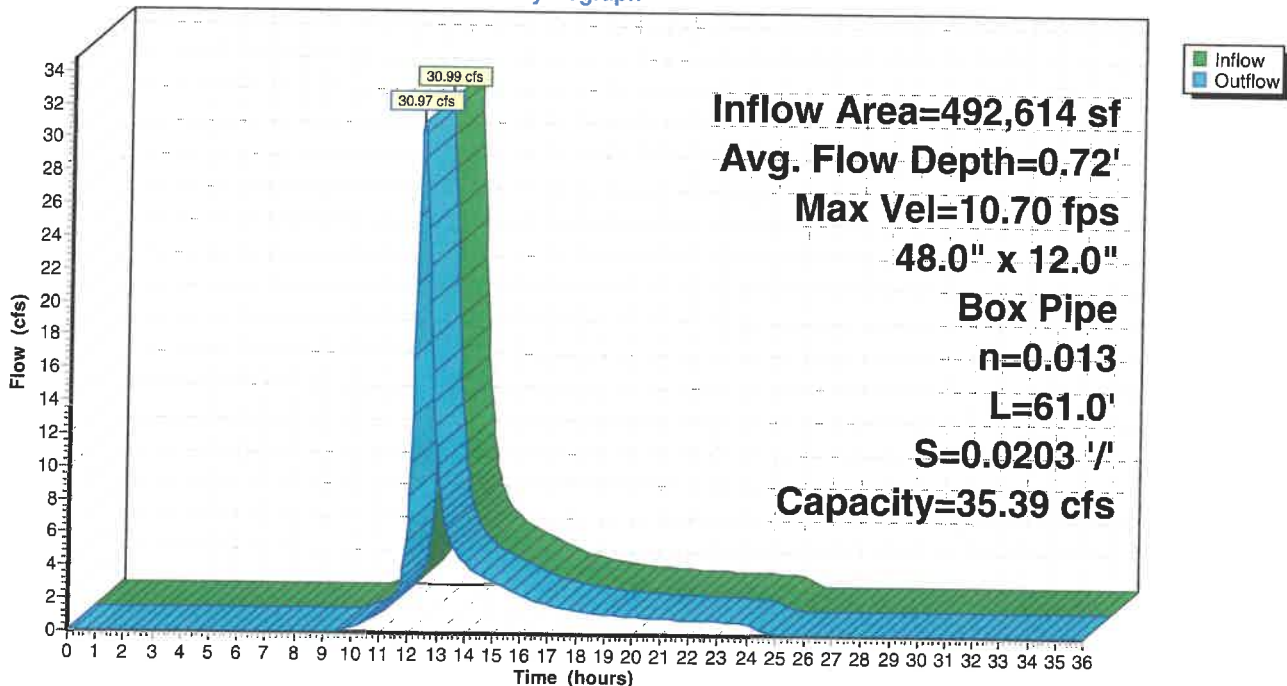
Length= 61.0' Slope= 0.0203 '/'

Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

Hydrograph



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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Reach EV1: North Of Blackstone St - Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth = 3.62" for 100 YR event
Inflow = 135.67 cfs @ 12.67 hrs, Volume= 830,132 cf
Outflow = 134.84 cfs @ 12.67 hrs, Volume= 830,132 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.42 fps, Min. Travel Time= 0.0 min

Avg. Velocity= 1.56 fps, Avg. Travel Time= 0.1 min

Peak Storage= 198 cf @ 12.67 hrs

Average Depth at Peak Storage= 1.04', Surface Width= 30.42'

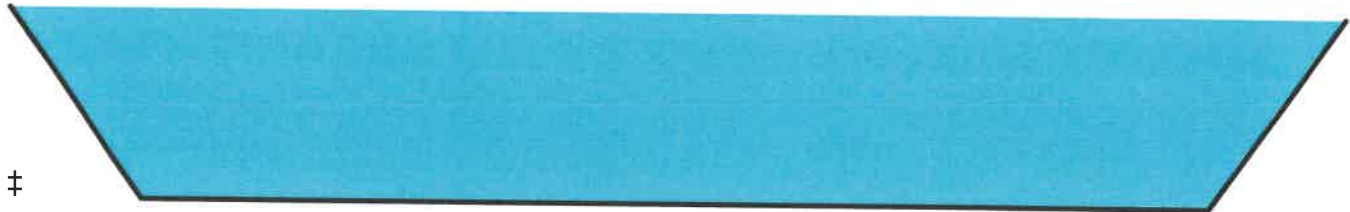
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides

Side Slope Z-value= 5.0 '/' Top Width= 25.00'

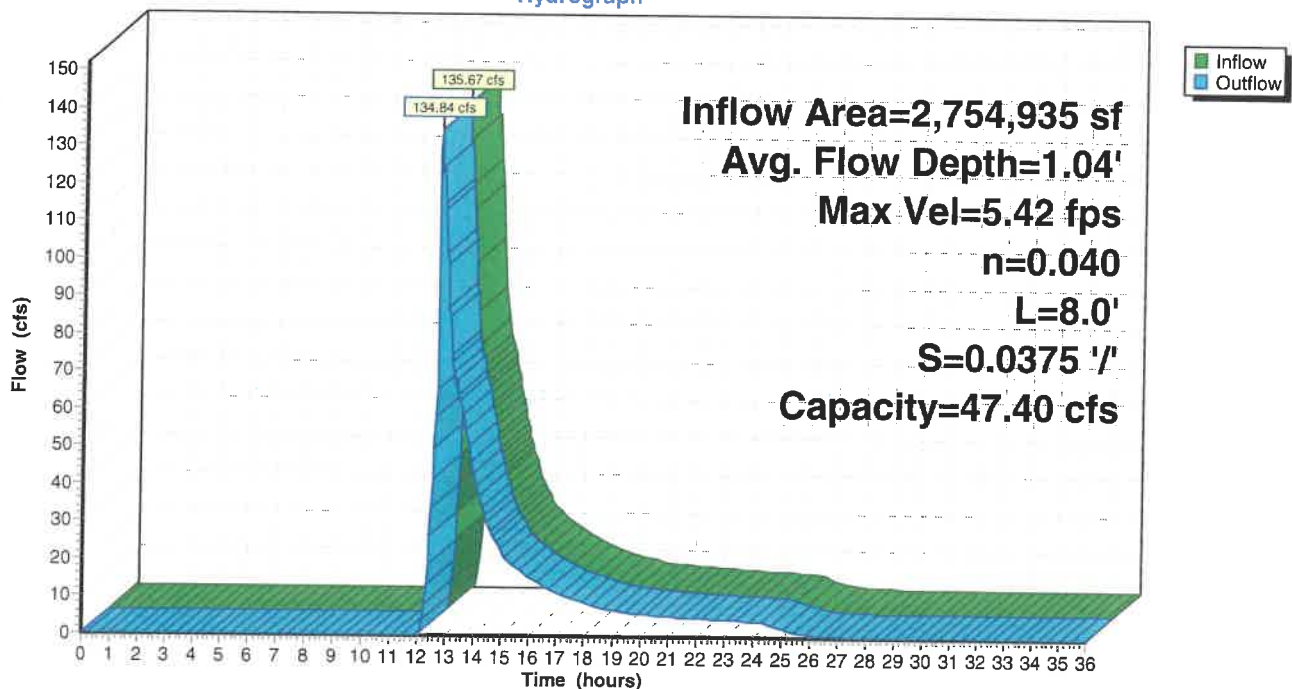
Length= 8.0' Slope= 0.0375 '/'

Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: North Of Blackstone St - Vernal Pool

Hydrograph



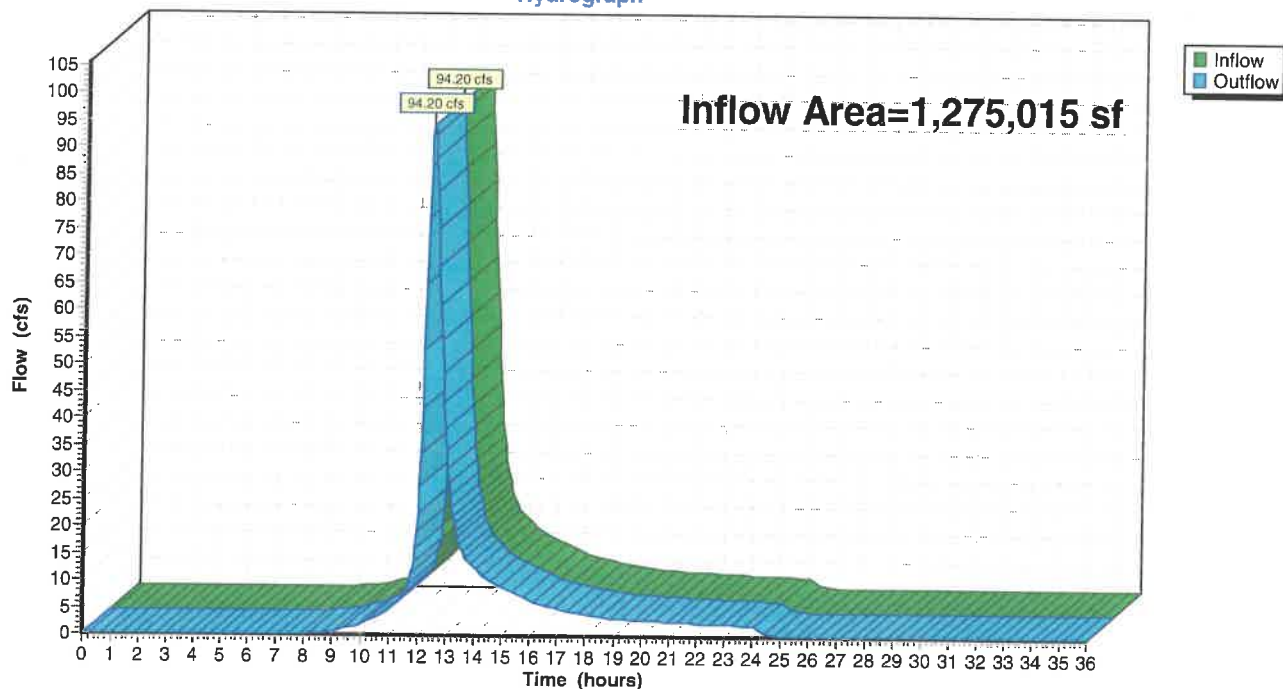
Summary for Reach EV2: Wetland

Inflow Area = 1,275,015 sf, 4.20% Impervious, Inflow Depth = 4.77" for 100 YR event
Inflow = 94.20 cfs @ 12.40 hrs, Volume= 506,716 cf
Outflow = 94.20 cfs @ 12.40 hrs, Volume= 506,716 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV2: Wetland

Hydrograph



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Type III 24-hr 100 YR Rainfall=8.75"

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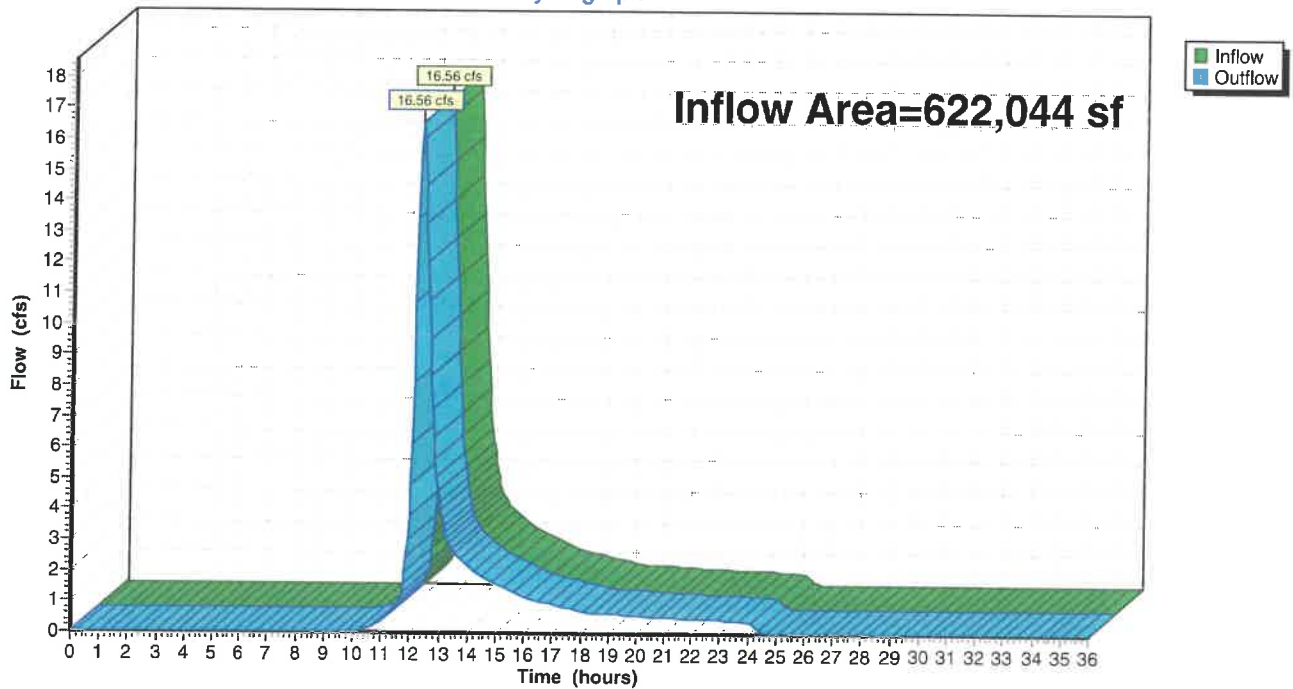
Summary for Reach EV3: Offsite

Inflow Area = 622,044 sf, 0.00% Impervious, Inflow Depth = 1.58" for 100 YR event
Inflow = 16.56 cfs @ 12.32 hrs, Volume= 81,780 cf
Outflow = 16.56 cfs @ 12.32 hrs, Volume= 81,780 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs

Reach EV3: Offsite

Hydrograph



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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Pond 1P: Vernal Pool

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth = 3.79" for 100 YR event
 Inflow = 126.66 cfs @ 12.65 hrs, Volume= 869,134 cf
 Outflow = 135.67 cfs @ 12.67 hrs, Volume= 830,132 cf, Atten= 0%, Lag= 1.0 min
 Primary = 135.67 cfs @ 12.67 hrs, Volume= 830,132 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 297.27' @ 12.67 hrs Surf.Area= 70,541 sf Storage= 143,915 cf

Plug-Flow detention time= 60.7 min calculated for 828,981 cf (95% of inflow)
 Center-of-Mass det. time= 36.7 min (917.2 - 880.5)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 291.30' | 143,915 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=127.01 cfs @ 12.67 hrs HW=297.11' (Free Discharge)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 127.01 cfs @ 4.80 fps)

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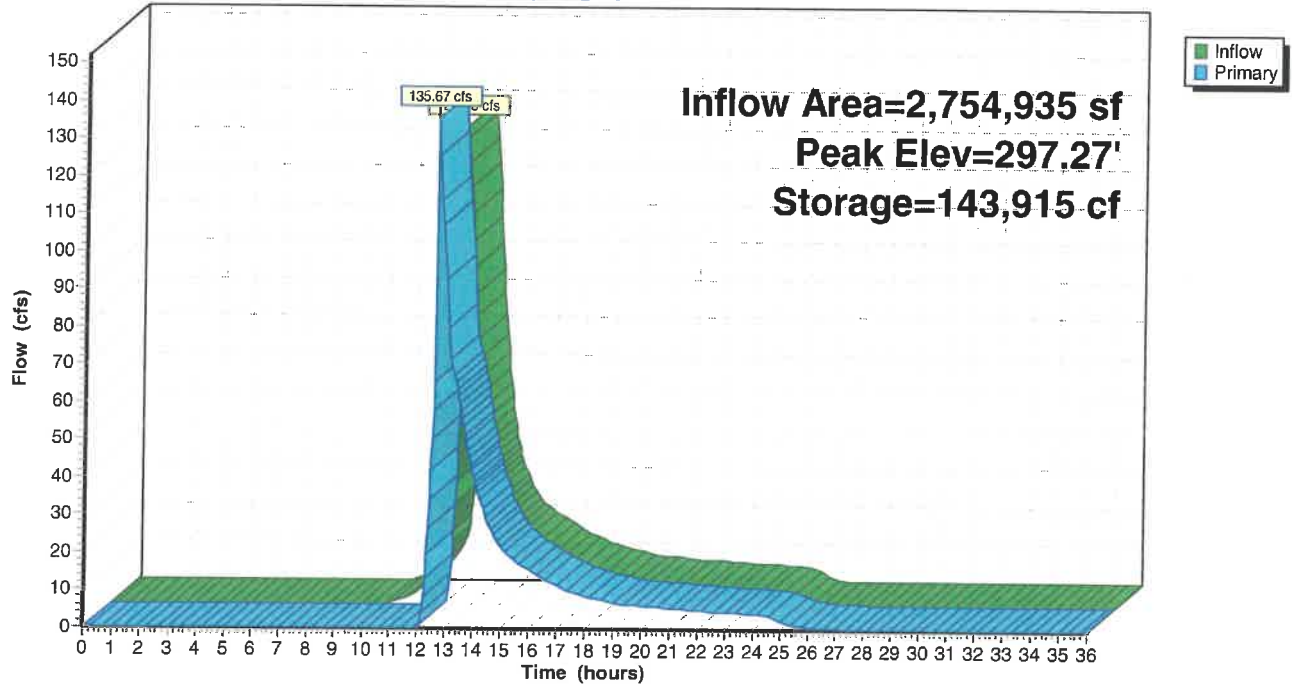
Type III 24-hr 100 YR Rainfall=8.75"

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Pond 1P: Vernal Pool

Hydrograph



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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Pond 2P: Road Berm

Inflow Area = 335,708 sf, 0.00% Impervious, Inflow Depth = 3.31" for 100 YR event
 Inflow = 18.61 cfs @ 12.33 hrs, Volume= 92,555 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.05 hrs
 Peak Elev= 312.22' @ 25.30 hrs Surf.Area= 71,387 sf Storage= 92,551 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 310.00' | 256,496 cf | Custom Stage Data (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

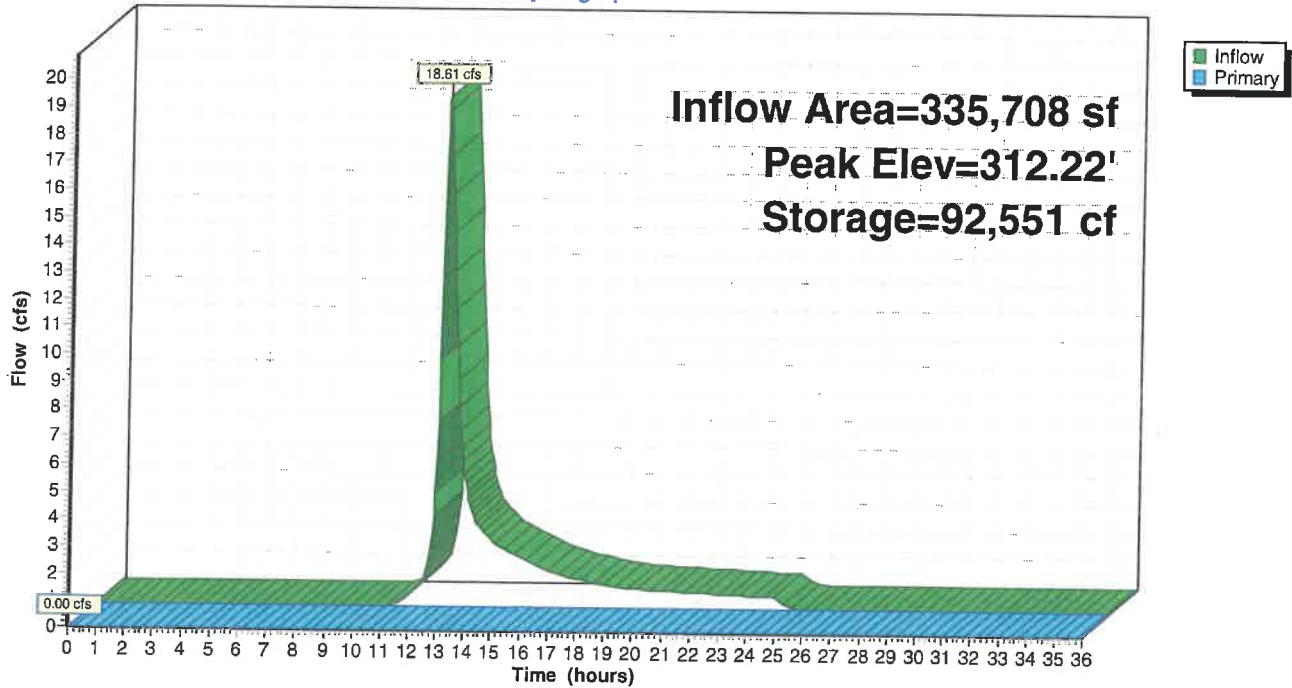
| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 313.50' | 8.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63 |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)

↑ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 2P: Road Berm

Hydrograph



Post-Development Drainage Calculations



DRAINAGE LEGEND

- SUBCATCHMENT AREA
- REACH
- POND
- SOIL DELINEATION
- TIME OF CONCENTRATION PATH
- DRAINAGE AREA LIMIT

LEGEND

- EDGE OF WETLAND
- EXISTING CONTOUR
- PROPOSED CONTOUR
- EXISTING TREE LINE

NOTES

- ELEVATIONS REFER TO NAVD 88 VERTICAL DATUM.
- HORIZONTAL DATUM: NAD 83
- PARTIAL WETLAND RESOURCES FLAGGED BY GODDARD CONSULTING IN 2024. FLAGS WERE FIELD LOCATED BY ALLEN ENGINEERING & ASSOCIATES IN NOVEMBER 2024.
- PROPERTY LINE AND TOPOGRAPHIC INFORMATION OBTAINED BY ALLEN ENGINEERING & ASSOCIATES, INC IN AUGUST, 2024.
- UNDERGROUND UTILITIES SHOWN HAVE BEEN COMPILED FROM AVAILABLE PLANS FROM PUBLIC AND PRIVATE AGENCIES, OR AS INDICATED BY FIELD OBSERVATIONS AND ARE APPROXIMATE ONLY. ADDITIONAL UNDERGROUND UTILITIES OR STRUCTURES NOT SHOWN HEREON MIGHT EXIST. BEFORE ANY EXCAVATION OR CONSTRUCTION CALL DIG-SAFE AT 811.
- TOPOGRAPHIC INFORMATION OBTAINED FROM LIDAR DATA ISSUED BY NOAA (2021).

OWNER:

Wall Street
Development Corp.
P.O. Box 272
Westwood, MA 02090

TITLE:

PROPOSED
DRAINAGE PLAN
for
Blackstone Street
Improvements
Bellingham, MA

SEAL:



PROFESSIONAL ENGINEER

PREPARED BY:



**ALLEN ENGINEERING
& ASSOCIATES, INC.**

Civil Engineers • Surveyors
Land Development Consultants

140 Hartford Avenue East
Hopedale, Ma 01747
(508) 381-3212 • www.allen-ea.com

SCALE:

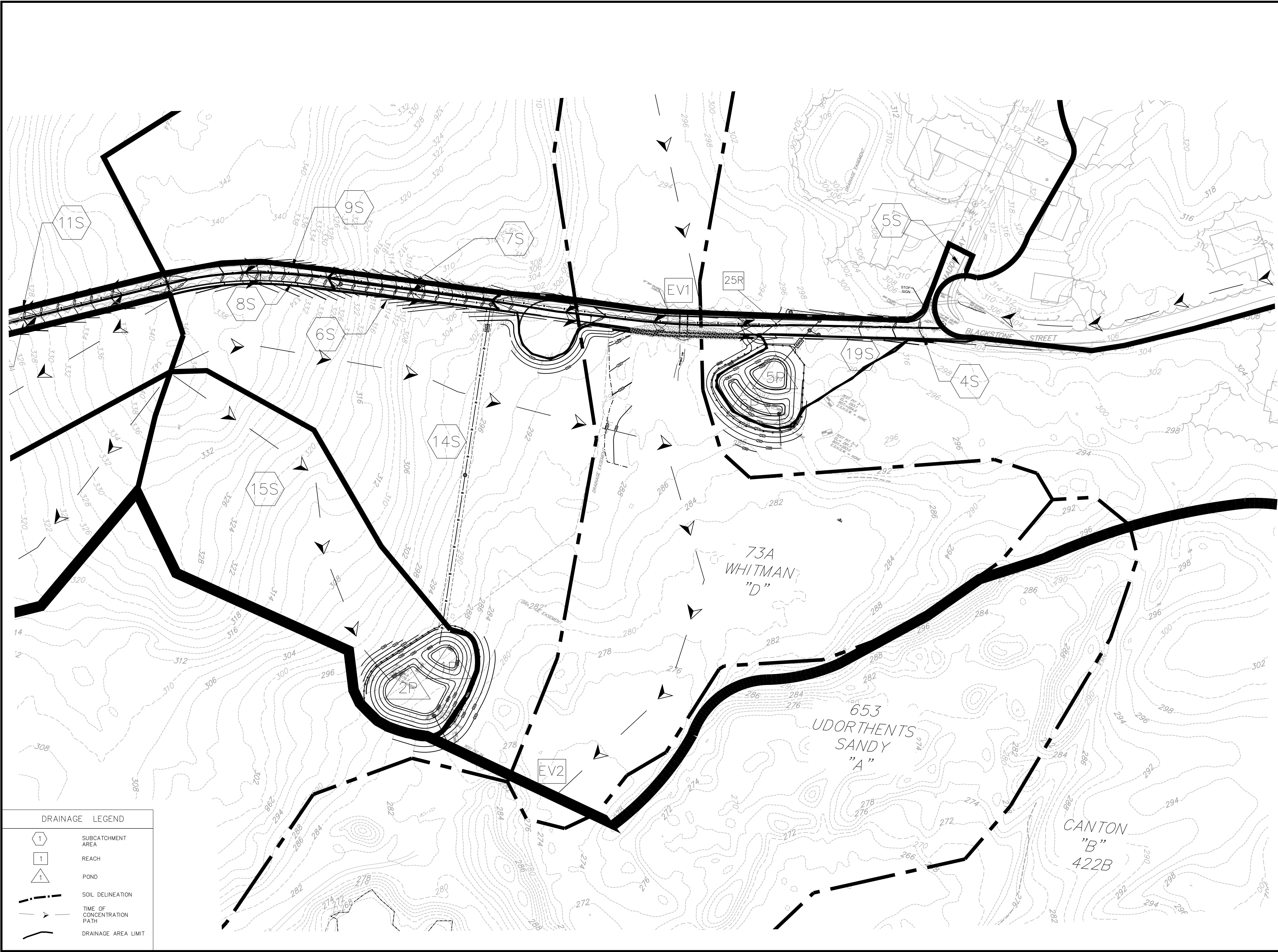
1" = 150 FEET

DATE:

Lcpwct{#1: #1247

REVISIONS

| # | DATE | DESCRIPTION | INIT. |
|---------|----------|----------------------|--------|
| 1 | 6-20-25 | ROADWAY AND DRAINAGE | MEA |
| 2 | 10-31-25 | PER PEER REVIEW | MEA |
| 3 | 1-22-26 | PER PEER REVIEW | MEA |
| JOB NO: | | SHEET: | 1 of 3 |
| 00454 | | | |



LEGEND

EDGE OF WETLAND

EXISTING CONTOUR

PROPOSED CONTOUR

EXISTING TREE LINE

NOTES

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- HORIZONTAL DATUM: NAD 83
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OWNER:

Wall Street Development Corp.
P.O. Box 272
Westwood, MA 02090

TITLE:

PROPOSED DRAINAGE PLAN
for
Blackstone Street Improvements
Bellingham, MA

SEAL:

PROFESSIONAL ENGINEER

PREPARED BY:

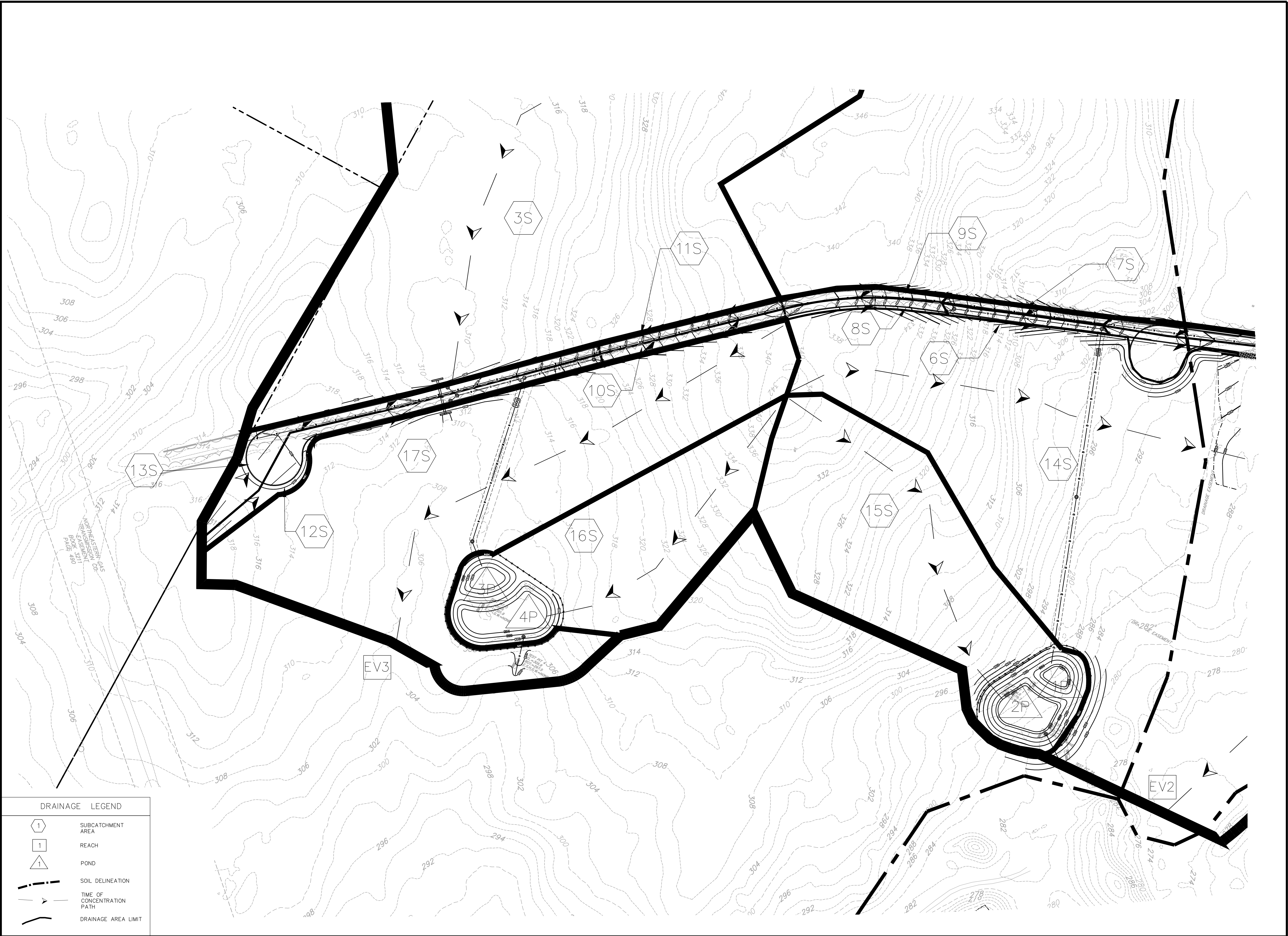
ALLEN ENGINEERING & ASSOCIATES, INC.
Civil Engineers • Surveyors
Land Development Consultants
140 Hartford Avenue East
Hopedale, Ma 01747
(508) 381-3212 • www.allen-ea.com

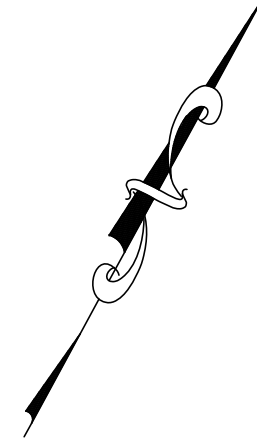
SCALE: 1" = 60 FEET

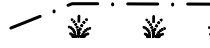
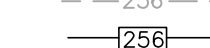

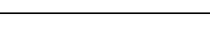
DATE: **Lcpwct{#1: #247**

| REVISIONS | | | |
|-----------|----------|----------------------|-------|
| # | DATE | DESCRIPTION | INIT. |
| 1 | 6-20-25 | ROADWAY AND DRAINAGE | MEA |
| 2 | 10-31-25 | PER PEER REVIEW | MEA |
| 3 | 1-22-26 | PER PEER REVIEW | MEA |

JOB NO: 00454 SHEET: **2** of **3**







LEGEND
 EDGE OF WETLAND
 EXISTING CONTOUR
 PROPOSED CONTOUR
 EXISTING TREE LINE


NOTES
1. ELEVATIONS REFER TO NAVD 88 VERTICAL DATUM.
2. HORIZONTAL DATUM: NAD 83
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4. PROPERTY LINE AND TOPOGRAPHIC INFORMATION OBTAINED BY ALLEN ENGINEERING & ASSOCIATES, INC IN AUGUST, 2024.
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6. TOPOGRAPHIC INFORMATION OBTAINED FROM LIDAR DATA ISSUED BY NOAA (2021).

OWNER:
Wall Street Development Corp.
P.O. Box 272
Westwood, MA 02090

TITLE:
PROPOSED DRAINAGE PLAN
for
Blackstone Street Improvements
Bellingham, MA

SEAL:

PROFESSIONAL ENGINEER

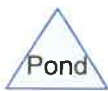
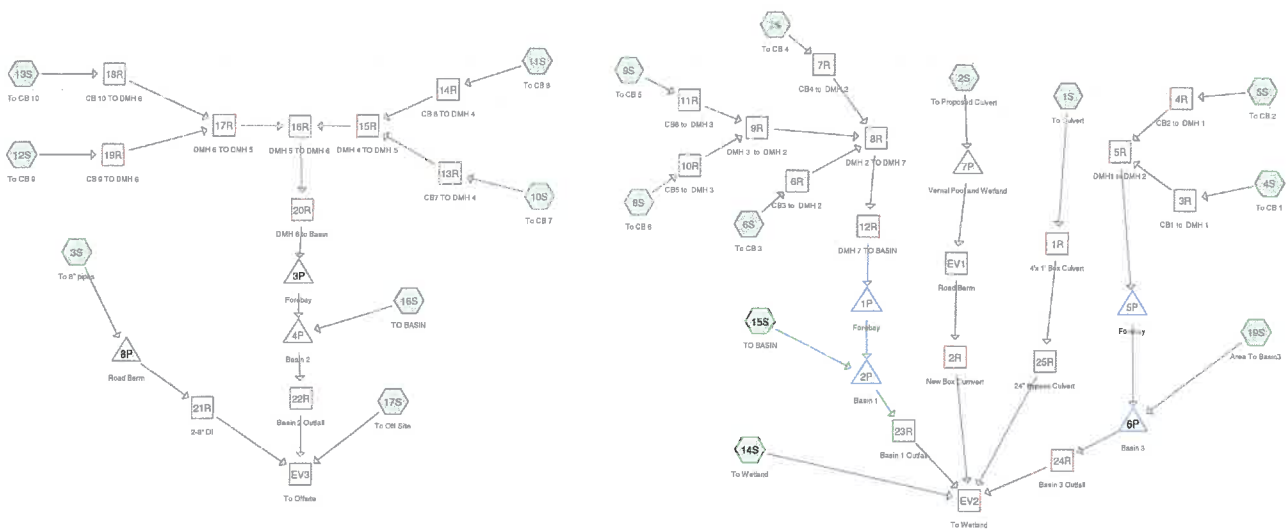
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140 Hartford Avenue East
Hopedale, Ma 01747
(508) 381-3212 · www.allen-ea.com

SCALE: 1" = 60 FEET


DATE: 1/22/26

| # | DATE | DESCRIPTION | INIT |
|---|----------|----------------------|------|
| 1 | 6-20-25 | ROADWAY AND DRAINAGE | MEA |
| 2 | 10-31-25 | PER PEER REVIEW | MEA |
| 3 | 1-22-26 | PER PEER REVIEW | MEA |

JOB NO: 00454
SHEET: 3 of 3



Routing Diagram for 00454 - PROPOSED DRAINAGE 1-22-26
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00454 - PROPOSED DRAINAGE 1-22-26

Prepared by {enter your company name here}

Printed 1/22/2026

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Area Listing (all nodes)

| Area (sq-ft) | CN | Description (subcatchment-numbers) |
|------------------|-----------|--|
| 525,559 | 61 | >75% Grass cover, Good, HSG B (1S, 2S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 19S) |
| 20,077 | 80 | >75% Grass cover, Good, HSG D (2S) |
| 882 | 96 | Gravel surface, HSG B (2S) |
| 7,421 | 96 | Gravel surface, HSG D (2S) |
| 146,933 | 98 | Paved roads w/curbs & sewers, HSG B (1S, 2S, 4S, 5S, 6S, 7S, 8S, 9S, 10S, 11S, 12S, 13S) |
| 39,866 | 98 | Roofs, HSG B (1S, 2S) |
| 3,202 | 98 | Roofs, HSG D (2S) |
| 3,410,518 | 55 | Woods, Good, HSG B (1S, 2S, 3S, 14S, 15S, 16S, 17S) |
| 497,536 | 77 | Woods, Good, HSG D (2S, 14S) |
| 4,651,994 | 60 | TOTAL AREA |

00454 - PROPOSED DRAINAGE 1-22-26

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00454 - PROPOSED Conditions
Type III 24-hr 2 YR Rainfall=3.38"

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Summary for Subcatchment 1S: To Culvert

Runoff = 2.90 cfs @ 12.51 hrs, Volume= 21,126 cf, Depth> 0.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

00454 - PROPOSED DRAINAGE 1-22-26

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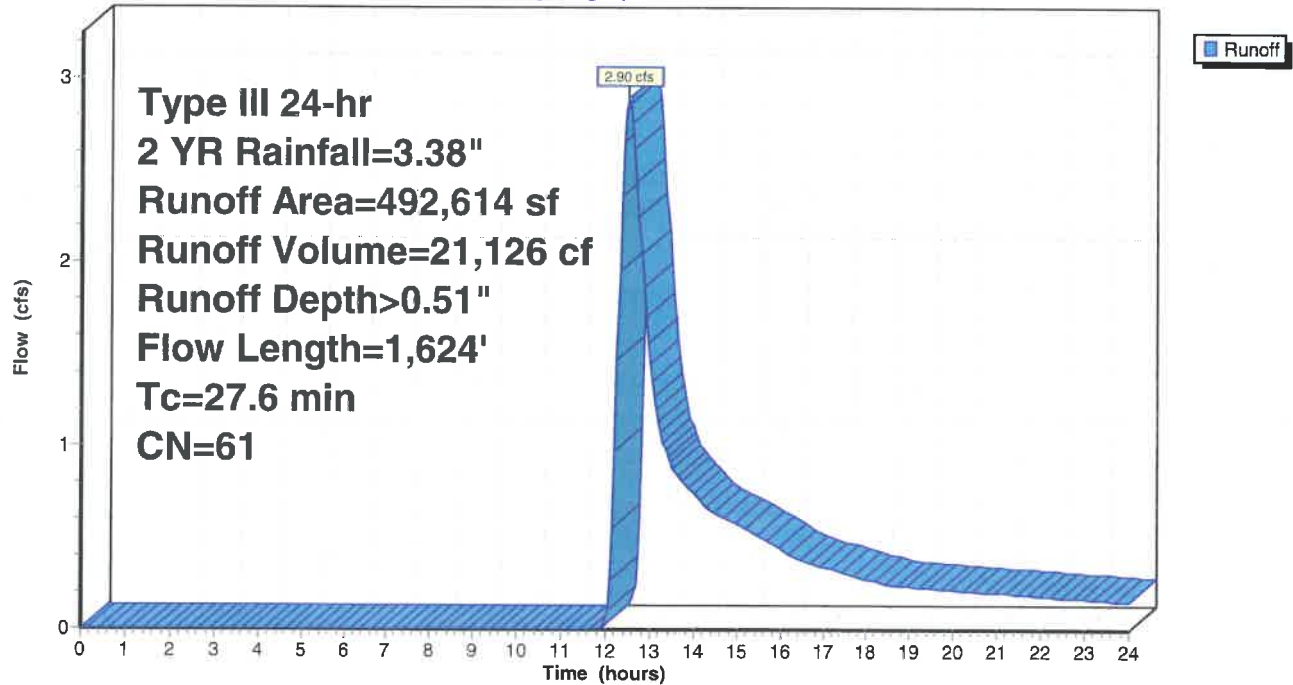
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Type III 24-hr 2 YR Rainfall=3.38"

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Subcatchment 1S: To Culvert

Hydrograph



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00454 - PROPOSED Conditions
Type III 24-hr 2 YR Rainfall=3.38"

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Summary for Subcatchment 2S: To Proposed Culvert

Runoff = 9.98 cfs @ 12.81 hrs, Volume= 99,846 cf, Depth> 0.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 45.8 | 2,588 | Total | | | |

00454 - PROPOSED DRAINAGE 1-22-26

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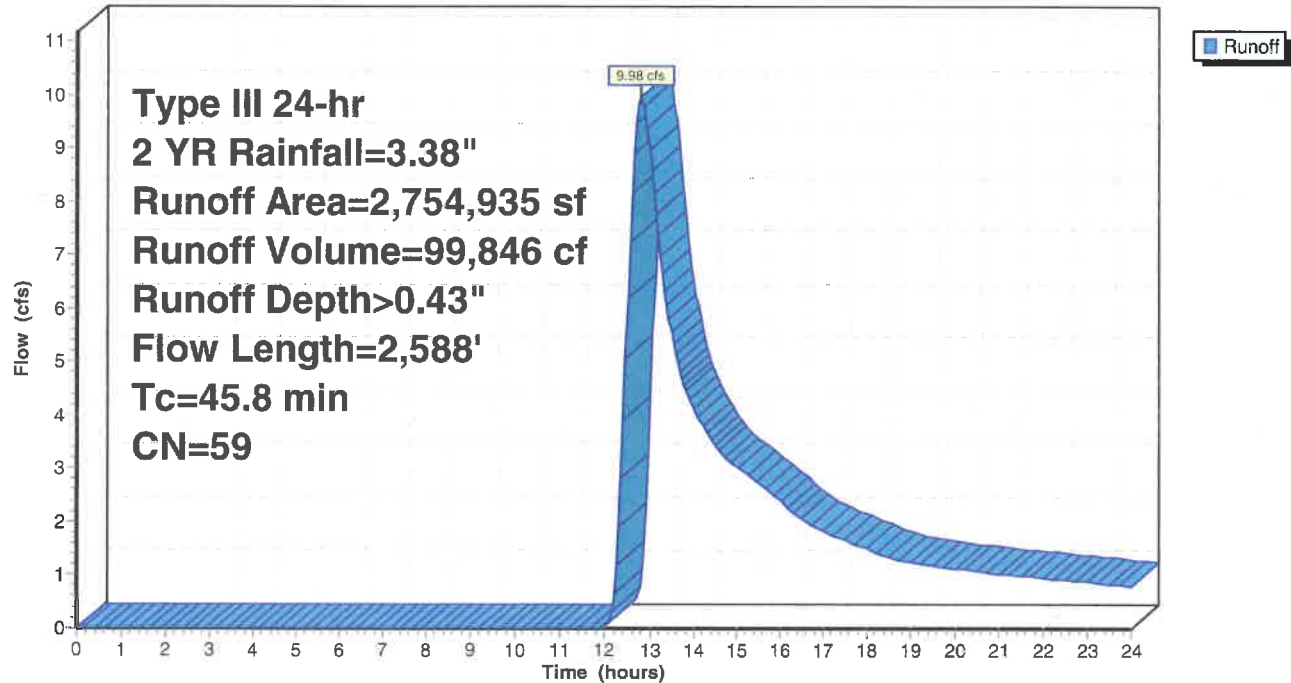
00454 - PROPOSED Conditions
Type III 24-hr 2 YR Rainfall=3.38"

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Subcatchment 2S: To Proposed Culvert

Hydrograph



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Summary for Subcatchment 3S: To 8" pipes

Runoff = 0.85 cfs @ 12.54 hrs, Volume= 8,039 cf, Depth> 0.30"

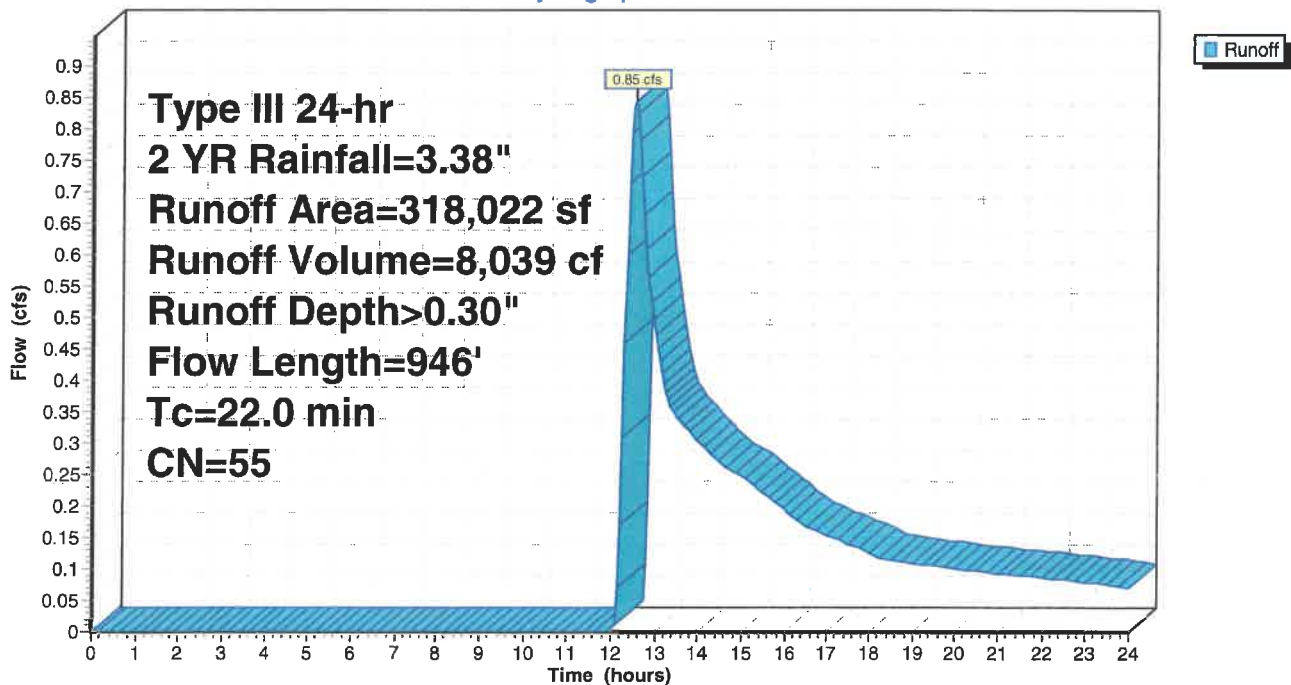
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 318,022 | 55 | Woods, Good, HSG B |
| 318,022 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Sheet Flow |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.22" |
| 14.9 | 896 | 0.0401 | 1.00 | | Shallow Concentrated Flow, |
| | | | | | Woodland Kv= 5.0 fps |
| 22.0 | 946 | Total | | | |

Subcatchment 3S: To 8" pipes

Hydrograph



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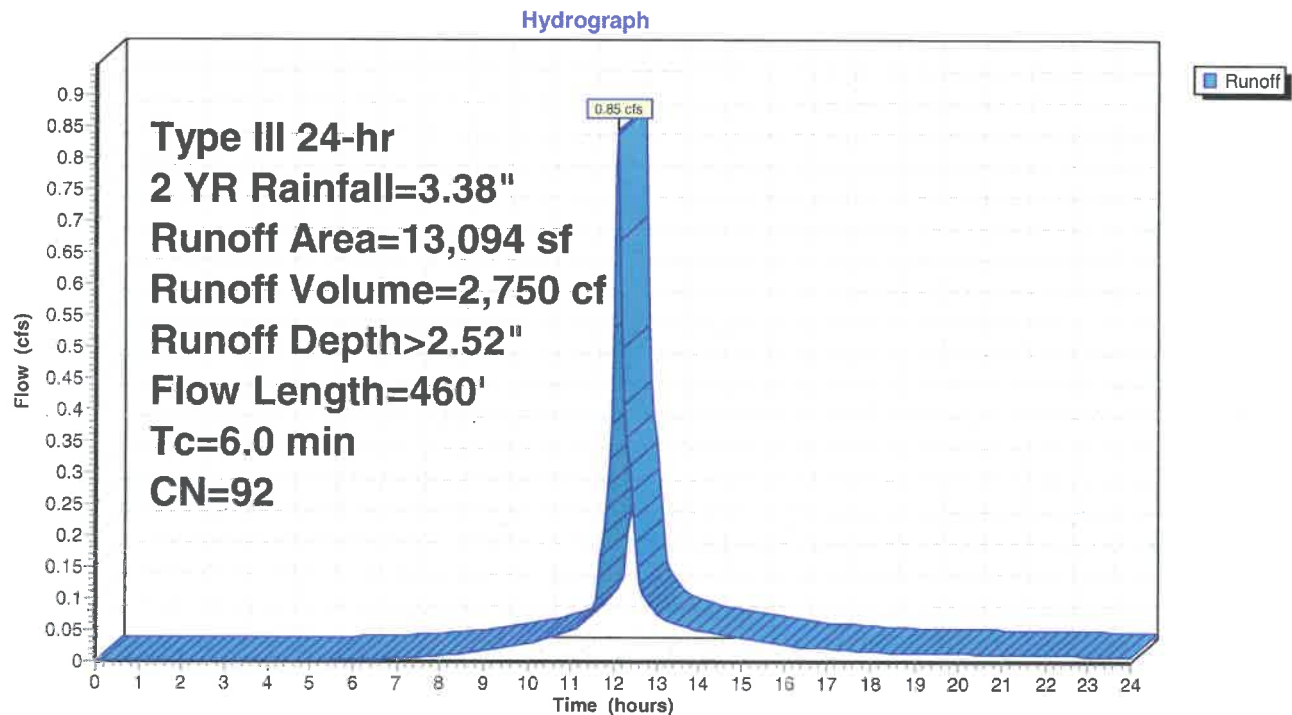
Summary for Subcatchment 4S: To CB 1

Runoff = 0.85 cfs @ 12.09 hrs, Volume= 2,750 cf, Depth> 2.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 11,086 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,008 | 61 | >75% Grass cover, Good, HSG B |
| 13,094 | 92 | Weighted Average |
| 2,008 | | 15.34% Pervious Area |
| 11,086 | | 84.66% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 4S: To CB 1

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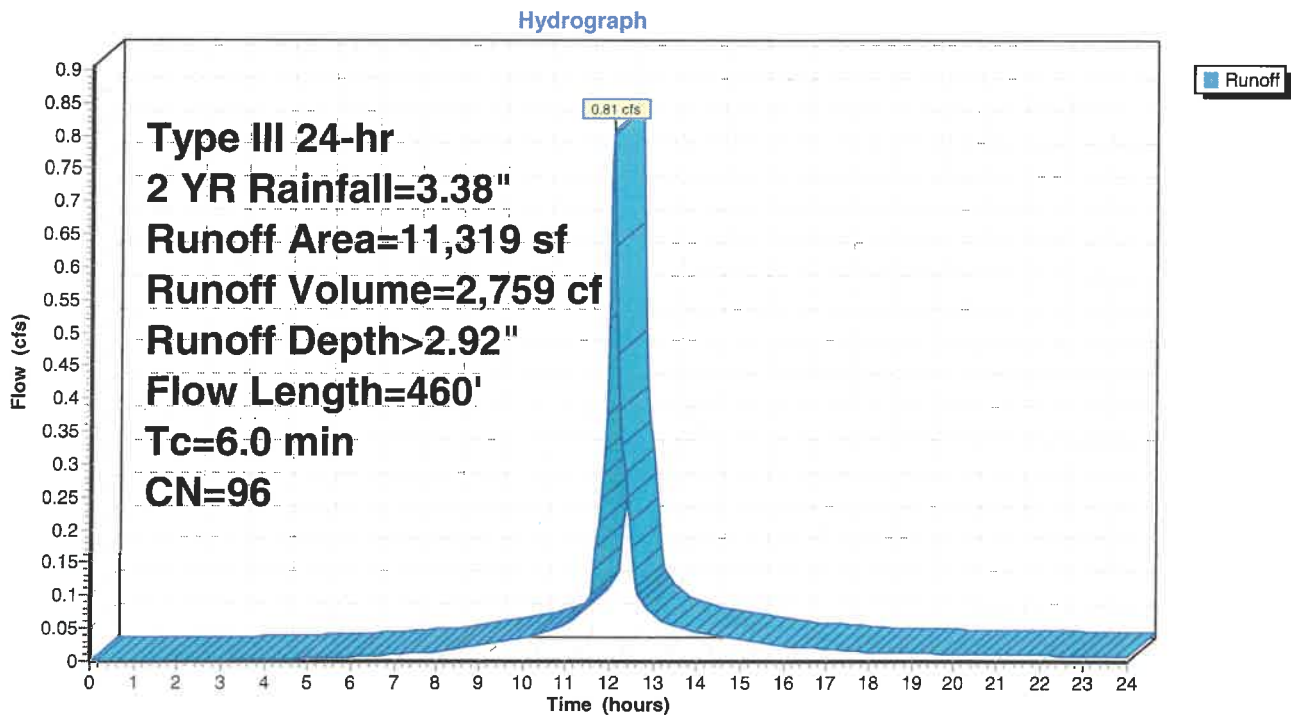
Summary for Subcatchment 5S: To CB 2

Runoff = 0.81 cfs @ 12.09 hrs, Volume= 2,759 cf, Depth> 2.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 10,719 | 98 | Paved roads w/curbs & sewers, HSG B |
| 600 | 61 | >75% Grass cover, Good, HSG B |
| 11,319 | 96 | Weighted Average |
| 600 | | 5.30% Pervious Area |
| 10,719 | | 94.70% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 5S: To CB 2

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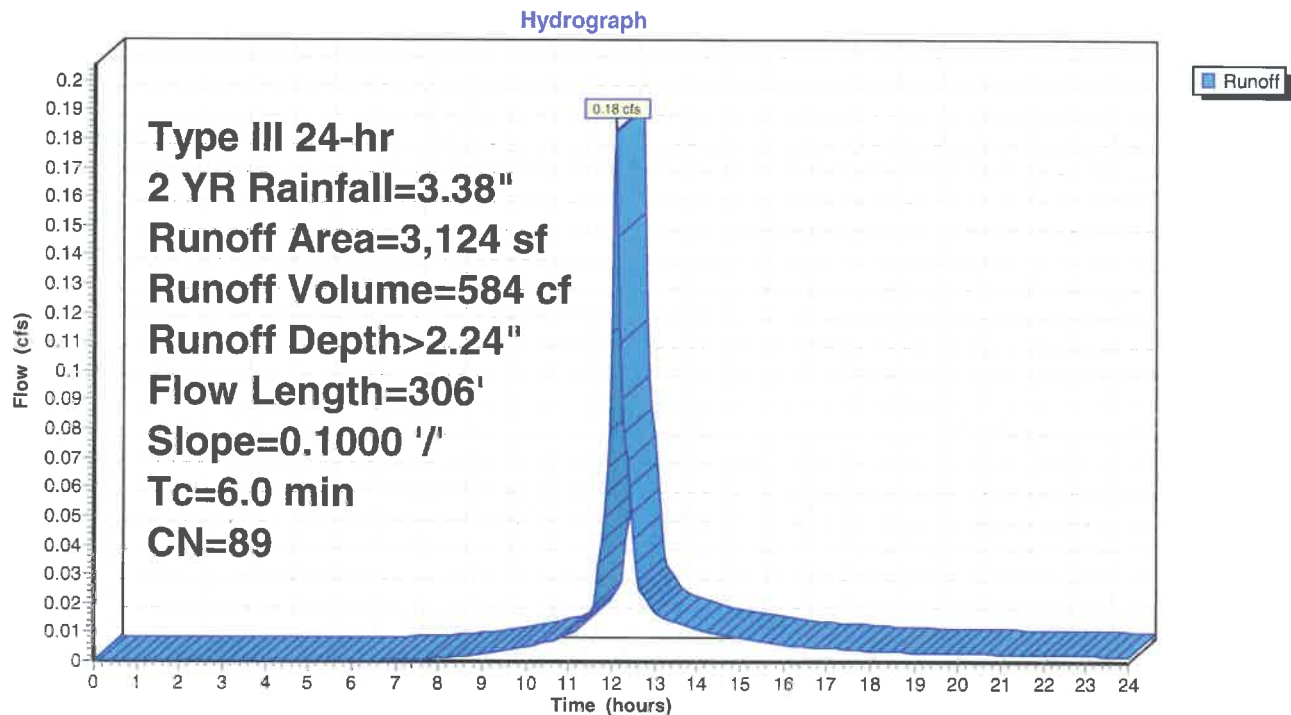
Summary for Subcatchment 6S: To CB 3

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 584 cf, Depth> 2.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,343 | 98 | Paved roads w/curbs & sewers, HSG B |
| 781 | 61 | >75% Grass cover, Good, HSG B |
| 3,124 | 89 | Weighted Average |
| 781 | | 25.00% Pervious Area |
| 2,343 | | 75.00% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 6S: To CB 3

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Summary for Subcatchment 7S: To CB 4

Runoff = 0.15 cfs @ 12.09 hrs, Volume= 477 cf, Depth> 2.24"

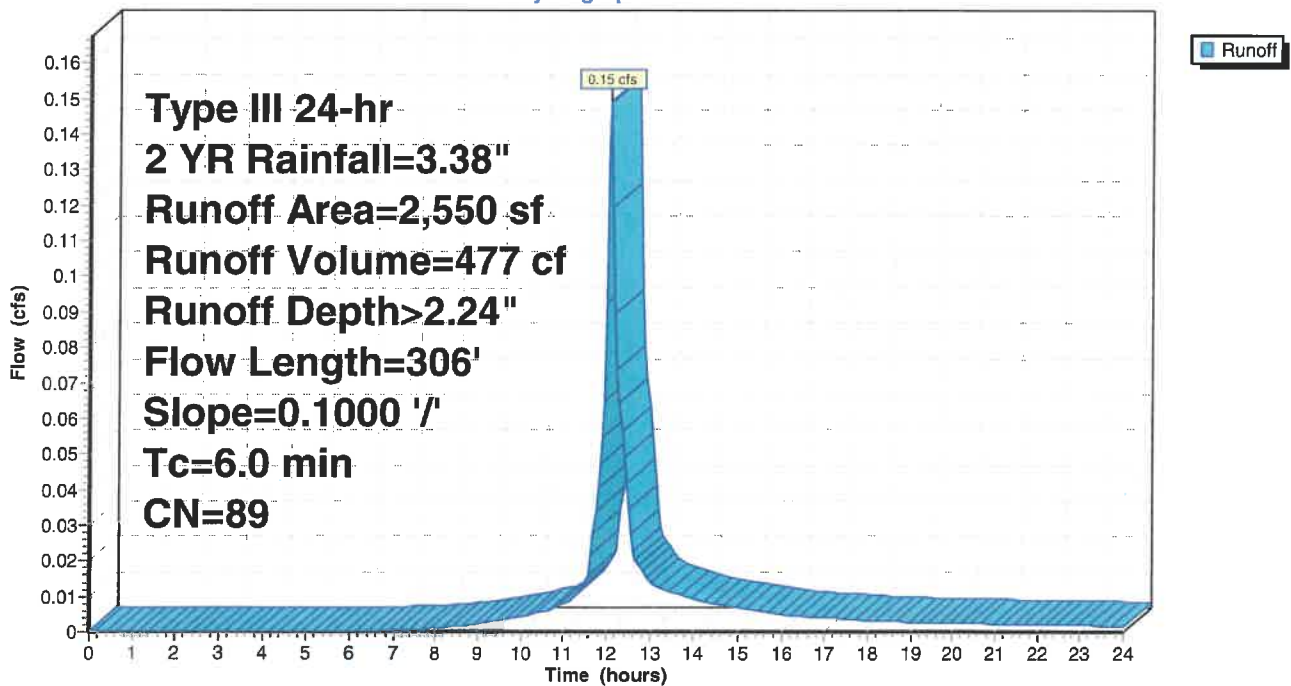
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 1,913 | 98 | Paved roads w/curbs & sewers, HSG B |
| 637 | 61 | >75% Grass cover, Good, HSG B |
| 2,550 | 89 | Weighted Average |
| 637 | | 24.98% Pervious Area |
| 1,913 | | 75.02% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 7S: To CB 4

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Summary for Subcatchment 8S: To CB 6

Runoff = 0.19 cfs @ 12.09 hrs, Volume= 612 cf, Depth> 1.99"

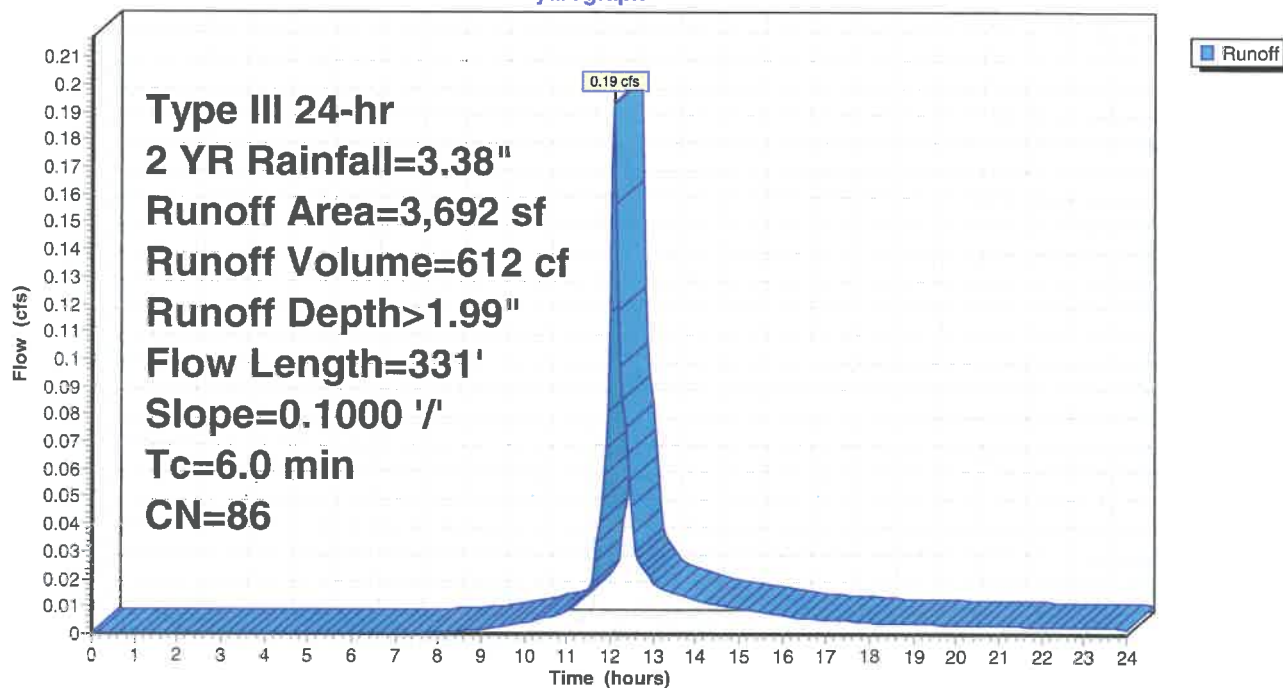
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,493 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,199 | 61 | >75% Grass cover, Good, HSG B |
| 3,692 | 86 | Weighted Average |
| 1,199 | | 32.48% Pervious Area |
| 2,493 | | 67.52% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 281 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 331 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 8S: To CB 6

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Summary for Subcatchment 9S: To CB 5

Runoff = 0.23 cfs @ 12.09 hrs, Volume= 762 cf, Depth> 2.62"

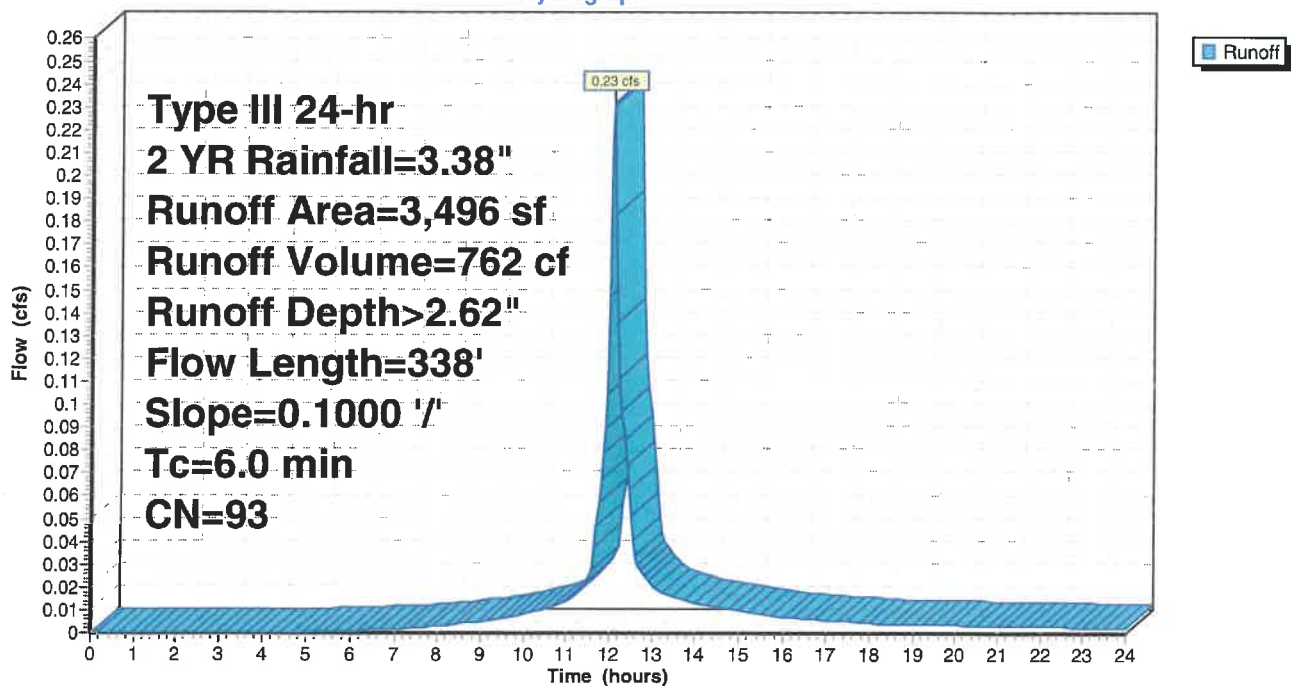
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 3,000 | 98 | Paved roads w/curbs & sewers, HSG B |
| 496 | 61 | >75% Grass cover, Good, HSG B |
| 3,496 | 93 | Weighted Average |
| 496 | | 14.19% Pervious Area |
| 3,000 | | 85.81% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 288 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 338 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 9S: To CB 5

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Summary for Subcatchment 10S: To CB 7

Runoff = 0.23 cfs @ 12.09 hrs, Volume= 722 cf, Depth> 1.99"

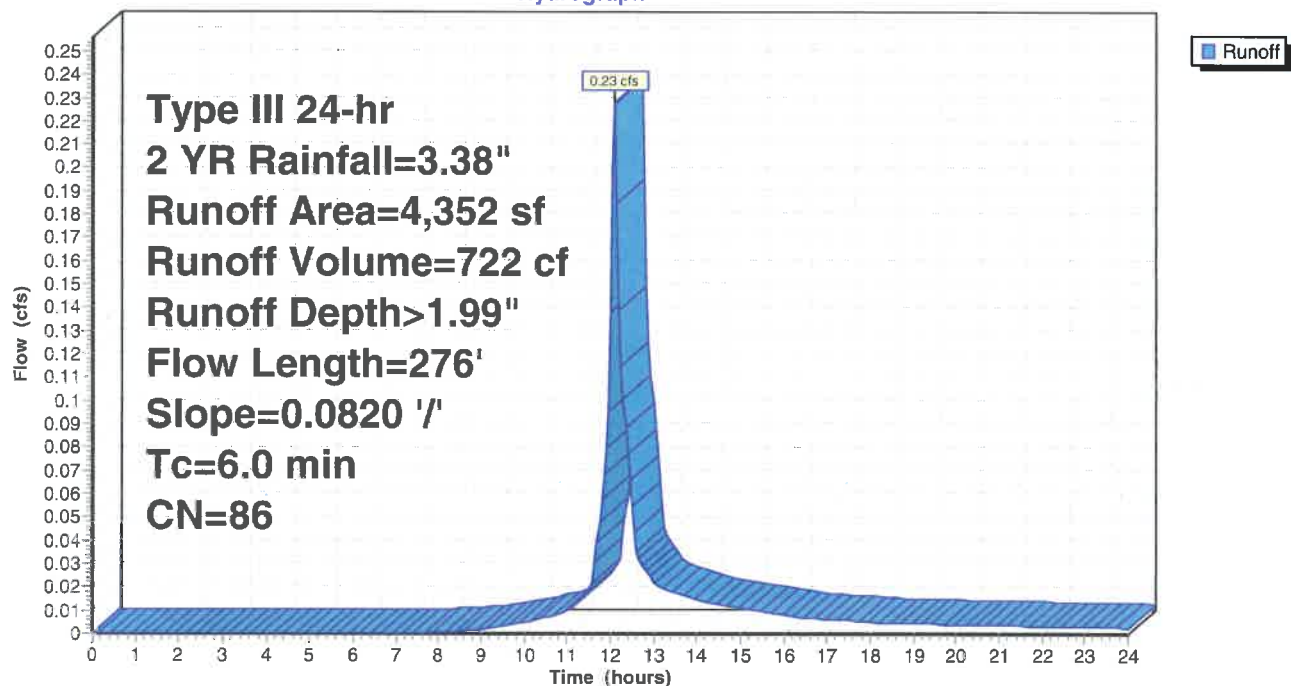
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,363 | 61 | >75% Grass cover, Good, HSG B |
| 4,352 | 86 | Weighted Average |
| 1,363 | | 31.32% Pervious Area |
| 2,989 | | 68.68% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.6 | 226 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.0 | 276 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 10S: To CB 7

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Summary for Subcatchment 11S: To CB 8

Runoff = 0.23 cfs @ 12.09 hrs, Volume= 733 cf, Depth> 2.16"

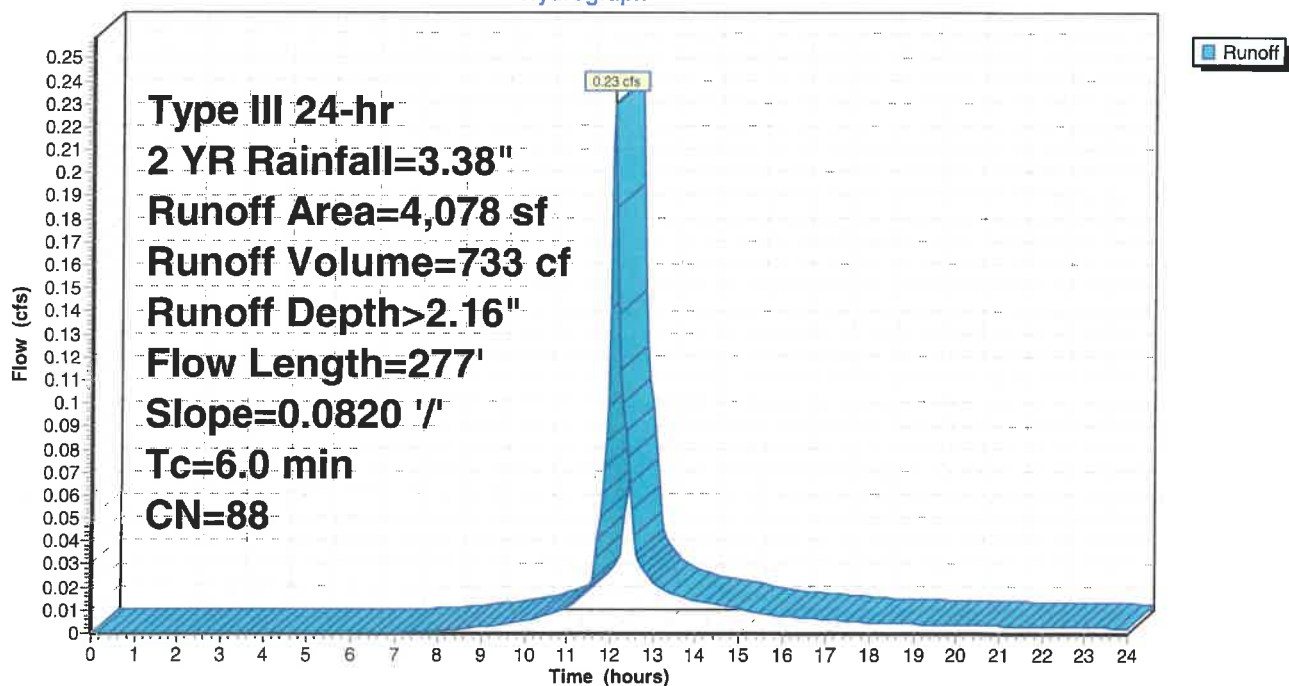
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,089 | 61 | >75% Grass cover, Good, HSG B |
| 4,078 | 88 | Weighted Average |
| 1,089 | | 26.70% Pervious Area |
| 2,989 | | 73.30% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 227 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 277 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 11S: To CB 8

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Summary for Subcatchment 12S: To CB 9

Runoff = 0.66 cfs @ 12.09 hrs, Volume= 2,069 cf, Depth> 1.83"

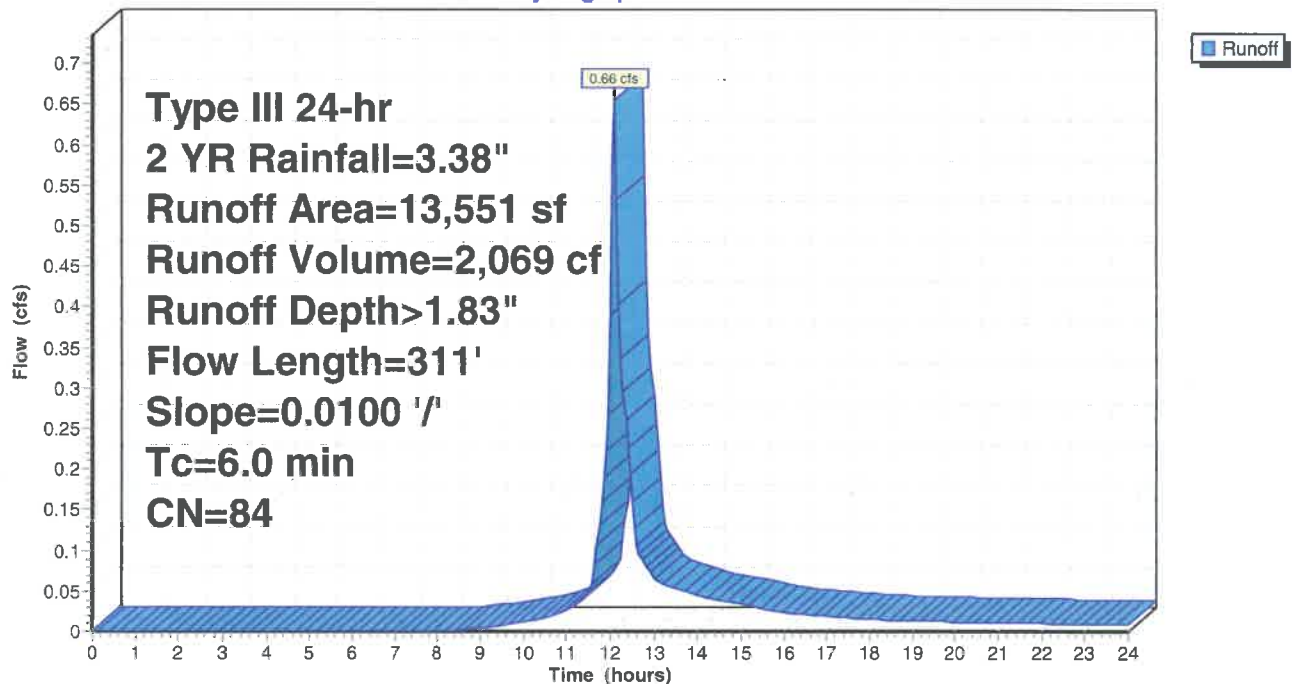
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| 5,218 | 61 | >75% Grass cover, Good, HSG B |
| 13,551 | 84 | Weighted Average |
| 5,218 | | 38.51% Pervious Area |
| 8,333 | | 61.49% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.1 | 261 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.0 | 311 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 12S: To CB 9

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Summary for Subcatchment 13S: To CB 10

Runoff = 0.66 cfs @ 12.09 hrs, Volume= 2,074 cf, Depth> 1.68"

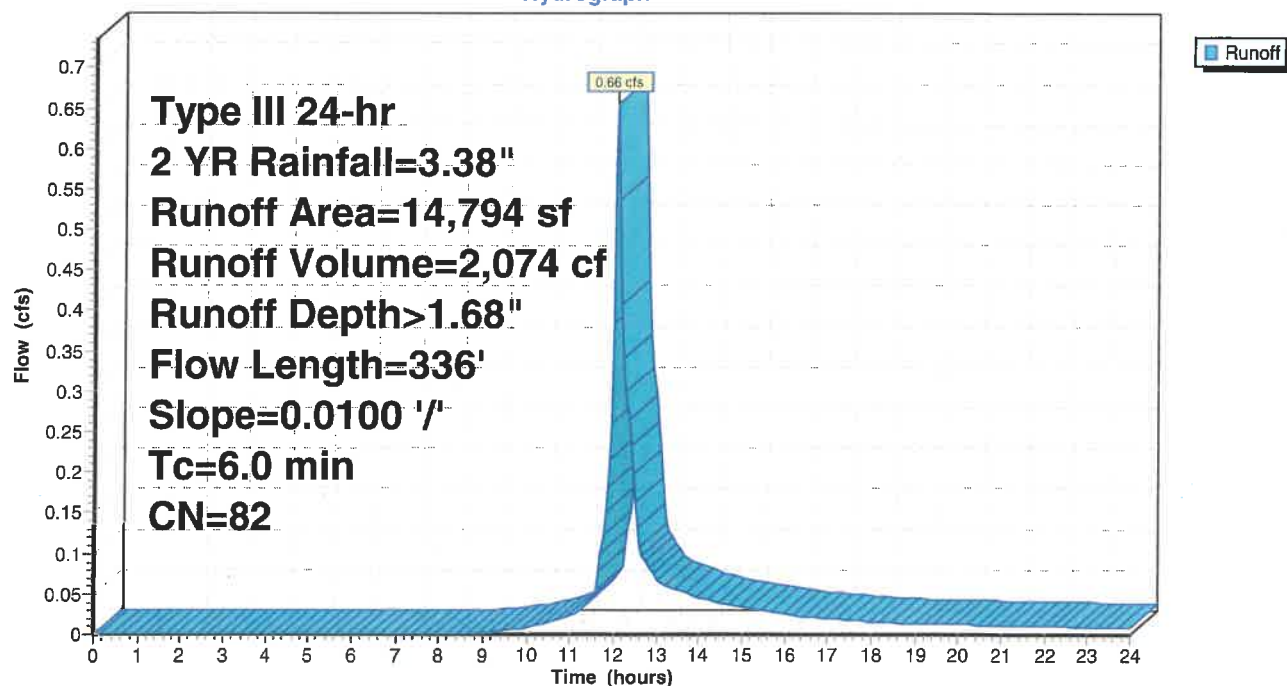
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| | Area (sf) | CN | Description |
|---|-----------|----|-------------------------------------|
| * | 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| | 6,461 | 61 | >75% Grass cover, Good, HSG B |
| | 14,794 | 82 | Weighted Average |
| | 6,461 | | 43.67% Pervious Area |
| | 8,333 | | 56.33% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.3 | 286 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.2 | 336 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 13S: To CB 10

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Summary for Subcatchment 14S: To Wetland

Runoff = 5.73 cfs @ 12.42 hrs, Volume= 34,852 cf, Depth> 0.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

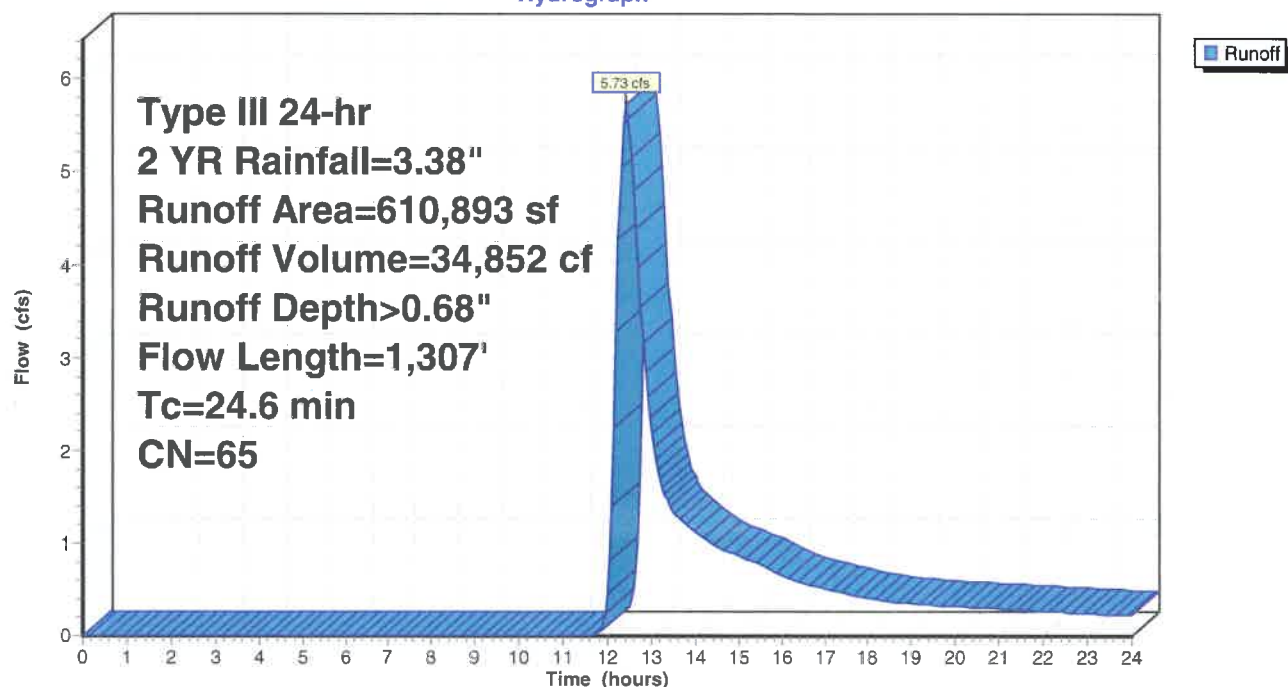
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 326,808 | 55 | Woods, Good, HSG B |
| 260,659 | 77 | Woods, Good, HSG D |
| 23,426 | 61 | >75% Grass cover, Good, HSG B |
| 610,893 | 65 | Weighted Average |
| 610,893 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 7.9 | 648 | 0.0750 | 1.37 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 6.9 | 609 | 0.0200 | 1.47 | 1.76 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 24.6 | 1,307 | Total | | | |

Subcatchment 14S: To Wetland

Hydrograph



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Summary for Subcatchment 15S: TO BASIN

Runoff = 0.39 cfs @ 12.39 hrs, Volume= 3,126 cf, Depth> 0.34"

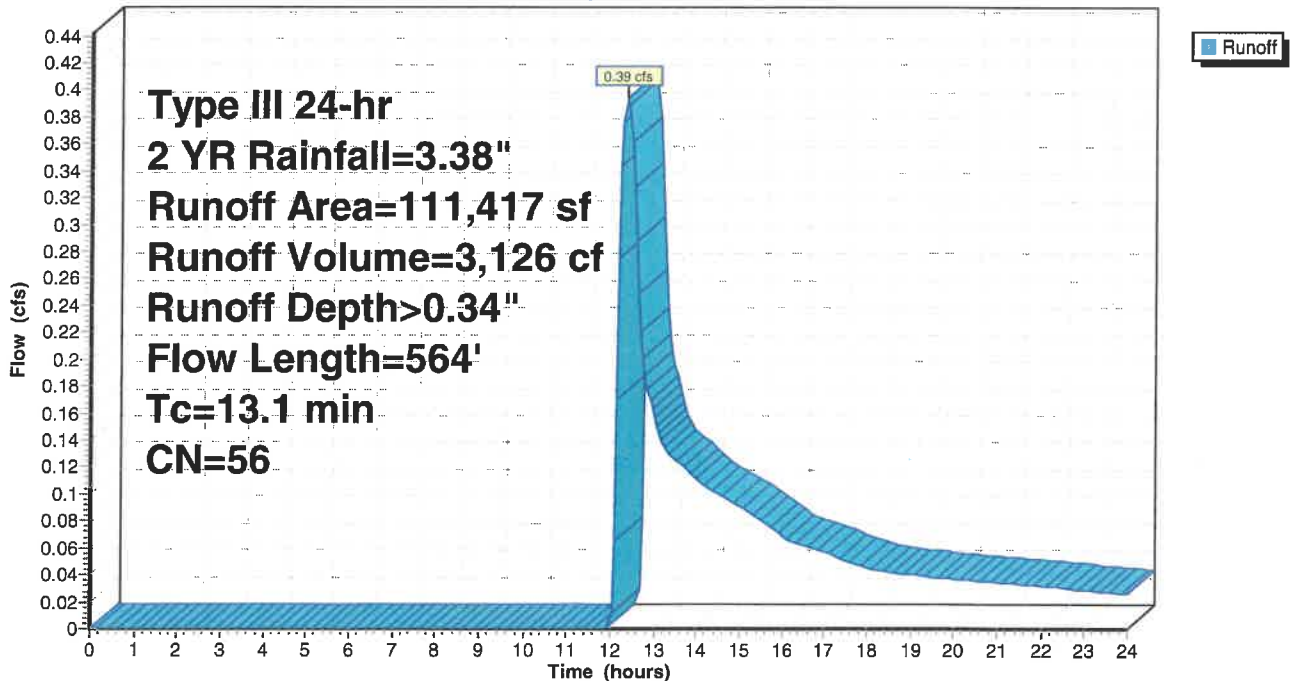
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 89,974 | 55 | Woods, Good, HSG B |
| 21,443 | 61 | >75% Grass cover, Good, HSG B |
| 111,417 | 56 | Weighted Average |
| 111,417 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.8 | 50 | 0.0620 | 0.11 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 5.1 | 467 | 0.0931 | 1.53 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.2 | 47 | 0.3190 | 3.95 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 13.1 | 564 | Total | | | |

Subcatchment 15S: TO BASIN

Hydrograph



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Summary for Subcatchment 16S: TO BASIN

Runoff = 0.39 cfs @ 12.28 hrs, Volume= 2,685 cf, Depth> 0.41"

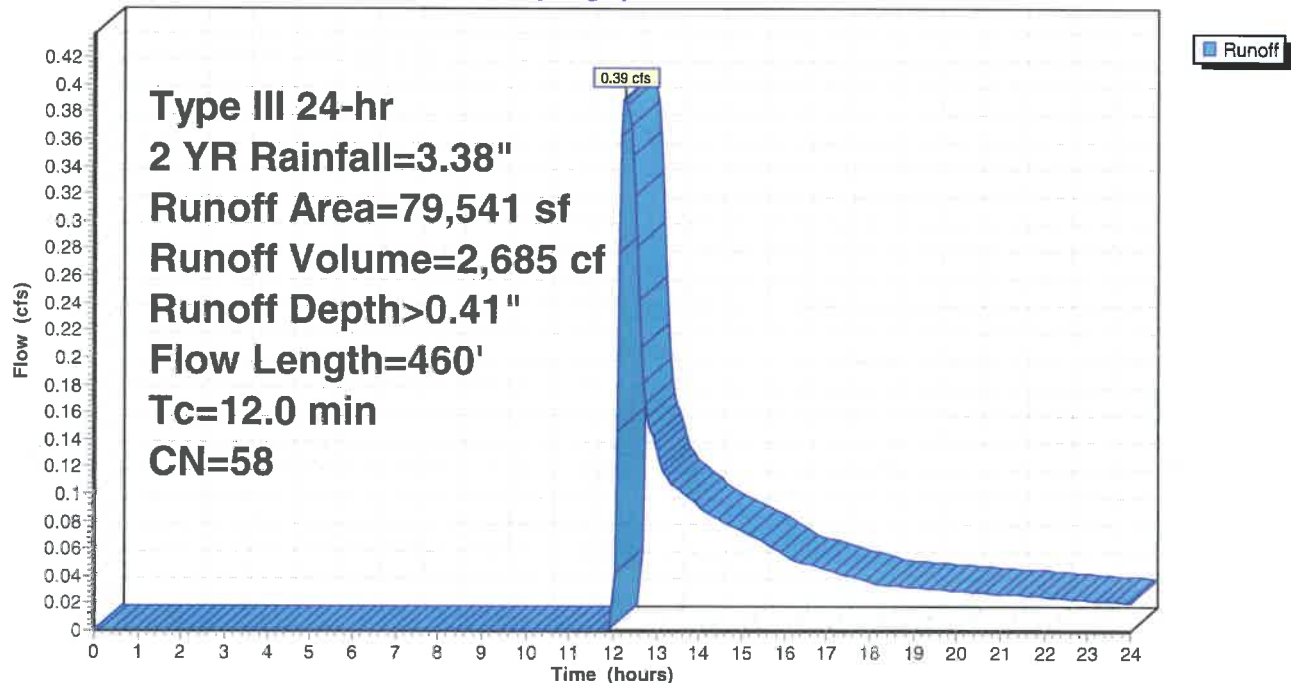
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 35,427 | 61 | >75% Grass cover, Good, HSG B |
| 44,114 | 55 | Woods, Good, HSG B |
| 79,541 | 58 | Weighted Average |
| 79,541 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 4.8 | 391 | 0.0735 | 1.36 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.1 | 19 | 0.3150 | 5.05 | | Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps |
| 12.0 | 460 | Total | | | |

Subcatchment 16S: TO BASIN

Hydrograph



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Summary for Subcatchment 17S: To Off Site

Runoff = 0.55 cfs @ 12.38 hrs, Volume= 4,554 cf, Depth> 0.30"

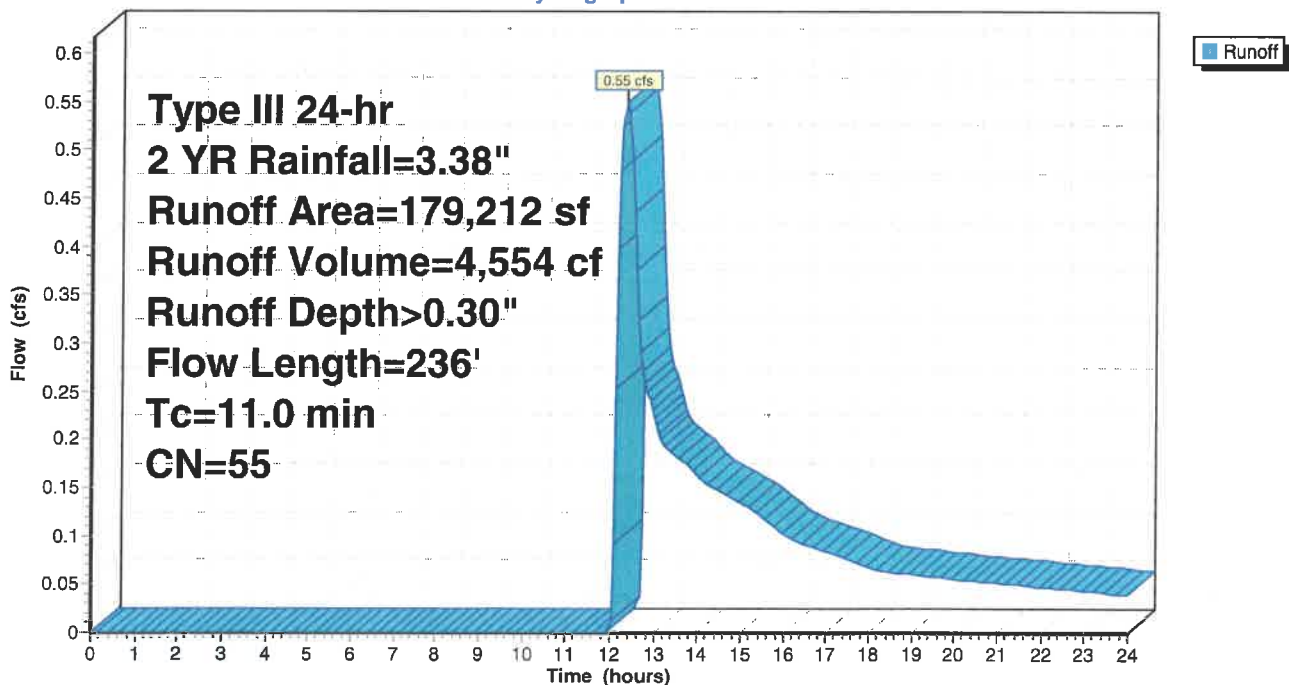
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 179,212 | 55 | Woods, Good, HSG B |
| 179,212 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 8.5 | 50 | 0.0500 | 0.10 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.5 | 186 | 0.0600 | 1.22 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 11.0 | 236 | Total | | | |

Subcatchment 17S: To Off Site

Hydrograph



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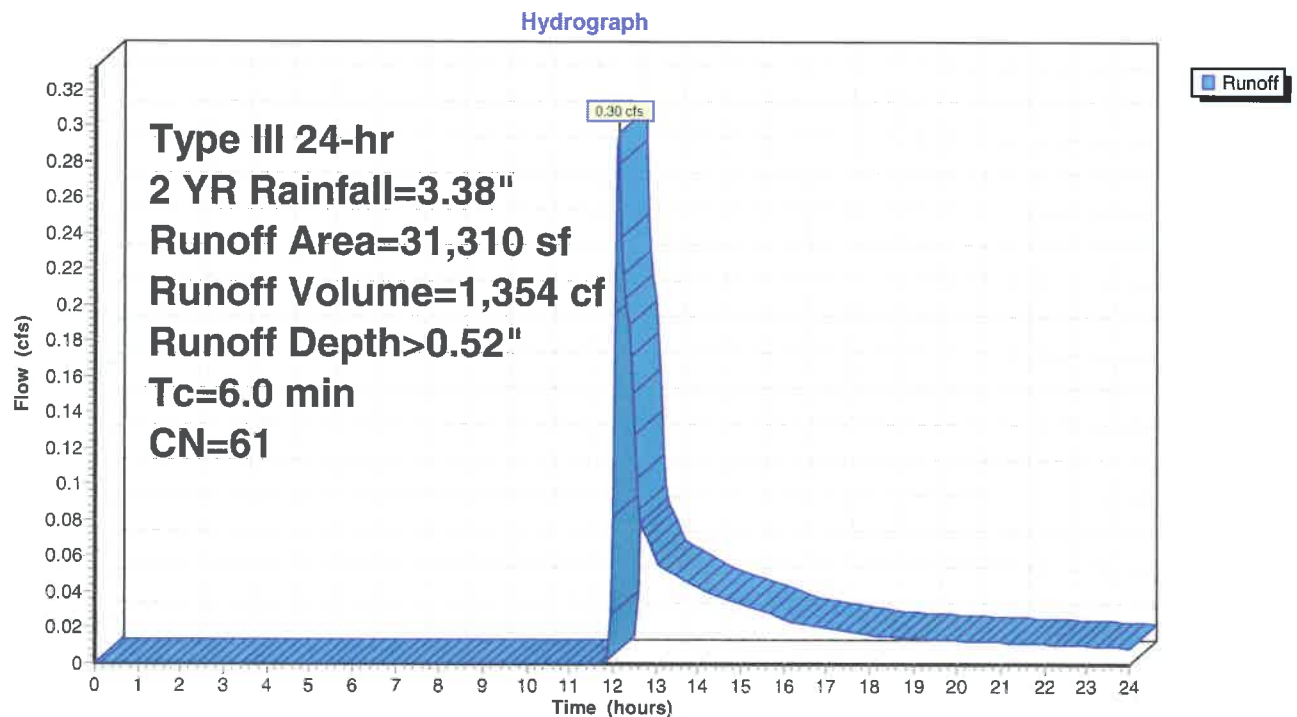
Summary for Subcatchment 19S: Area To Basin3

Runoff = 0.30 cfs @ 12.12 hrs, Volume= 1,354 cf, Depth> 0.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 2 YR Rainfall=3.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 31,310 | 61 | >75% Grass cover, Good, HSG B |
| 31,310 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|-------------------|
| 6.0 | | | | | Direct Entry, a-b |

Subcatchment 19S: Area To Basin3

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Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 0.51" for 2 YR event
Inflow = 2.90 cfs @ 12.51 hrs, Volume= 21,126 cf
Outflow = 2.90 cfs @ 12.51 hrs, Volume= 21,118 cf, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.55 fps, Min. Travel Time= 0.2 min
Avg. Velocity= 2.07 fps, Avg. Travel Time= 0.5 min

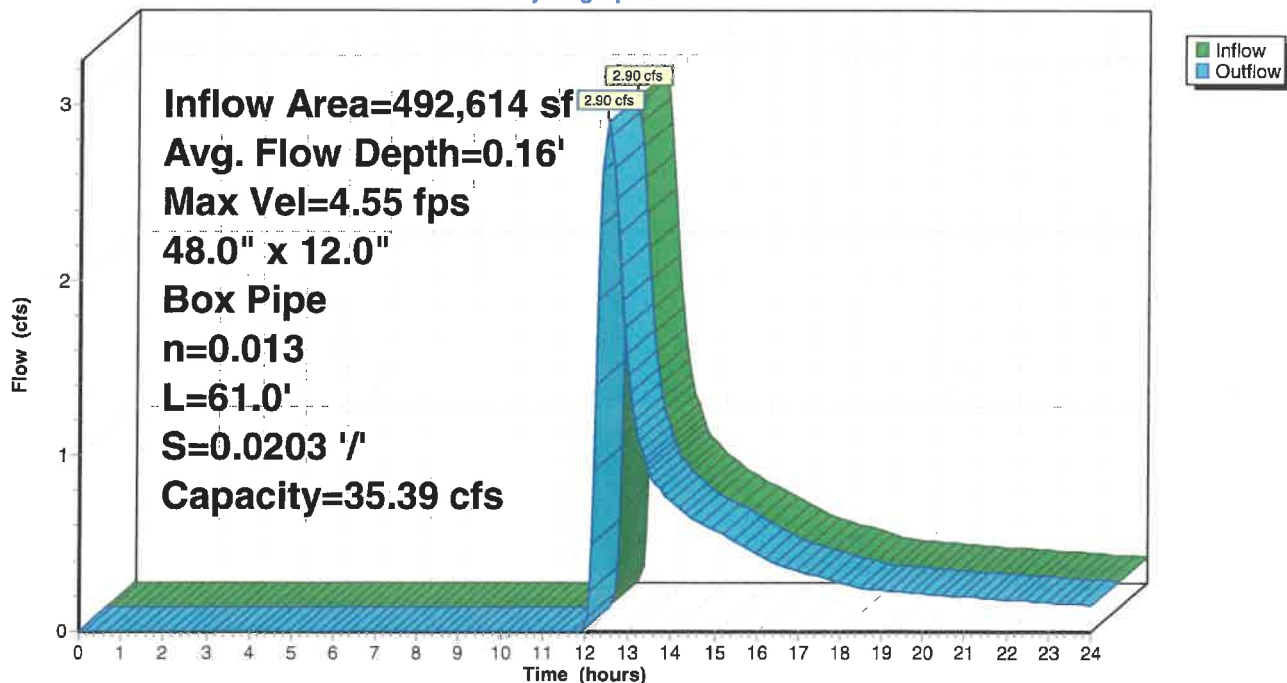
Peak Storage= 39 cf @ 12.51 hrs
Average Depth at Peak Storage= 0.16' , Surface Width= 4.00'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe
n= 0.013 Concrete, trowel finish
Length= 61.0' Slope= 0.0203 '/'
Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

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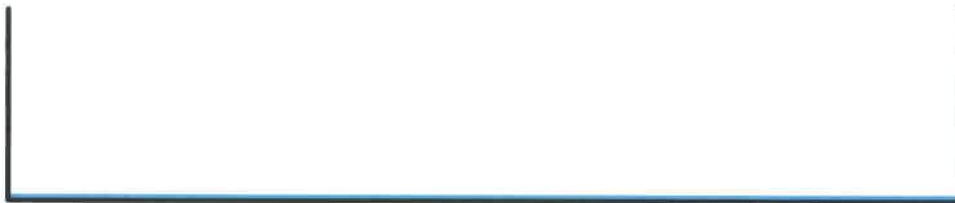
Summary for Reach 2R: New Box Cumvert

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 0.25" for 2 YR event
Inflow = 2.78 cfs @ 15.44 hrs, Volume= 57,114 cf
Outflow = 2.78 cfs @ 15.42 hrs, Volume= 57,097 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.24 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 3.32 fps, Avg. Travel Time= 0.2 min

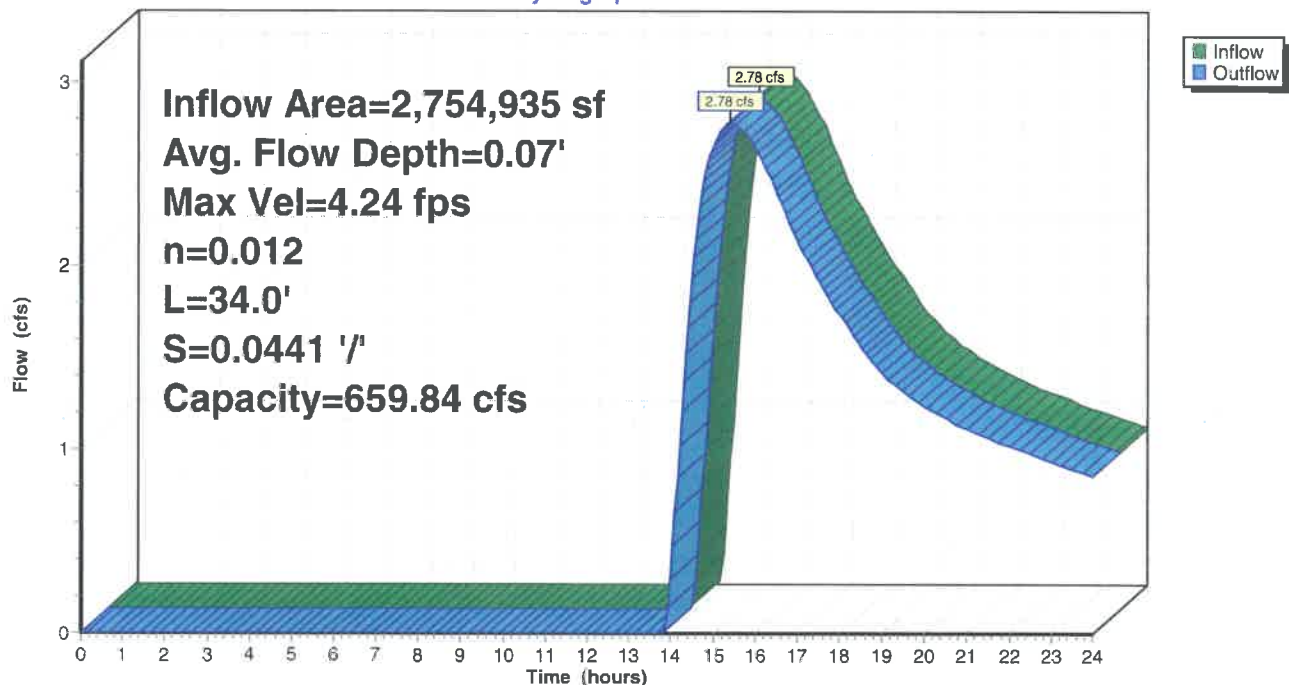
Peak Storage= 22 cf @ 15.42 hrs
Average Depth at Peak Storage= 0.07' , Surface Width= 10.00'
Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 659.84 cfs

10.00' x 2.00' deep channel, n= 0.012 Concrete pipe, finished
Length= 34.0' Slope= 0.0441 '/'
Inlet Invert= 294.00', Outlet Invert= 292.50'



Reach 2R: New Box Cumvert

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Summary for Reach 3R: CB1 to DMH 1

Inflow Area = 13,094 sf, 84.66% Impervious, Inflow Depth > 2.52" for 2 YR event
Inflow = 0.85 cfs @ 12.09 hrs, Volume= 2,750 cf
Outflow = 0.85 cfs @ 12.09 hrs, Volume= 2,750 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.64 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.22 fps, Avg. Travel Time= 0.1 min

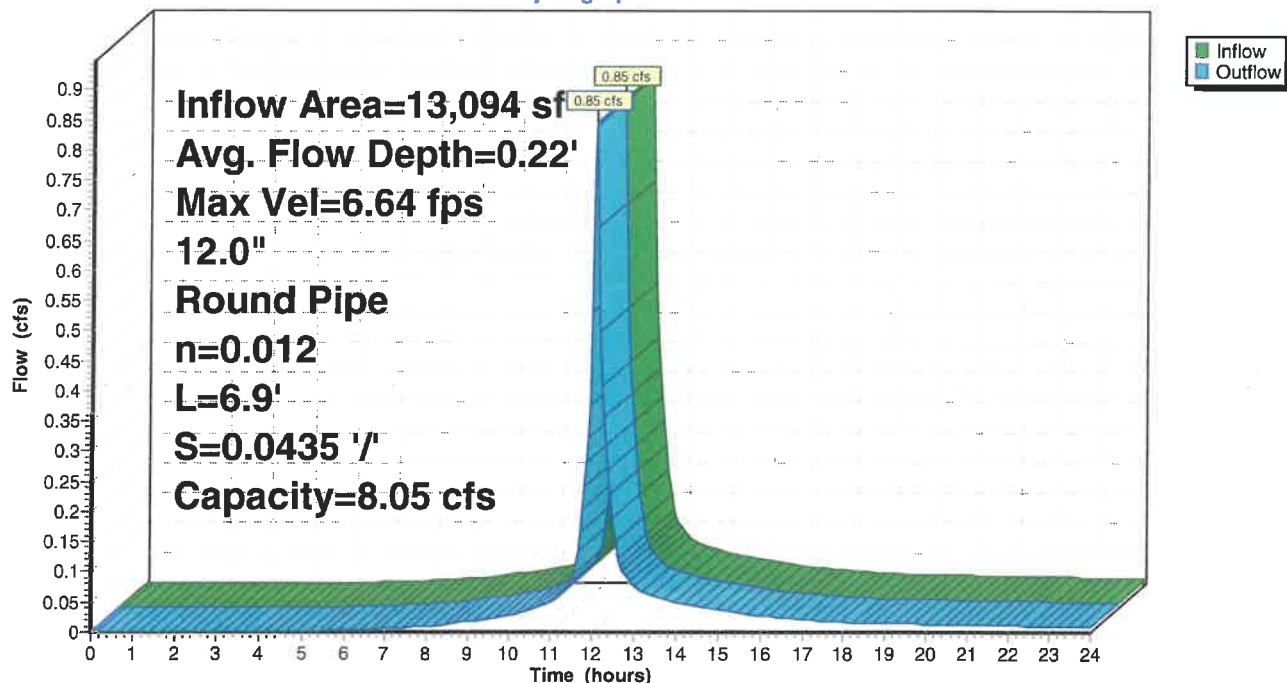
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.22', Surface Width= 0.83'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 3R: CB1 to DMH 1

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Summary for Reach 4R: CB2 to DMH 1

Inflow Area = 11,319 sf, 94.70% Impervious, Inflow Depth > 2.92" for 2 YR event
Inflow = 0.81 cfs @ 12.09 hrs, Volume= 2,759 cf
Outflow = 0.81 cfs @ 12.09 hrs, Volume= 2,759 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.54 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.16 fps, Avg. Travel Time= 0.1 min

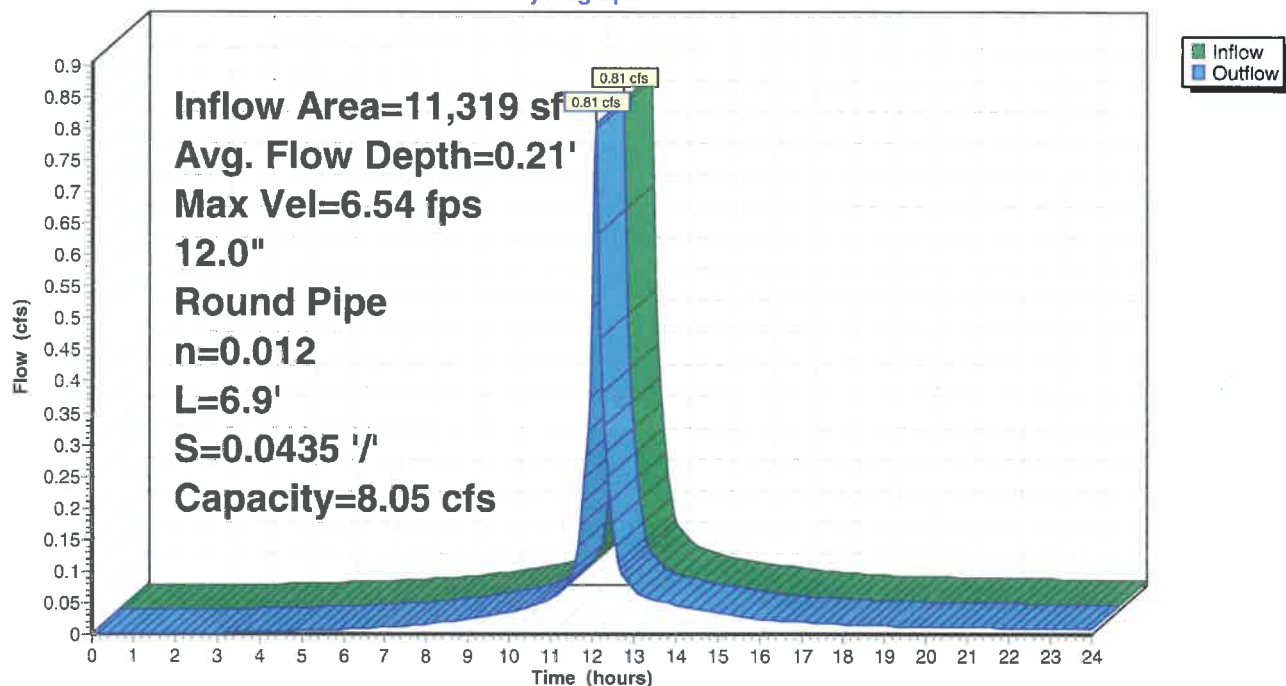
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.21' , Surface Width= 0.82'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 4R: CB2 to DMH 1

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Summary for Reach 5R: DMH1 to DMH 2

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 2.71" for 2 YR event
Inflow = 1.65 cfs @ 12.09 hrs, Volume= 5,509 cf
Outflow = 1.64 cfs @ 12.10 hrs, Volume= 5,507 cf, Atten= 1%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.57 fps, Min. Travel Time= 0.3 min

Avg. Velocity = 1.47 fps, Avg. Travel Time= 0.9 min

Peak Storage= 28 cf @ 12.09 hrs

Average Depth at Peak Storage= 0.39' , Surface Width= 1.31'

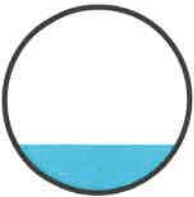
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 11.30 cfs

18.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

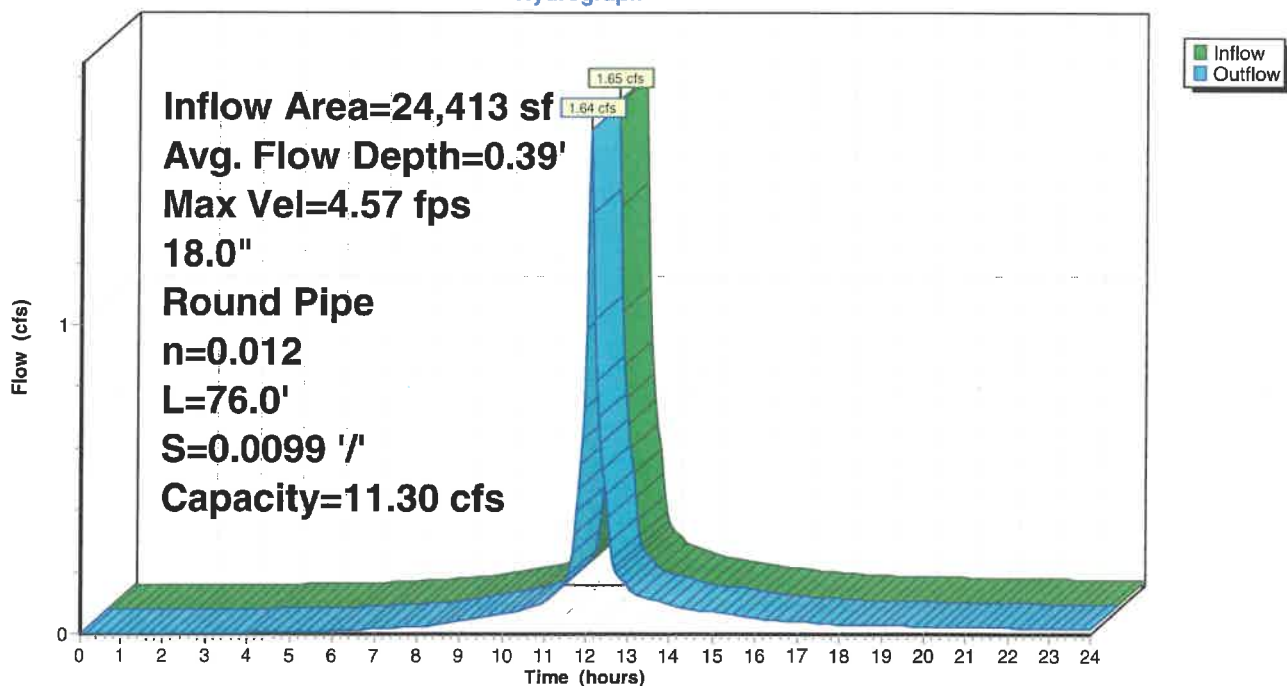
Length= 76.0' Slope= 0.0099 1'

Inlet Invert= 291.75', Outlet Invert= 291.00'



Reach 5R: DMH1 to DMH 2

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Summary for Reach 6R: CB3 to DMH 2

Inflow Area = 3,124 sf, 75.00% Impervious, Inflow Depth > 2.24" for 2 YR event
Inflow = 0.18 cfs @ 12.09 hrs, Volume= 584 cf
Outflow = 0.18 cfs @ 12.09 hrs, Volume= 584 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.34 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 1.52 fps, Avg. Travel Time= 0.1 min

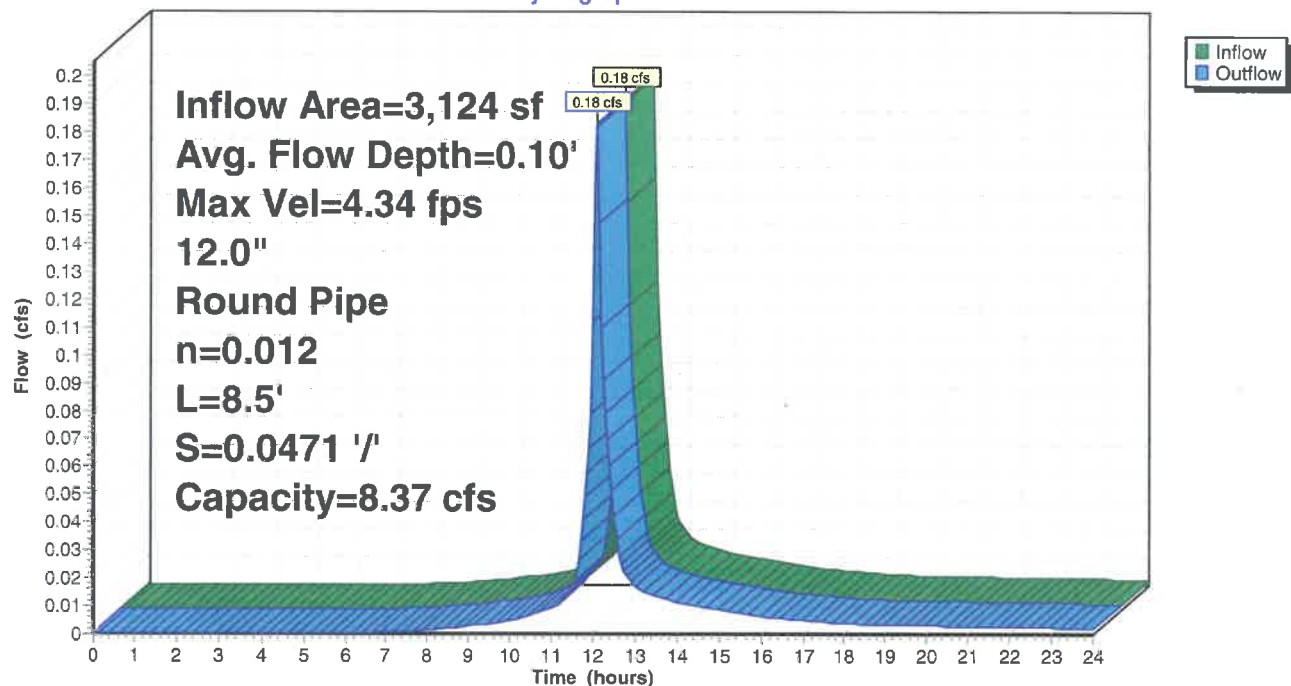
Peak Storage= 0 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.10' , Surface Width= 0.61'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 8.5' Slope= 0.0471 '/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 6R: CB3 to DMH 2

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Summary for Reach 7R: CB4 to DMH 2

Inflow Area = 2,550 sf, 75.02% Impervious, Inflow Depth > 2.24" for 2 YR event
Inflow = 0.15 cfs @ 12.09 hrs, Volume= 477 cf
Outflow = 0.15 cfs @ 12.09 hrs, Volume= 477 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.43 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.20 fps, Avg. Travel Time= 0.2 min

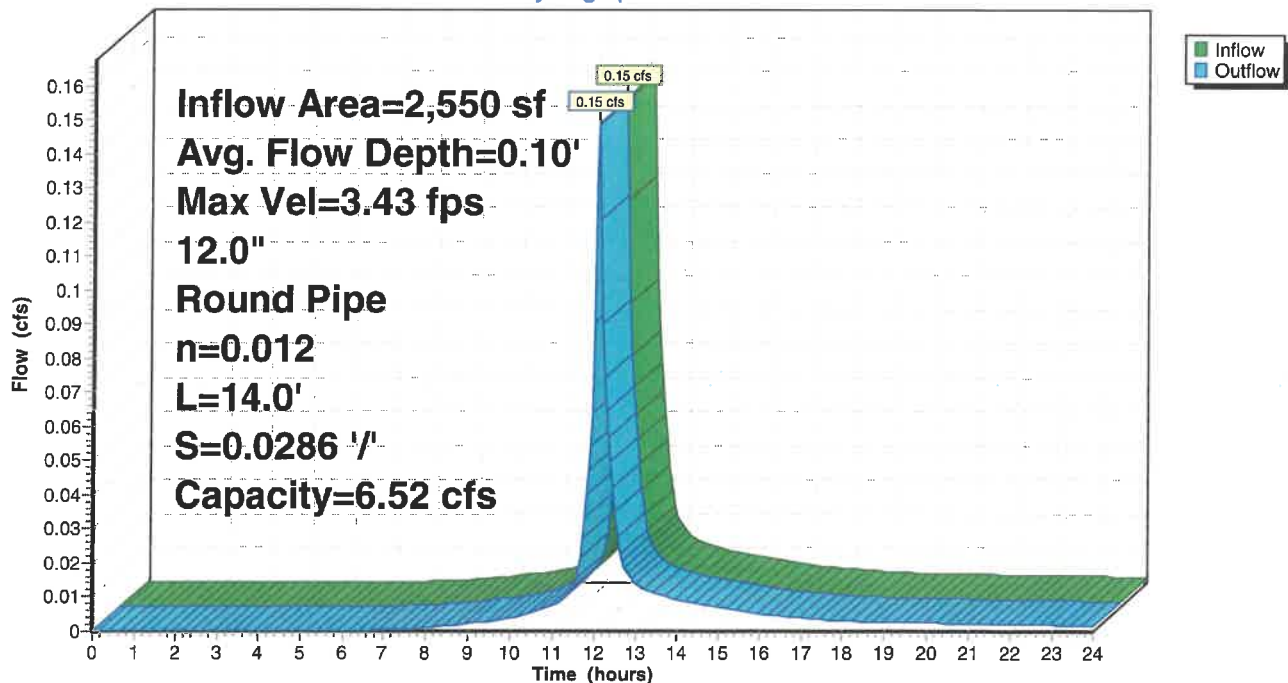
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.10', Surface Width= 0.61'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 6.52 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 14.0' Slope= 0.0286 1/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 7R: CB4 to DMH 2

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Summary for Reach 8R: DMH 2 TO DMH 7

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 2.27" for 2 YR event
Inflow = 0.75 cfs @ 12.10 hrs, Volume= 2,436 cf
Outflow = 0.71 cfs @ 12.12 hrs, Volume= 2,434 cf, Atten= 4%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.63 fps, Min. Travel Time= 0.9 min

Avg. Velocity = 1.53 fps, Avg. Travel Time= 2.6 min

Peak Storage= 38 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.22' , Surface Width= 1.06'

Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 16.09 cfs

18.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

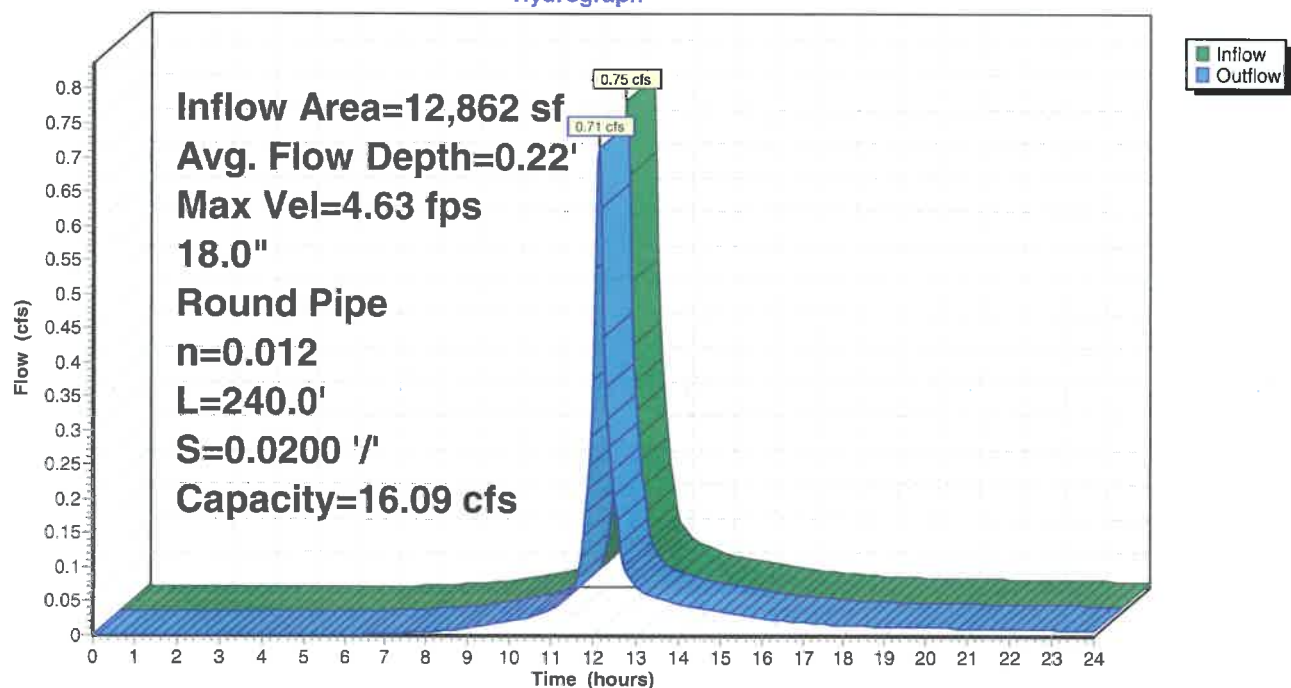
Length= 240.0' Slope= 0.0200 '/'

Inlet Invert= 294.80', Outlet Invert= 290.00'



Reach 8R: DMH 2 TO DMH 7

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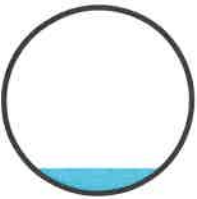
Summary for Reach 9R: DMH 3 to DMH 2

Inflow Area = 7,188 sf, 76.42% Impervious, Inflow Depth > 2.30" for 2 YR event
Inflow = 0.43 cfs @ 12.09 hrs, Volume= 1,375 cf
Outflow = 0.42 cfs @ 12.10 hrs, Volume= 1,374 cf, Atten= 2%, Lag= 0.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.73 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 2.23 fps, Avg. Travel Time= 1.6 min

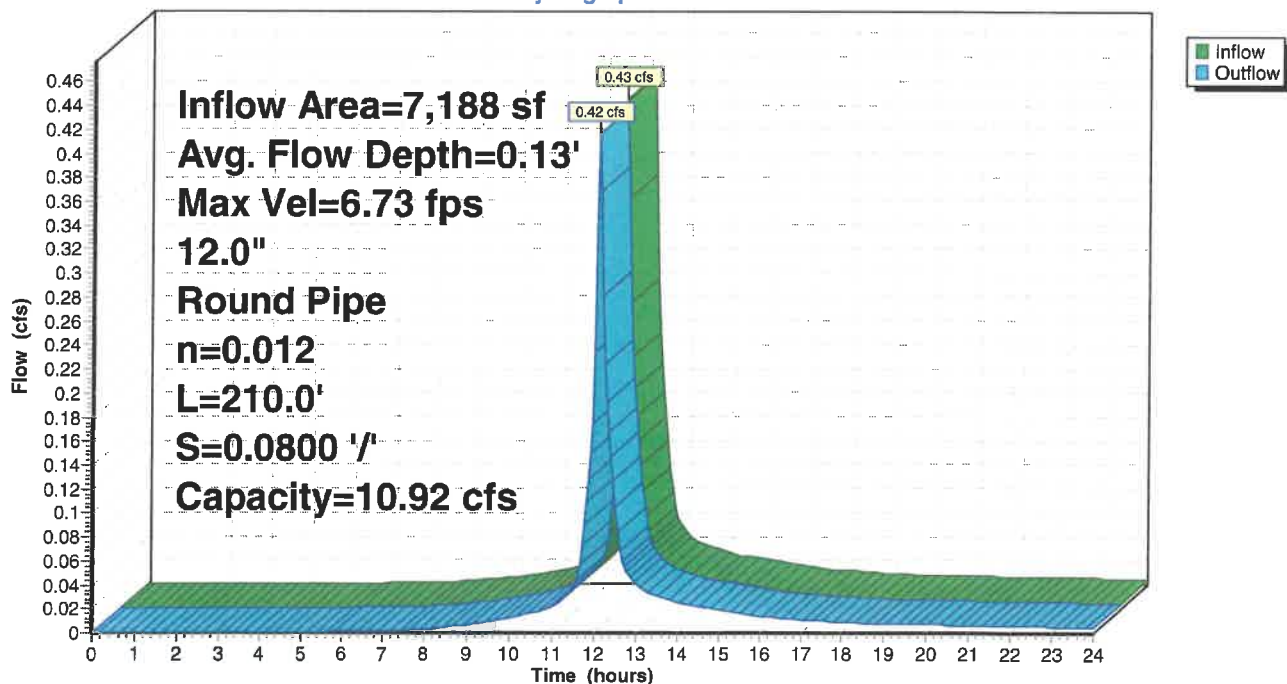
Peak Storage= 13 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.13', Surface Width= 0.68'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 210.0' Slope= 0.0800 1'
Inlet Invert= 315.40', Outlet Invert= 298.60'



Reach 9R: DMH 3 to DMH 2

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Summary for Reach 10R: CB5 to DMH 3

Inflow Area = 3,692 sf, 67.52% Impervious, Inflow Depth > 1.99" for 2 YR event
Inflow = 0.19 cfs @ 12.09 hrs, Volume= 612 cf
Outflow = 0.19 cfs @ 12.09 hrs, Volume= 612 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.31 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.54 fps, Avg. Travel Time= 0.1 min

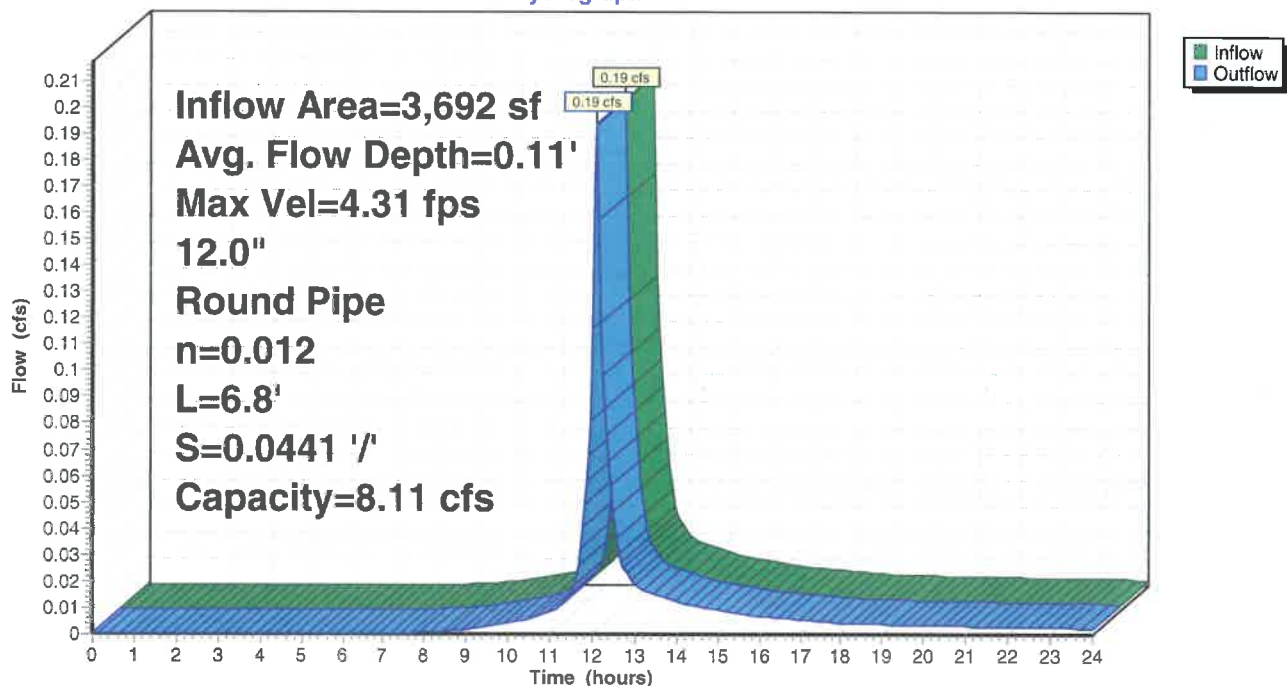
Peak Storage= 0 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.11', Surface Width= 0.62'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.11 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.8' Slope= 0.0441 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 10R: CB5 to DMH 3

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Summary for Reach 11R: CB6 to DMH 3

Inflow Area = 3,496 sf, 85.81% Impervious, Inflow Depth > 2.62" for 2 YR event
Inflow = 0.23 cfs @ 12.09 hrs, Volume= 762 cf
Outflow = 0.23 cfs @ 12.09 hrs, Volume= 762 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.59 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.20 fps, Avg. Travel Time= 0.2 min

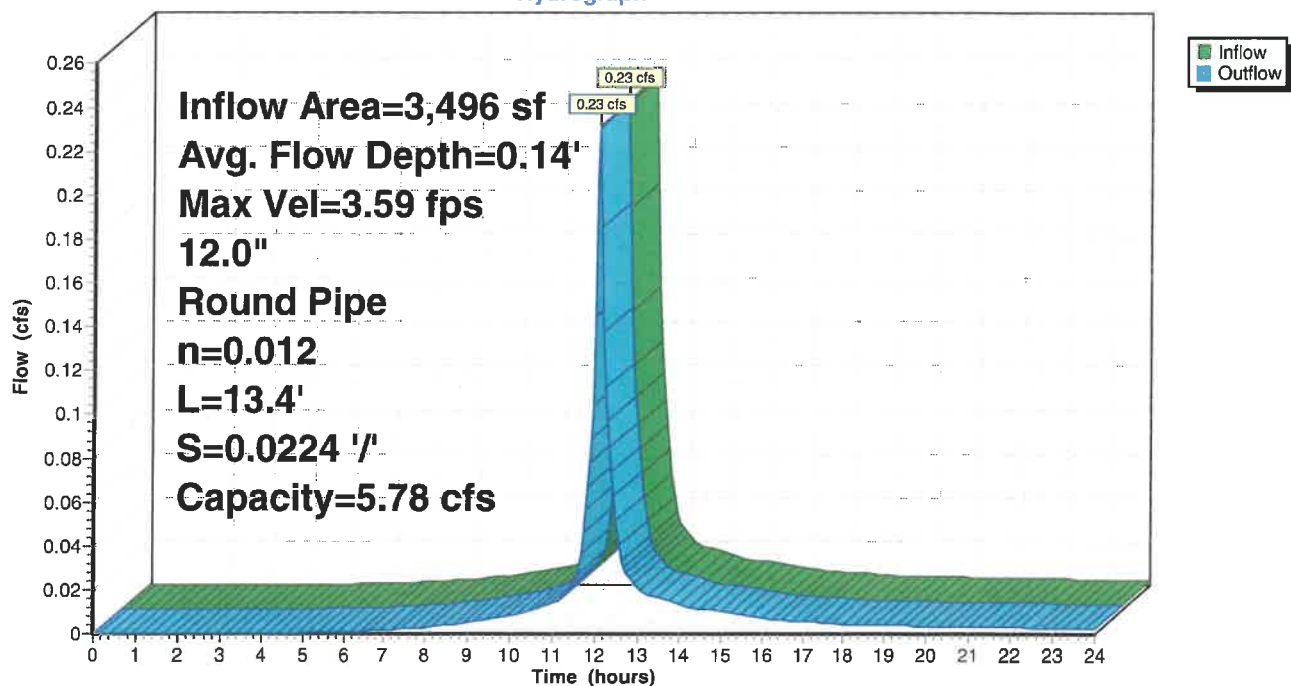
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.14', Surface Width= 0.69'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.78 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0224 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 11R: CB6 to DMH 3

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Summary for Reach 12R: DMH 7 TO BASIN

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 2.27" for 2 YR event
Inflow = 0.71 cfs @ 12.12 hrs, Volume= 2,434 cf
Outflow = 0.70 cfs @ 12.15 hrs, Volume= 2,432 cf, Atten= 3%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.21 fps, Min. Travel Time= 0.8 min
Avg. Velocity = 1.77 fps, Avg. Travel Time= 2.3 min

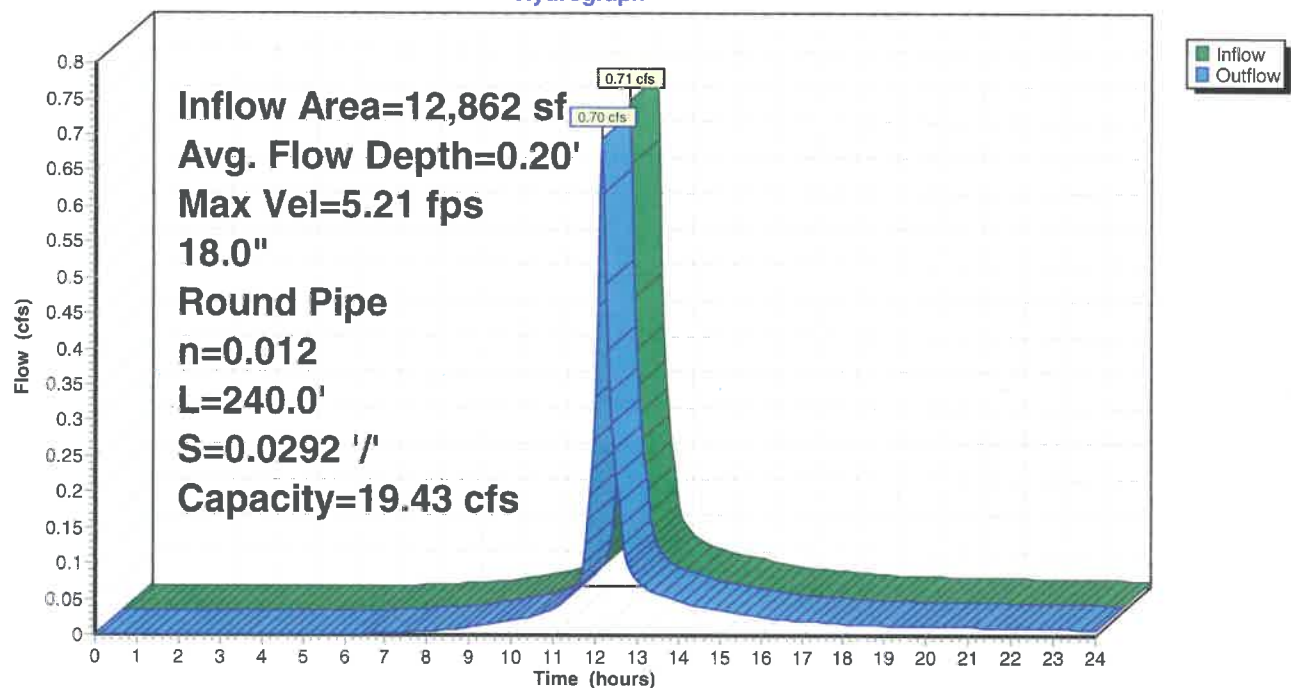
Peak Storage= 33 cf @ 12.14 hrs
Average Depth at Peak Storage= 0.20' , Surface Width= 1.01'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 19.43 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0292 1/'
Inlet Invert= 290.00', Outlet Invert= 283.00'



Reach 12R: DMH 7 TO BASIN

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Summary for Reach 13R: CB7 TO DMH 4

Inflow Area = 4,352 sf, 68.68% Impervious, Inflow Depth > 1.99" for 2 YR event
Inflow = 0.23 cfs @ 12.09 hrs, Volume= 722 cf
Outflow = 0.23 cfs @ 12.09 hrs, Volume= 722 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.19 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.48 fps, Avg. Travel Time= 0.1 min

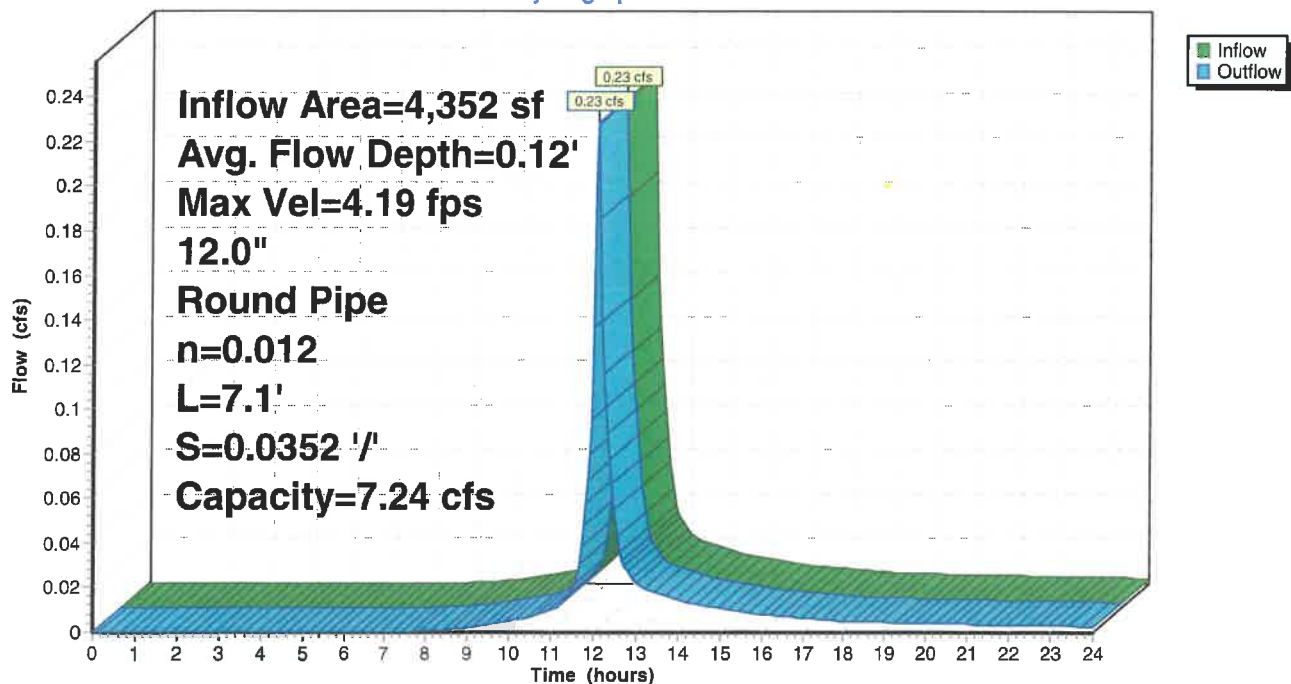
Peak Storage= 0 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.12', Surface Width= 0.65'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.24 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.1' Slope= 0.0352 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 13R: CB7 TO DMH 4

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Summary for Reach 14R: CB 8 TO DMH 4

Inflow Area = 4,078 sf, 73.30% Impervious, Inflow Depth > 2.16" for 2 YR event
Inflow = 0.23 cfs @ 12.09 hrs, Volume= 733 cf
Outflow = 0.23 cfs @ 12.09 hrs, Volume= 733 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.36 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.16 fps, Avg. Travel Time= 0.2 min

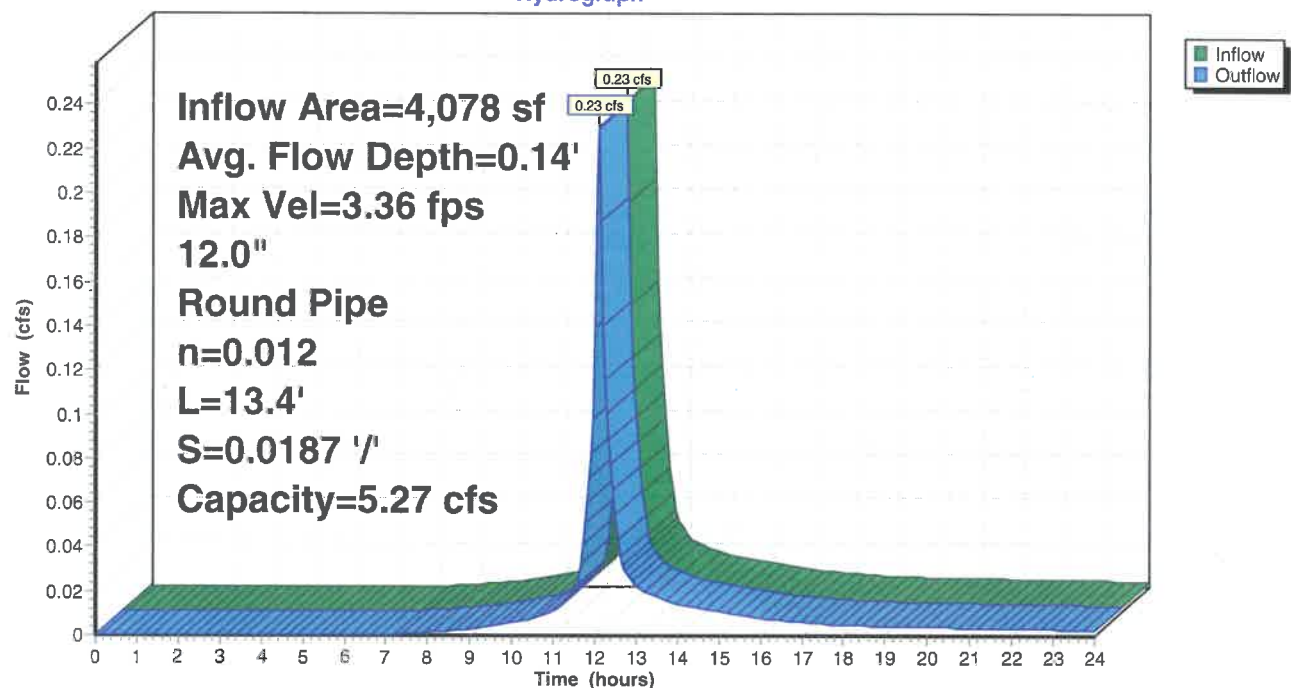
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.14', Surface Width= 0.70'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.27 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0187 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 14R: CB 8 TO DMH 4

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Summary for Reach 15R: DMH 4 TO DMH 5

Inflow Area = 8,430 sf, 70.91% Impervious, Inflow Depth > 2.07" for 2 YR event
Inflow = 0.46 cfs @ 12.09 hrs, Volume= 1,455 cf
Outflow = 0.45 cfs @ 12.10 hrs, Volume= 1,455 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.88 fps, Min. Travel Time= 0.2 min

Avg. Velocity= 2.38 fps, Avg. Travel Time= 0.7 min

Peak Storage= 7 cf @ 12.10 hrs

Average Depth at Peak Storage= 0.14', Surface Width= 0.69'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

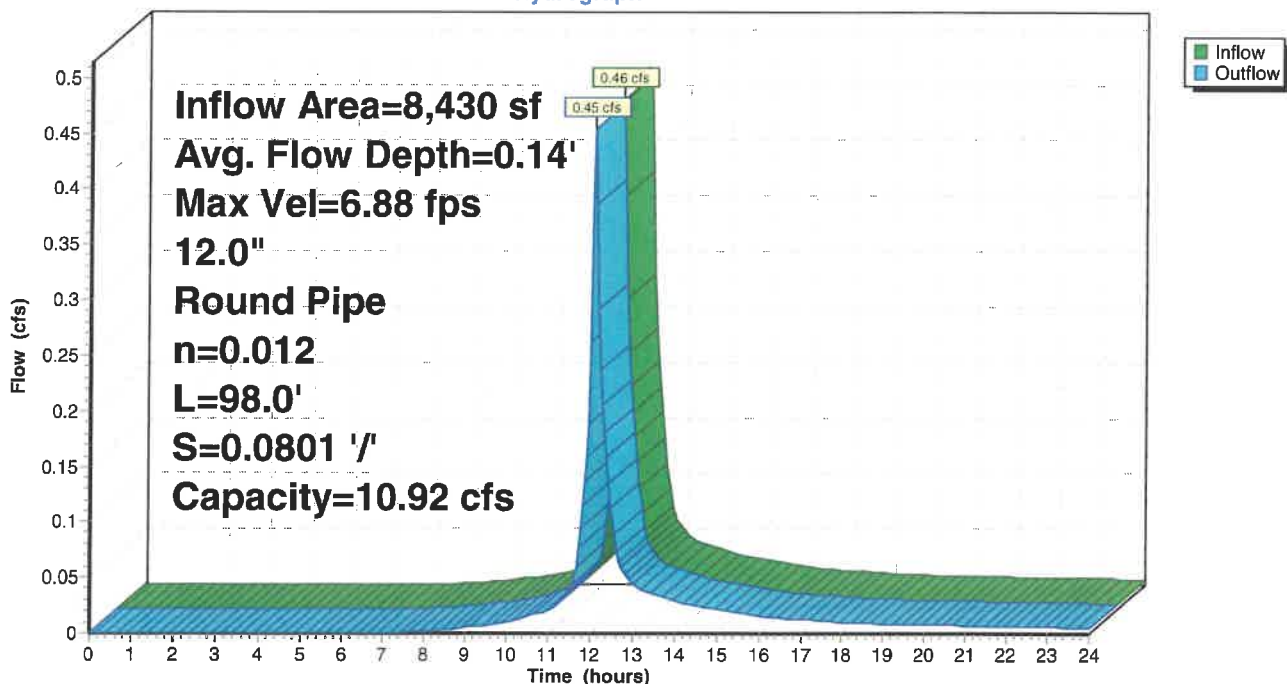
Length= 98.0' Slope= 0.0801 1'

Inlet Invert= 316.20', Outlet Invert= 308.35'



Reach 15R: DMH 4 TO DMH 5

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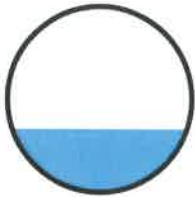
Summary for Reach 16R: DMH 5 TO DMH 6

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 1.83" for 2 YR event
Inflow = 1.75 cfs @ 12.10 hrs, Volume= 5,597 cf
Outflow = 1.66 cfs @ 12.13 hrs, Volume= 5,592 cf, Atten= 5%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.74 fps, Min. Travel Time= 0.9 min
Avg. Velocity = 1.64 fps, Avg. Travel Time= 2.5 min

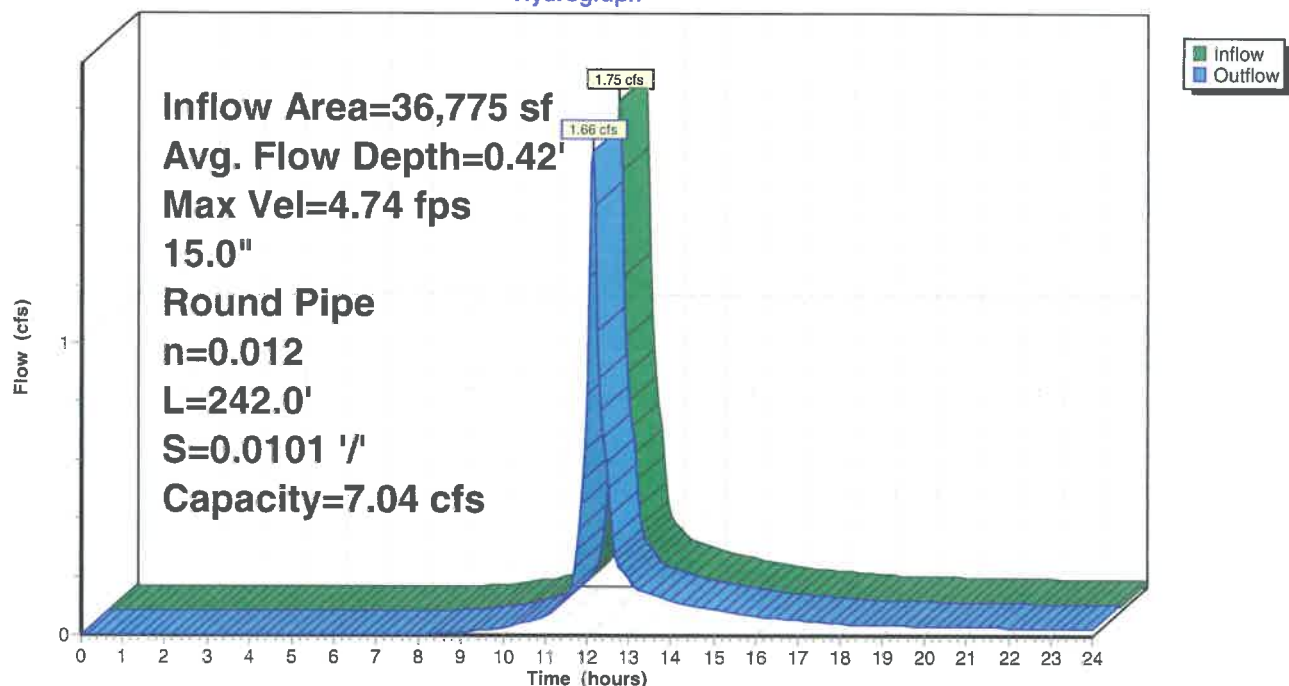
Peak Storage= 89 cf @ 12.11 hrs
Average Depth at Peak Storage= 0.42', Surface Width= 1.18'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.04 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 242.0' Slope= 0.0101 '/'
Inlet Invert= 308.10', Outlet Invert= 305.65'



Reach 16R: DMH 5 TO DMH 6

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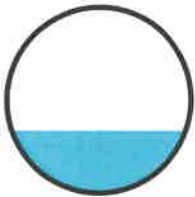
Summary for Reach 17R: DMH 6 TO DMH 5

Inflow Area = 28,345 sf, 58.80% Impervious, Inflow Depth > 1.75" for 2 YR event
Inflow = 1.31 cfs @ 12.09 hrs, Volume= 4,143 cf
Outflow = 1.30 cfs @ 12.10 hrs, Volume= 4,142 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.71 fps, Min. Travel Time= 0.3 min
Avg. Velocity = 2.05 fps, Avg. Travel Time= 0.8 min

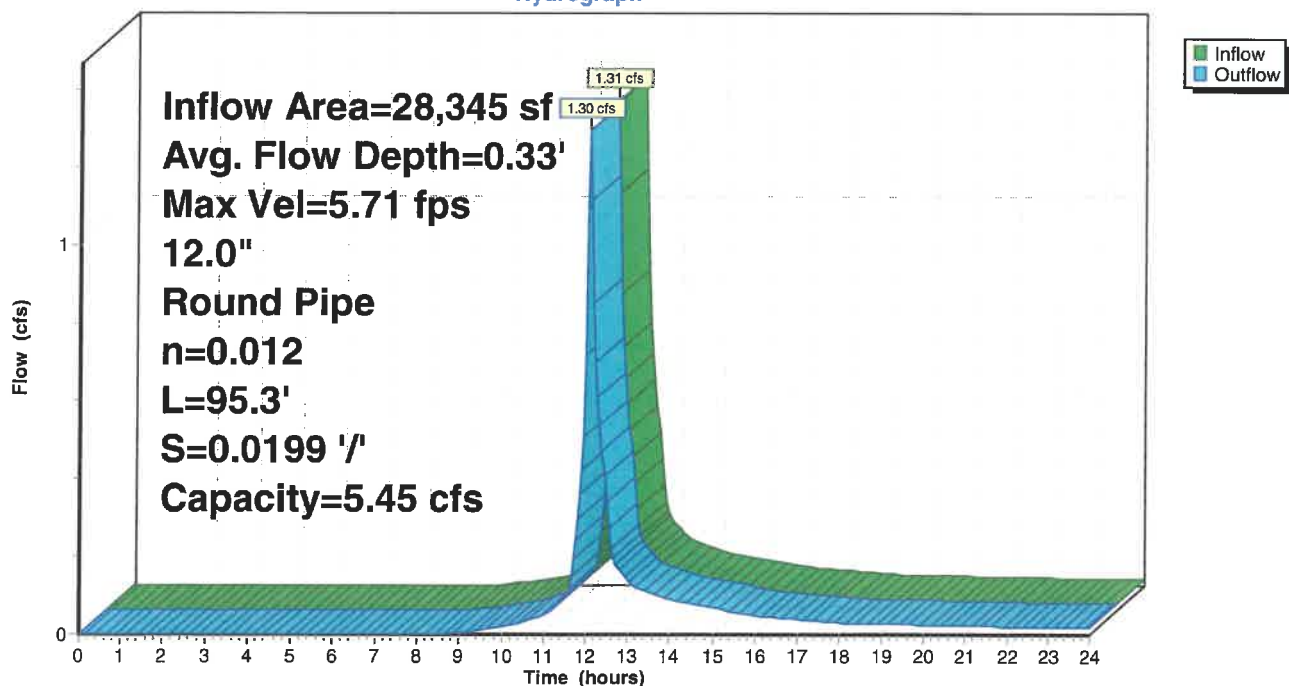
Peak Storage= 22 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.33', Surface Width= 0.94'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.45 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 95.3' Slope= 0.0199 '/'
Inlet Invert= 310.25', Outlet Invert= 308.35'



Reach 17R: DMH 6 TO DMH 5

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Summary for Reach 18R: CB 10 TO DMH 6

Inflow Area = 14,794 sf, 56.33% Impervious, Inflow Depth > 1.68" for 2 YR event
Inflow = 0.66 cfs @ 12.09 hrs, Volume= 2,074 cf
Outflow = 0.66 cfs @ 12.10 hrs, Volume= 2,074 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.64 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.68 fps, Avg. Travel Time= 0.1 min

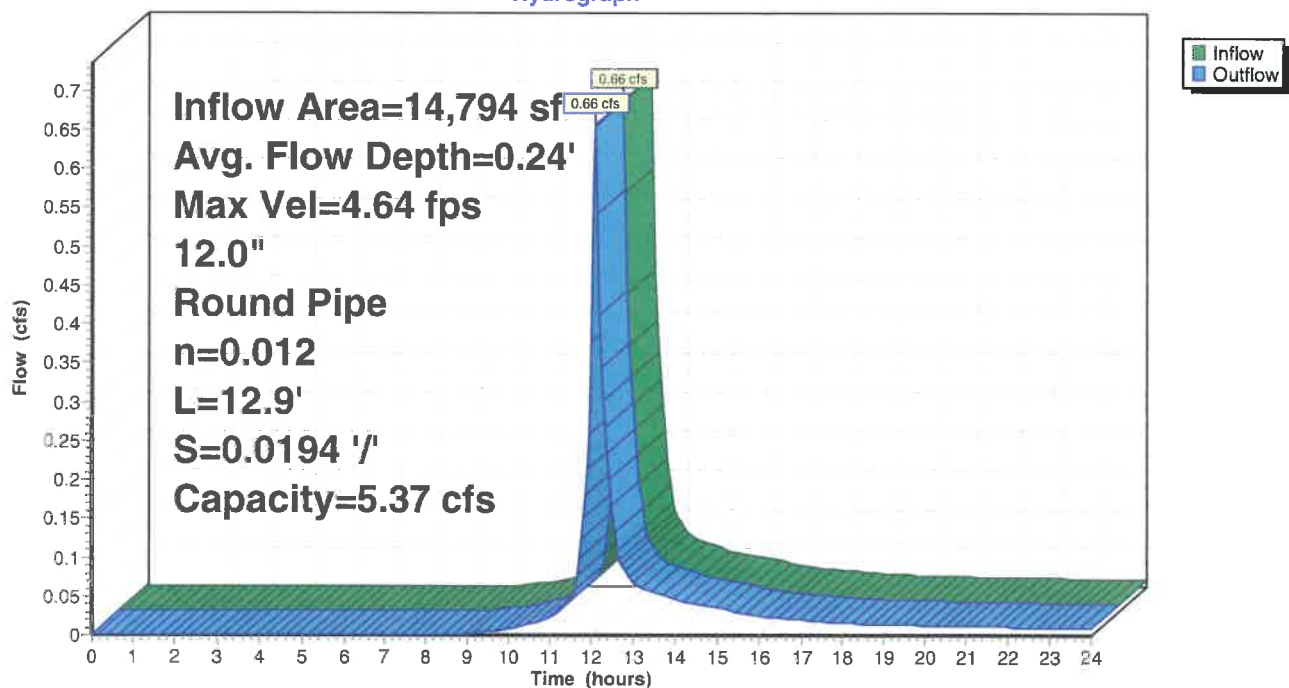
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.24' , Surface Width= 0.85'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 12.9' Slope= 0.0194 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 18R: CB 10 TO DMH 6

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Summary for Reach 19R: CB 9 TO DMH 6

Inflow Area = 13,551 sf, 61.49% Impervious, Inflow Depth > 1.83" for 2 YR event
Inflow = 0.66 cfs @ 12.09 hrs, Volume= 2,069 cf
Outflow = 0.66 cfs @ 12.09 hrs, Volume= 2,069 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.75 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 2.05 fps, Avg. Travel Time= 0.1 min

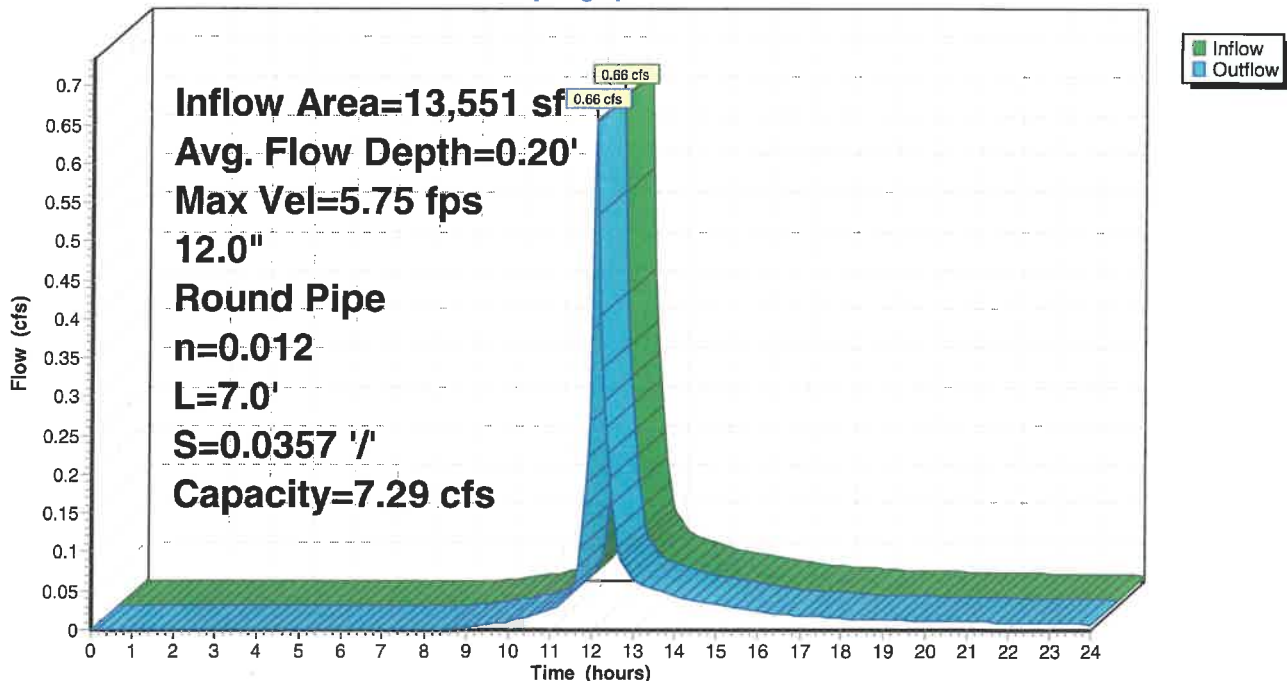
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.20' , Surface Width= 0.80'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.29 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.0' Slope= 0.0357 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 19R: CB 9 TO DMH 6

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Summary for Reach 20R: DMH 6 to Basin

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 1.82" for 2 YR event
Inflow = 1.66 cfs @ 12.13 hrs, Volume= 5,592 cf
Outflow = 1.66 cfs @ 12.13 hrs, Volume= 5,591 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 6.11 fps, Min. Travel Time= 0.1 min

Avg. Velocity = 2.14 fps, Avg. Travel Time= 0.2 min

Peak Storage= 8 cf @ 12.13 hrs

Average Depth at Peak Storage= 0.34' , Surface Width= 1.11'

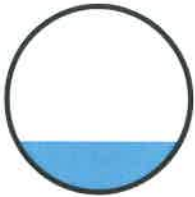
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 10.25 cfs

15.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

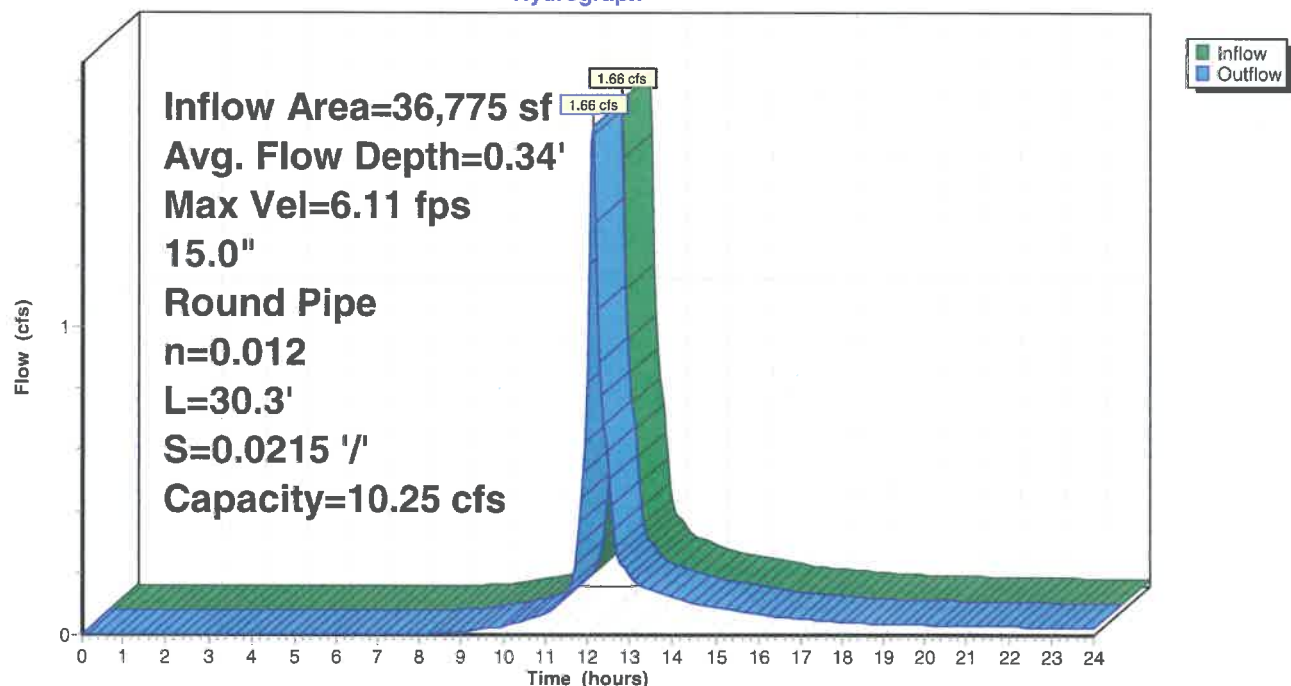
Length= 30.3' Slope= 0.0215 '/'

Inlet Invert= 305.65', Outlet Invert= 305.00'



Reach 20R: DMH 6 to Basin

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Summary for Reach 21R: 2-8" DI

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth = 0.00" for 2 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 0.67' Flow Area= 0.7 sf, Capacity= 3.82 cfs

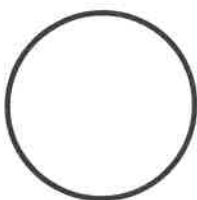
A factor of 2.00 has been applied to the storage and discharge capacity

8.0" Round Pipe

n= 0.013 Corrugated PE, smooth interior

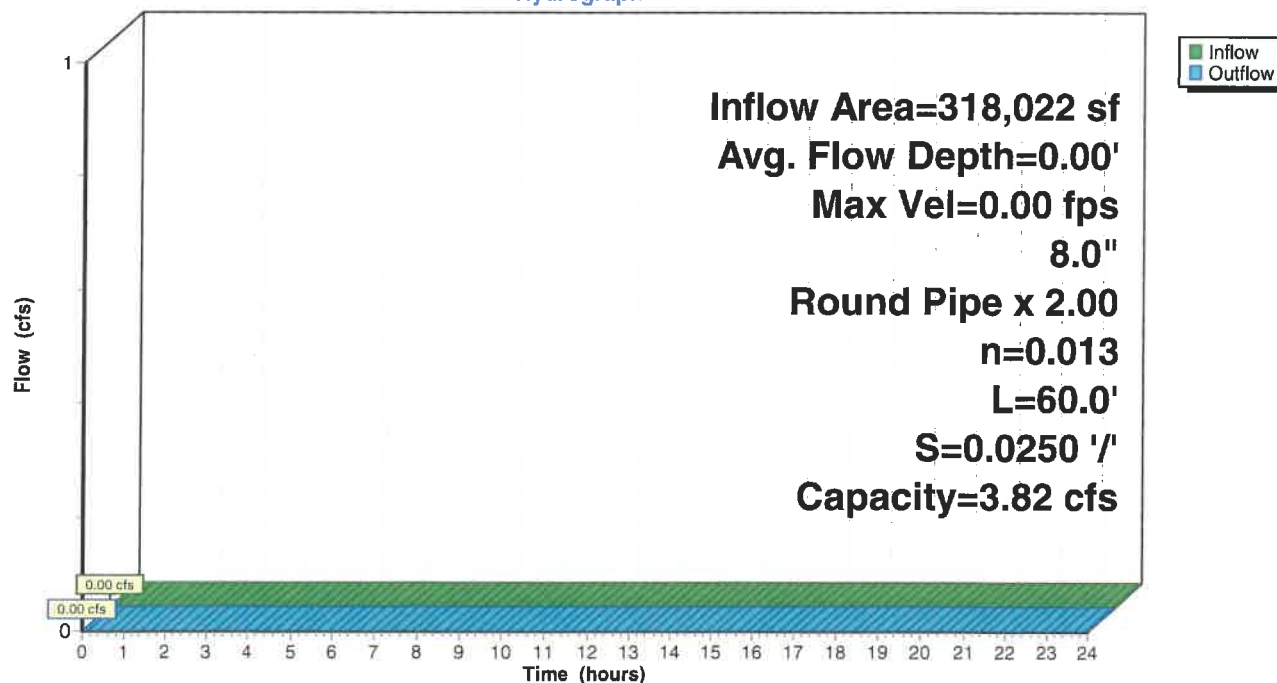
Length= 60.0' Slope= 0.0250 '/'

Inlet Invert= 312.50', Outlet Invert= 311.00'



Reach 21R: 2-8" DI

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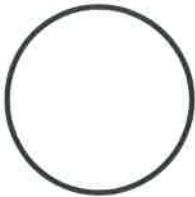
Summary for Reach 22R: Basin 2 Outfall

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth = 0.00" for 2 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

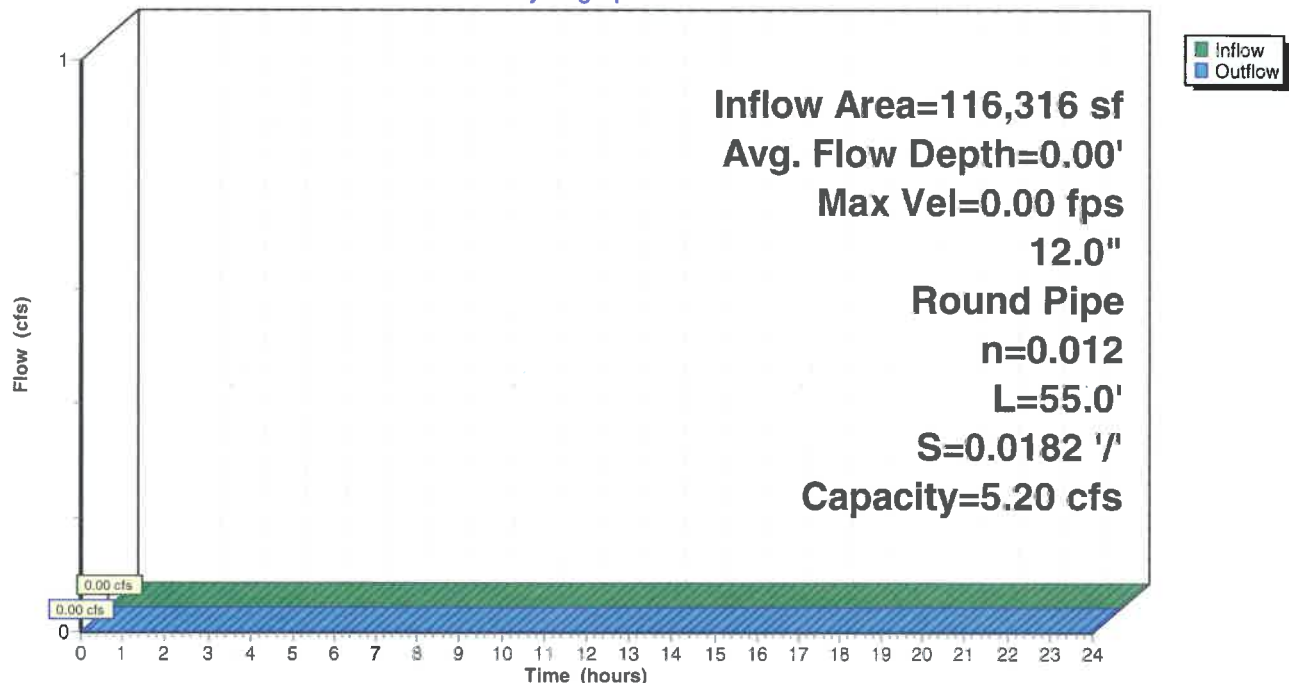
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.20 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 55.0' Slope= 0.0182 '/'
Inlet Invert= 304.00', Outlet Invert= 303.00'



Reach 22R: Basin 2 Outfall

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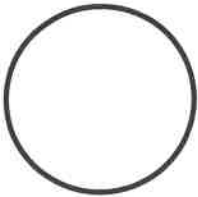
Summary for Reach 23R: Basin 1 Outfall

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth = 0.00" for 2 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

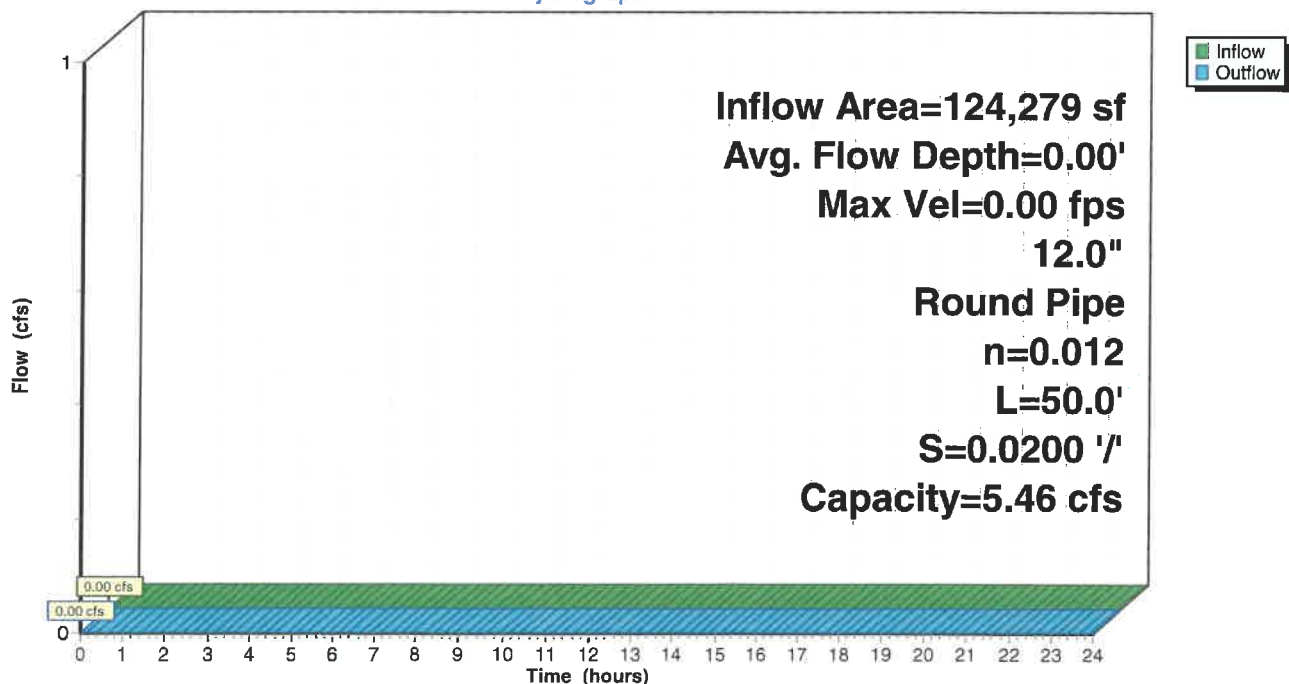
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.46 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 50.0' Slope= 0.0200 '/'
Inlet Invert= 281.00', Outlet Invert= 280.00'



Reach 23R: Basin 1 Outfall

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Summary for Reach 24R: Basin 3 Outfall

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth = 0.08" for 2 YR event
Inflow = 0.03 cfs @ 16.51 hrs, Volume= 358 cf
Outflow = 0.03 cfs @ 16.52 hrs, Volume= 358 cf, Atten= 0%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.88 fps, Min. Travel Time= 0.4 min
Avg. Velocity = 1.36 fps, Avg. Travel Time= 0.6 min

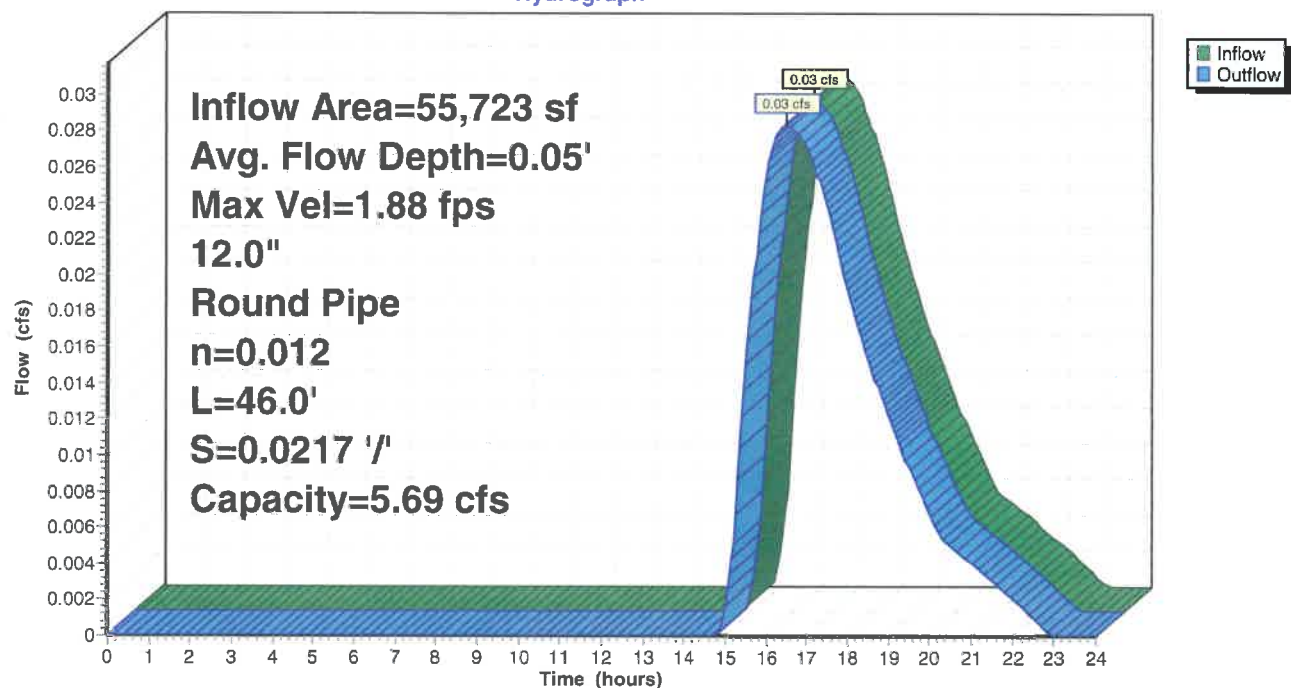
Peak Storage= 1 cf @ 16.51 hrs
Average Depth at Peak Storage= 0.05', Surface Width= 0.44'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.69 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 46.0' Slope= 0.0217 '/'
Inlet Invert= 289.00', Outlet Invert= 288.00'



Reach 24R: Basin 3 Outfall

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Summary for Reach 25R: 24" Bypass Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 0.51" for 2 YR event
Inflow = 2.90 cfs @ 12.51 hrs, Volume= 21,118 cf
Outflow = 2.90 cfs @ 12.52 hrs, Volume= 21,111 cf, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.80 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 3.14 fps, Avg. Travel Time= 0.4 min

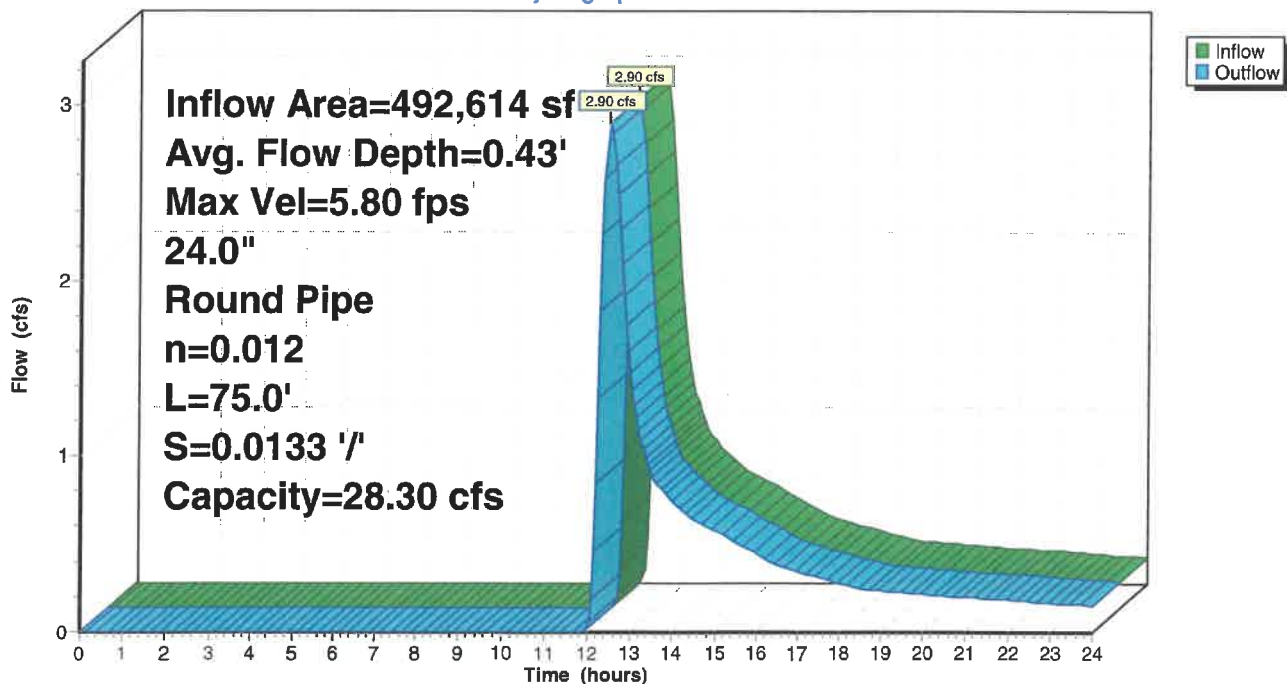
Peak Storage= 37 cf @ 12.52 hrs
Average Depth at Peak Storage= 0.43' , Surface Width= 1.65'
Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 28.30 cfs

24.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 75.0' Slope= 0.0133 '/'
Inlet Invert= 292.50', Outlet Invert= 291.50'



Reach 25R: 24" Bypass Culvert

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Summary for Reach EV1: Road Berm

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 0.25" for 2 YR event
Inflow = 2.78 cfs @ 15.43 hrs, Volume= 57,126 cf
Outflow = 2.78 cfs @ 15.44 hrs, Volume= 57,114 cf, Atten= 0%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.46 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.14 fps, Avg. Travel Time= 0.1 min

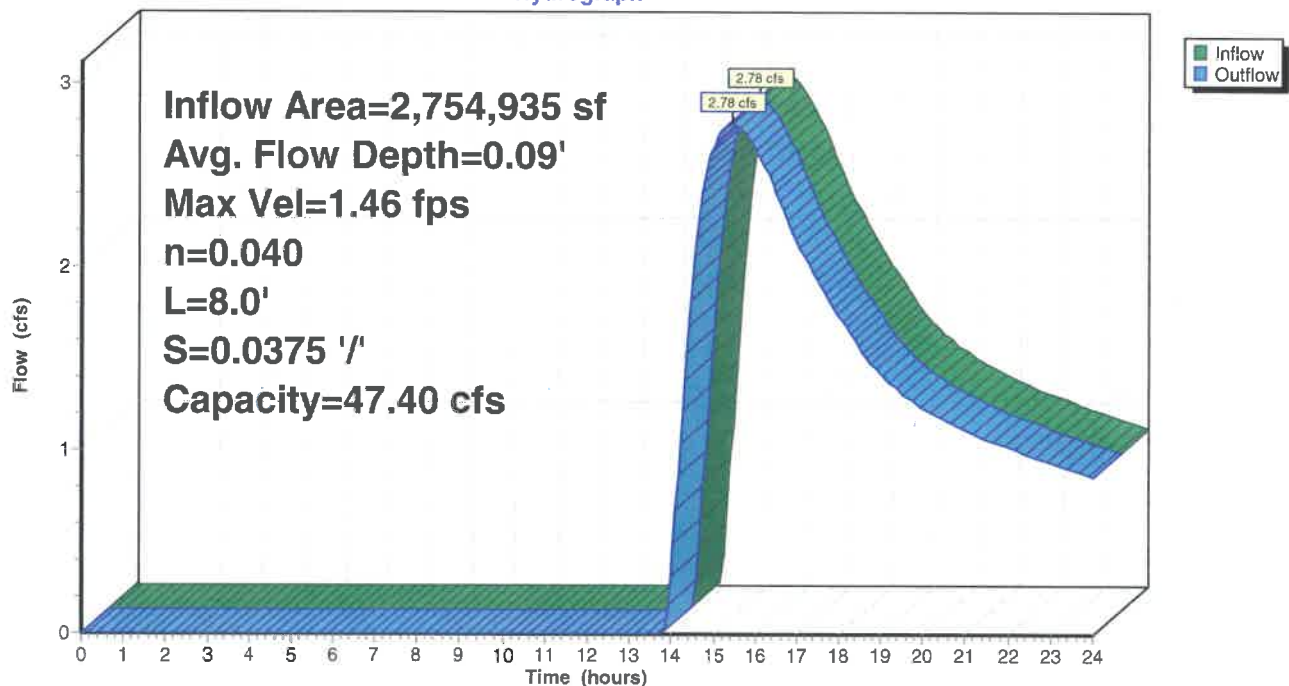
Peak Storage= 15 cf @ 15.44 hrs
Average Depth at Peak Storage= 0.09' , Surface Width= 20.93'
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides
Side Slope Z-value= 5.0 '/' Top Width= 25.00'
Length= 8.0' Slope= 0.0375 '/'
Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: Road Berm

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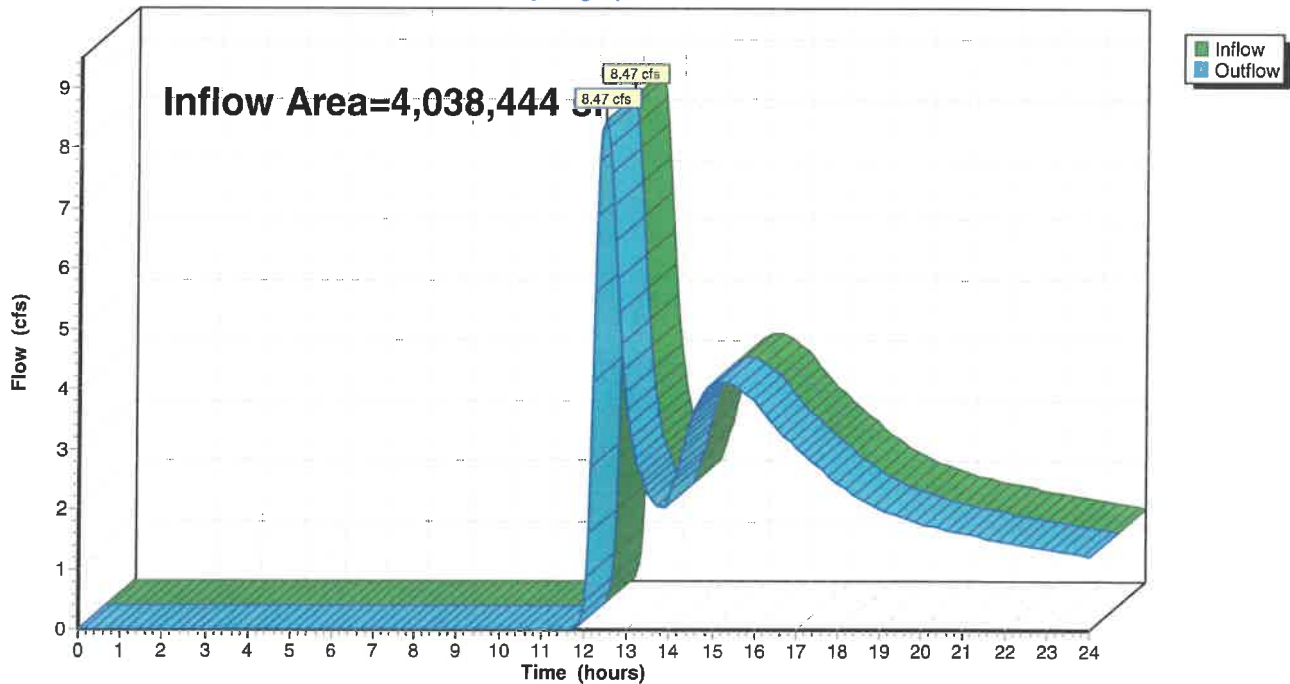
Summary for Reach EV2: To Wetland

Inflow Area = 4,038,444 sf, 4.14% Impervious, Inflow Depth > 0.34" for 2 YR event
Inflow = 8.47 cfs @ 12.45 hrs, Volume= 113,418 cf
Outflow = 8.47 cfs @ 12.45 hrs, Volume= 113,418 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV2: To Wetland

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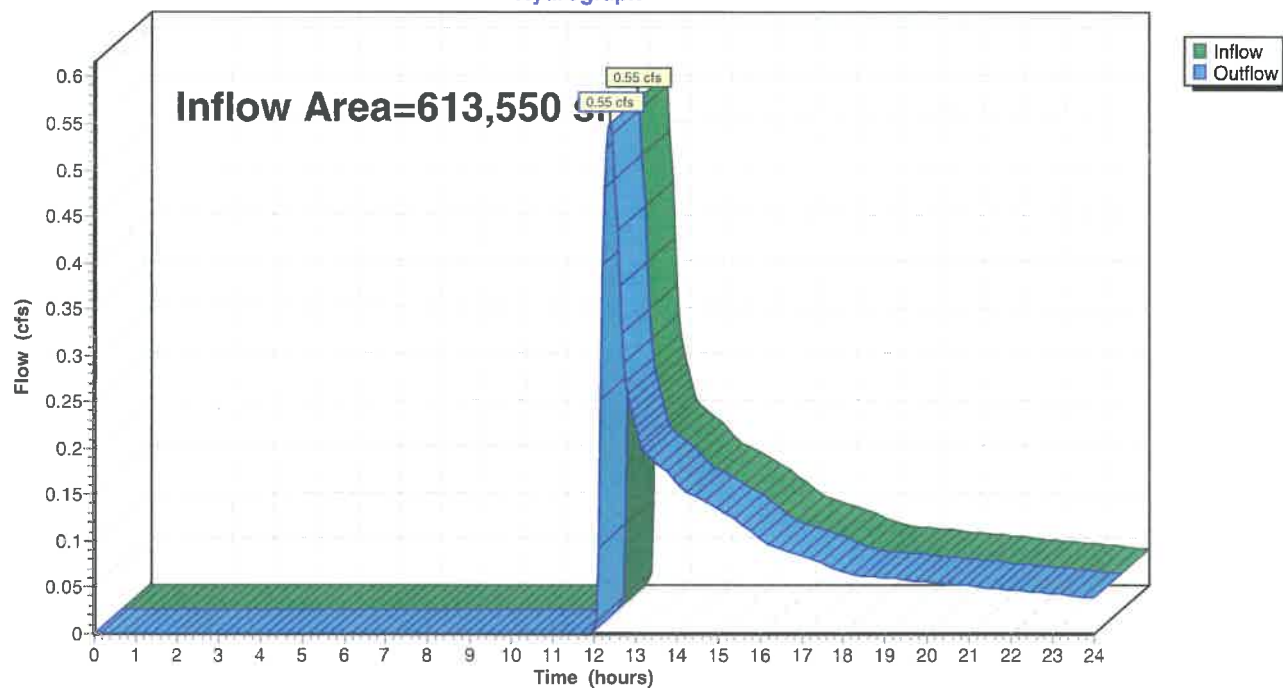
Summary for Reach EV3: To Offsite

Inflow Area = 613,550 sf, 3.69% Impervious, Inflow Depth > 0.09" for 2 YR event
Inflow = 0.55 cfs @ 12.38 hrs, Volume= 4,554 cf
Outflow = 0.55 cfs @ 12.38 hrs, Volume= 4,554 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV3: To Offsite

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Summary for Pond 1P: Forebay

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 2.27" for 2 YR event
 Inflow = 0.70 cfs @ 12.15 hrs, Volume= 2,432 cf
 Outflow = 0.09 cfs @ 12.86 hrs, Volume= 951 cf, Atten= 87%, Lag= 42.9 min
 Primary = 0.09 cfs @ 12.86 hrs, Volume= 951 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 282.51' @ 12.86 hrs Surf.Area= 1,262 sf Storage= 1,494 cf

Plug-Flow detention time= 284.9 min calculated for 949 cf (39% of inflow)
 Center-of-Mass det. time= 162.7 min (973.1 - 810.3)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 281.00' | 3,838 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 281.00 | 737 | 101.0 | 0 | 0 | 737 |
| 282.00 | 1,071 | 120.0 | 899 | 899 | 1,089 |
| 284.00 | 1,908 | 158.0 | 2,939 | 3,838 | 1,974 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 282.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=0.05 cfs @ 12.86 hrs HW=282.51' (Free Discharge)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 0.05 cfs @ 0.25 fps)

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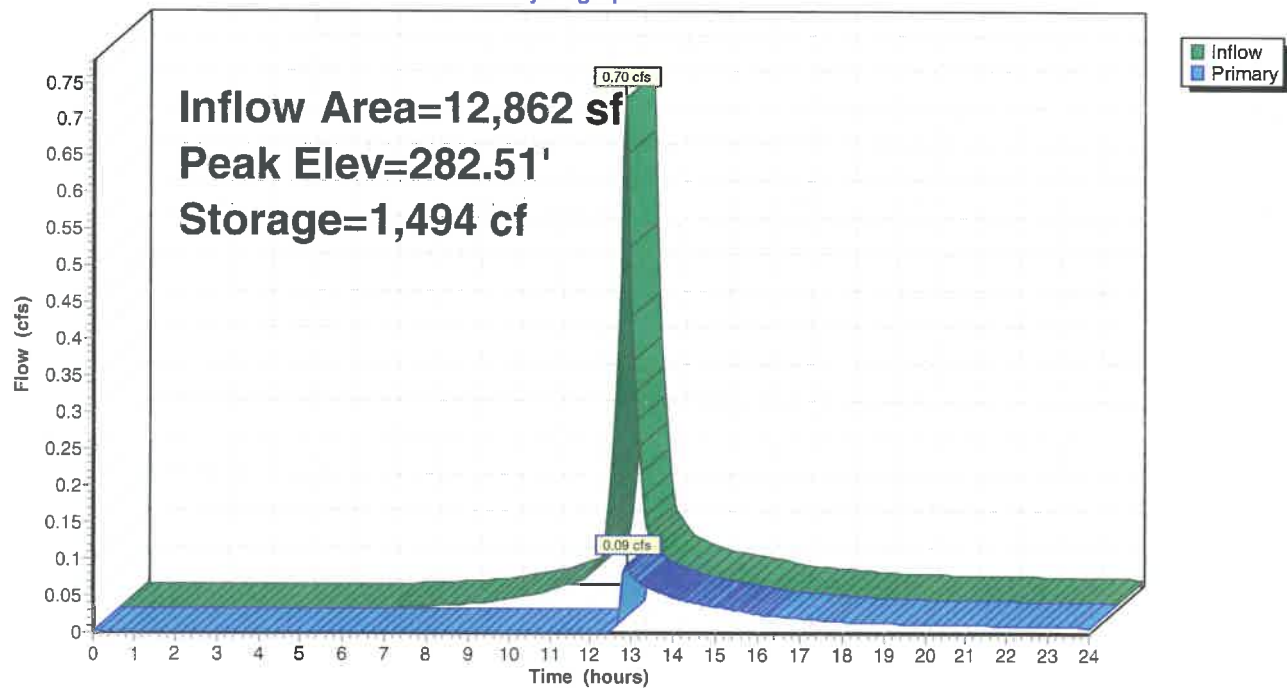
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Pond 1P: Forebay

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Summary for Pond 2P: Basin 1

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 0.39" for 2 YR event
 Inflow = 0.39 cfs @ 12.39 hrs, Volume= 4,077 cf
 Outflow = 0.10 cfs @ 15.85 hrs, Volume= 3,950 cf, Atten= 75%, Lag= 207.8 min
 Discarded = 0.10 cfs @ 15.85 hrs, Volume= 3,950 cf
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 281.32' @ 15.85 hrs Surf.Area= 3,930 sf Storage= 1,204 cf

Plug-Flow detention time= 135.6 min calculated for 3,950 cf (97% of inflow)
 Center-of-Mass det. time= 120.9 min (1,069.5 - 948.7)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 281.00' | 51,257 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 281.00 | 3,708 | 232.0 | 0 | 0 | 3,708 |
| 282.00 | 4,433 | 251.0 | 4,065 | 4,065 | 4,477 |
| 284.00 | 6,052 | 288.0 | 10,443 | 14,508 | 6,153 |
| 286.00 | 7,890 | 326.0 | 13,901 | 28,410 | 8,109 |
| 288.00 | 9,968 | 364.0 | 17,818 | 46,227 | 10,306 |
| 288.50 | 10,153 | 374.0 | 5,030 | 51,257 | 10,922 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 287.50' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 281.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 281.00' | 12.0" Round Culvert L= 44.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 281.00' / 280.00' S= 0.0227 ' S= 0.0227 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 282.00' | 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 282.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 283.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 286.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.10 cfs @ 15.85 hrs HW=281.32' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=281.00' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Controls 0.00 cfs)

↳ **4=Orifice/Grate** (Controls 0.00 cfs)

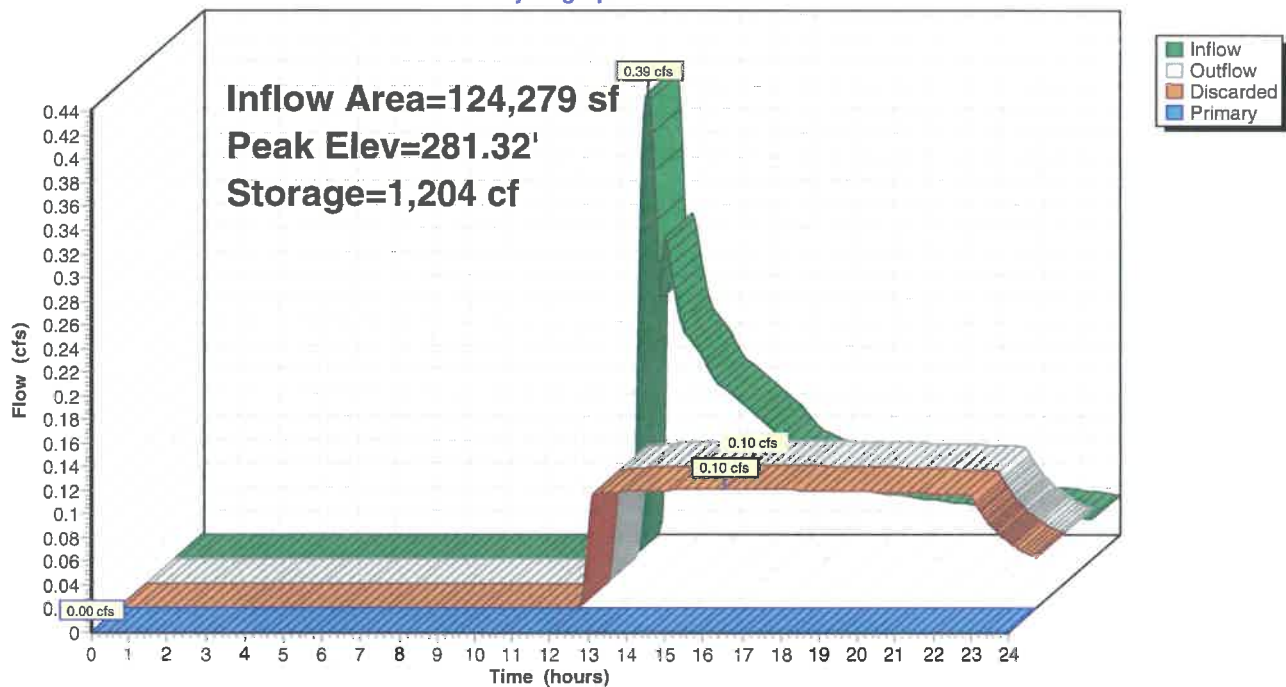
↳ **5=Orifice/Grate** (Controls 0.00 cfs)

↳ **6=Orifice/Grate** (Controls 0.00 cfs)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 2P: Basin 1

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Summary for Pond 3P: Forebay

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 1.82" for 2 YR event
 Inflow = 1.66 cfs @ 12.13 hrs, Volume= 5,591 cf
 Outflow = 0.83 cfs @ 12.34 hrs, Volume= 3,383 cf, Atten= 50%, Lag= 12.7 min
 Primary = 0.83 cfs @ 12.34 hrs, Volume= 3,383 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.56' @ 12.34 hrs Surf.Area= 1,833 sf Storage= 2,322 cf

Plug-Flow detention time= 190.2 min calculated for 3,383 cf (60% of inflow)
 Center-of-Mass det. time= 82.7 min (911.7 - 829.0)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 10,058 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 1,160 | 129.0 | 0 | 0 | 1,160 |
| 306.00 | 2,048 | 166.0 | 3,166 | 3,166 | 2,077 |
| 308.00 | 3,164 | 204.0 | 5,172 | 8,338 | 3,256 |
| 308.50 | 3,725 | 227.0 | 1,720 | 10,058 | 4,052 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 305.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=0.79 cfs @ 12.34 hrs HW=305.56' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 0.79 cfs @ 0.62 fps)

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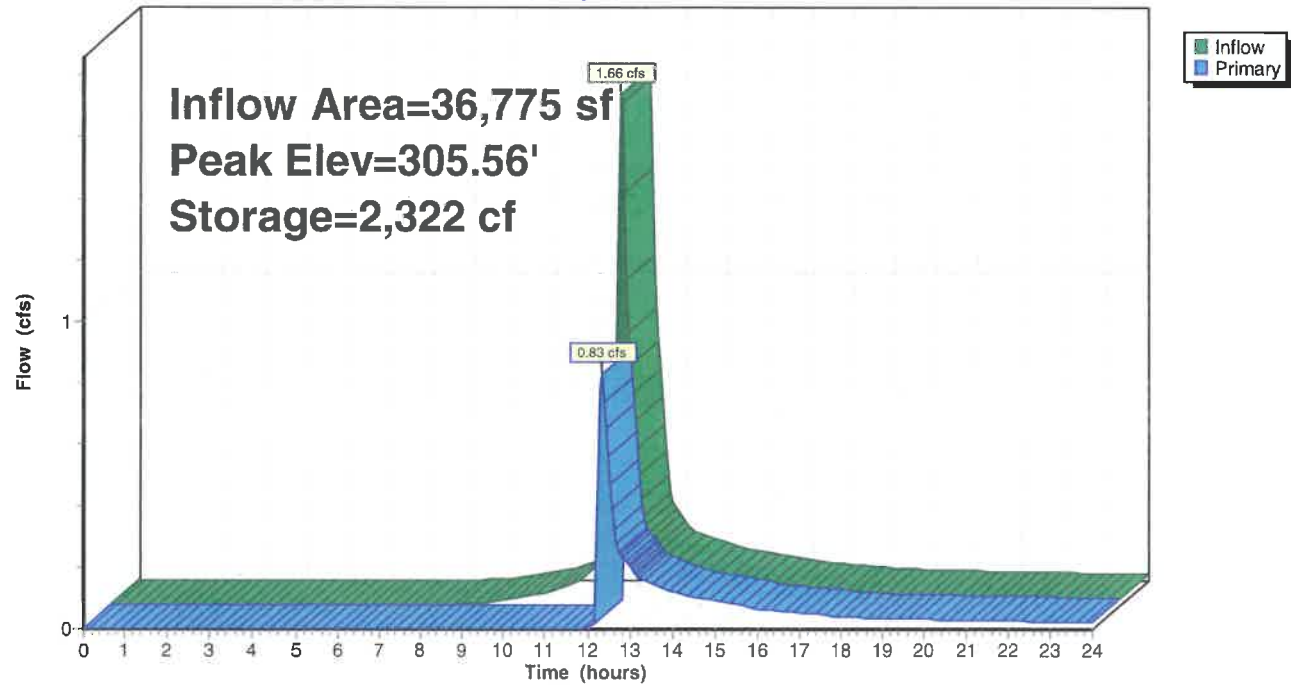
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Pond 3P: Forebay

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Summary for Pond 4P: Basin 2

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 0.63" for 2 YR event
 Inflow = 1.22 cfs @ 12.34 hrs, Volume= 6,067 cf
 Outflow = 0.13 cfs @ 15.92 hrs, Volume= 5,264 cf, Atten= 90%, Lag= 214.8 min
 Discarded = 0.13 cfs @ 15.92 hrs, Volume= 5,264 cf
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 304.51' @ 15.92 hrs Surf.Area= 4,966 sf Storage= 2,427 cf

Plug-Flow detention time= 224.4 min calculated for 5,253 cf (87% of inflow)
 Center-of-Mass det. time= 165.6 min (1,084.4 - 918.8)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 304.00' | 30,313 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 304.00 | 4,504 | 302.0 | 0 | 0 | 4,504 |
| 306.00 | 6,434 | 340.0 | 10,881 | 10,881 | 6,549 |
| 308.00 | 8,590 | 378.0 | 14,972 | 25,853 | 8,836 |
| 308.50 | 9,256 | 397.0 | 4,460 | 30,313 | 10,023 |

| Device | Routing | Invert | Outlet Devices | |
|--------|-----------|---------|--|--|
| #1 | Primary | 307.70' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 | |
| #2 | Discarded | 304.00' | 1.100 in/hr Exfiltration over Surface area | |
| #3 | Primary | 304.00' | 12.0" Round Culvert L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 304.00' / 303.00' S= 0.0286 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf | |
| #4 | Device 3 | 305.00' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads | |
| #5 | Device 3 | 305.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads | |
| #6 | Device 3 | 306.00' | 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads | |

Discarded OutFlow Max=0.13 cfs @ 15.92 hrs HW=304.51' (Free Discharge)

2=Exfiltration (Exfiltration Controls 0.13 cfs)
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=304.00' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

3=Culvert (Controls 0.00 cfs)

4=Orifice/Grate (Controls 0.00 cfs)

5=Orifice/Grate (Controls 0.00 cfs)

6=Orifice/Grate (Controls 0.00 cfs)

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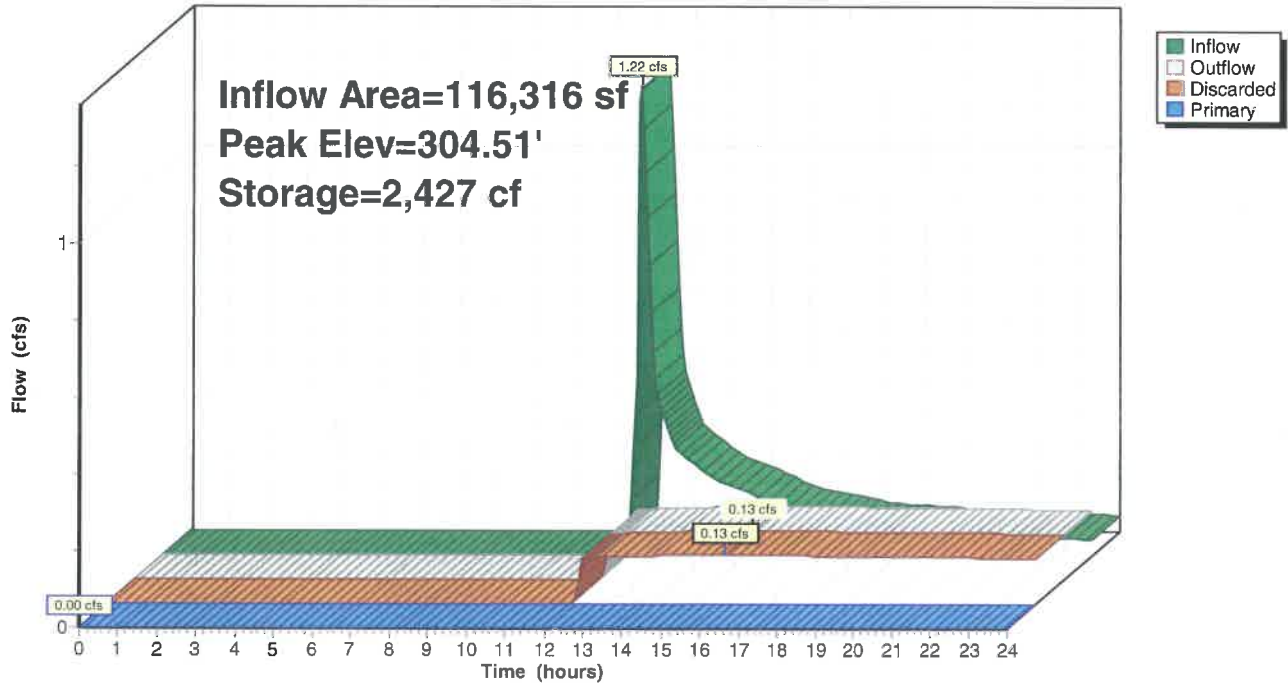
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Pond 4P: Basin 2

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Summary for Pond 5P: Forebay

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 2.71" for 2 YR event
 Inflow = 1.64 cfs @ 12.10 hrs, Volume= 5,507 cf
 Outflow = 0.12 cfs @ 13.41 hrs, Volume= 1,655 cf, Atten= 93%, Lag= 78.9 min
 Primary = 0.12 cfs @ 13.41 hrs, Volume= 1,655 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 291.22' @ 13.41 hrs Surf.Area= 2,259 sf Storage= 3,887 cf

Plug-Flow detention time= 368.6 min calculated for 1,651 cf (30% of inflow)
 Center-of-Mass det. time= 217.8 min (1,002.2 - 784.4)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 289.00' | 5,821 cf | Forebay (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 289.00 | 1,270 | 132.0 | 0 | 0 | 1,270 |
| 290.00 | 1,710 | 153.0 | 1,485 | 1,485 | 1,767 |
| 291.00 | 2,145 | 172.0 | 1,923 | 3,408 | 2,285 |
| 292.00 | 2,692 | 191.0 | 2,413 | 5,821 | 2,863 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 291.20' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=0.11 cfs @ 13.41 hrs HW=291.22' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 0.11 cfs @ 0.32 fps)

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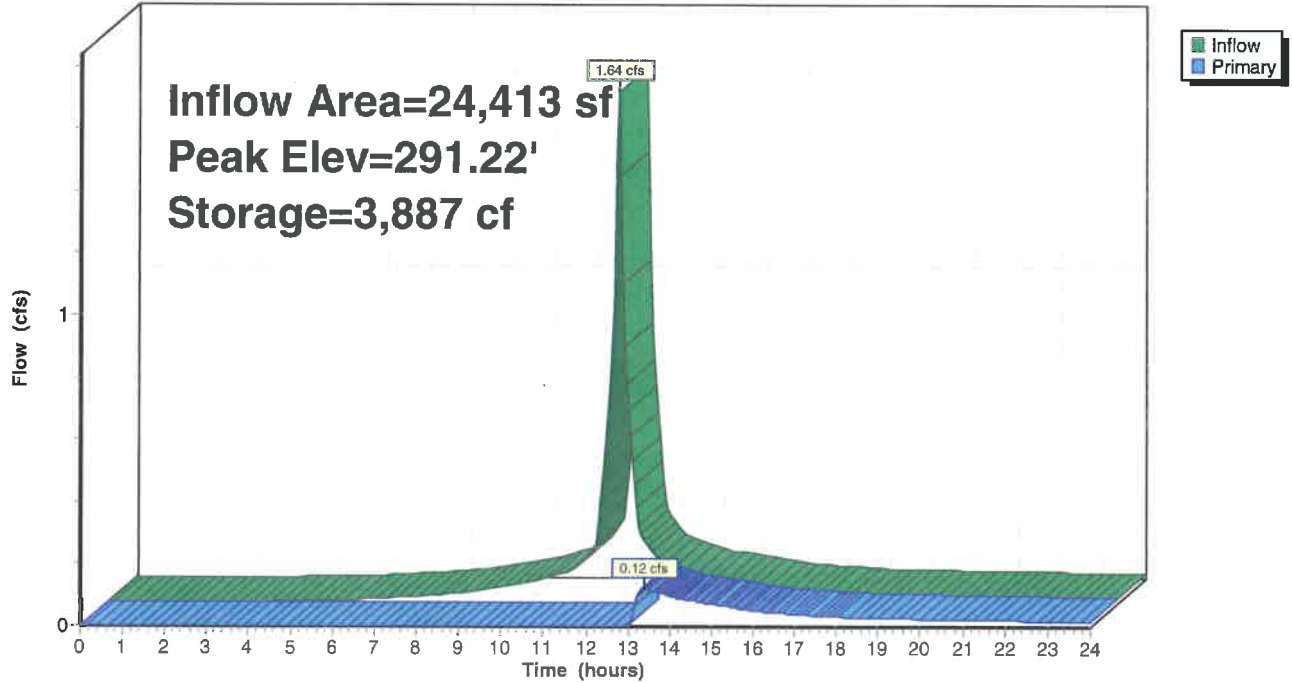
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Pond 5P: Forebay

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Summary for Pond 6P: Basin 3

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 0.65" for 2 YR event
 Inflow = 0.30 cfs @ 12.12 hrs, Volume= 3,009 cf
 Outflow = 0.07 cfs @ 16.51 hrs, Volume= 1,930 cf, Atten= 77%, Lag= 263.4 min
 Discarded = 0.04 cfs @ 16.51 hrs, Volume= 1,572 cf
 Primary = 0.03 cfs @ 16.51 hrs, Volume= 358 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 290.12' @ 16.51 hrs Surf.Area= 1,494 sf Storage= 1,295 cf

Plug-Flow detention time= 250.4 min calculated for 1,926 cf (64% of inflow)
 Center-of-Mass det. time= 127.3 min (1,086.7 - 959.4)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 289.00' | 12,306 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 289.00 | 840 | 185.0 | 0 | 0 | 840 |
| 290.00 | 1,423 | 204.0 | 1,119 | 1,119 | 1,459 |
| 291.00 | 2,065 | 223.0 | 1,734 | 2,853 | 2,139 |
| 292.00 | 2,763 | 242.0 | 2,406 | 5,258 | 2,880 |
| 293.00 | 3,517 | 260.0 | 3,132 | 8,391 | 3,642 |
| 294.00 | 4,328 | 279.0 | 3,915 | 12,306 | 4,500 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 293.25' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 289.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 289.00' | 12.0" Round Culvert L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 289.00' / 288.00' S= 0.0222 ' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 290.00' | 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 290.50' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 291.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 292.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.04 cfs @ 16.51 hrs HW=290.12' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.03 cfs @ 16.51 hrs HW=290.12' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 0.03 cfs of 2.63 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.03 cfs @ 1.18 fps)

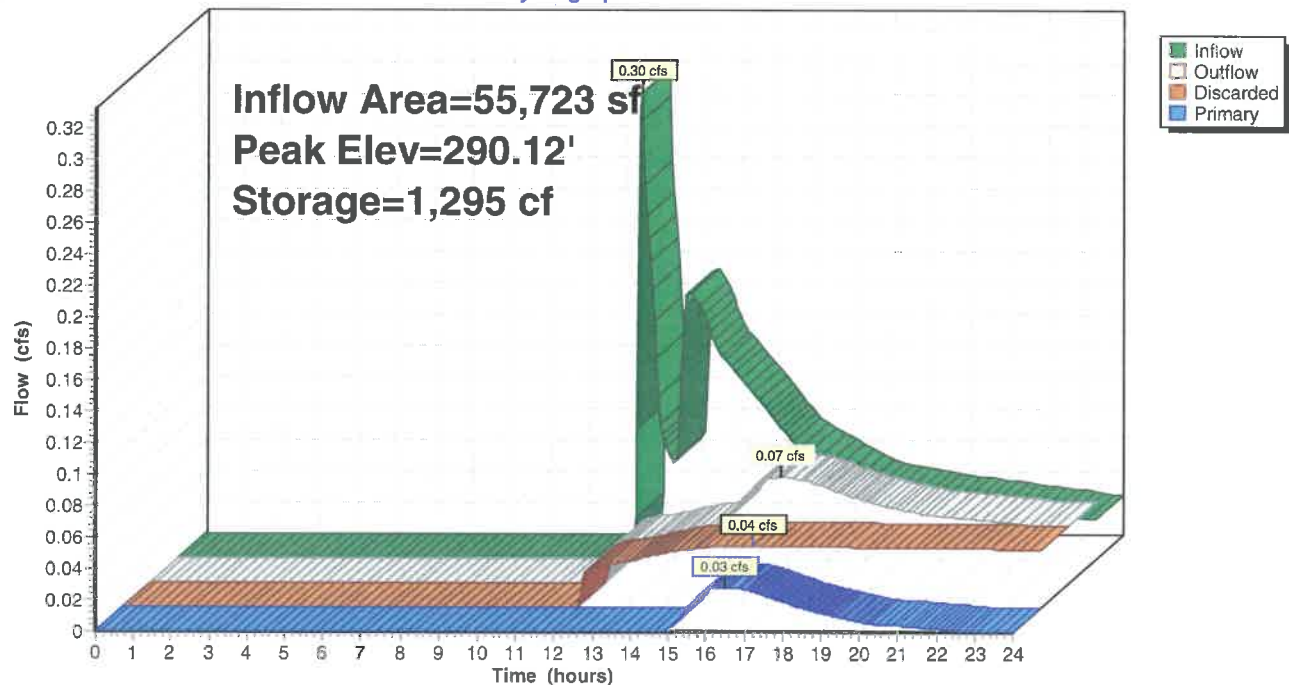
↳ **5=Orifice/Grate** (Controls 0.00 cfs)

↳ **6=Orifice/Grate** (Controls 0.00 cfs)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 6P: Basin 3

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Summary for Pond 7P: Vernal Pool and Wetland

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 0.43" for 2 YR event
 Inflow = 9.98 cfs @ 12.81 hrs, Volume= 99,846 cf
 Outflow = 2.78 cfs @ 15.43 hrs, Volume= 57,126 cf, Atten= 72%, Lag= 157.3 min
 Primary = 2.78 cfs @ 15.43 hrs, Volume= 57,126 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 294.07' @ 15.43 hrs Surf.Area= 33,327 sf Storage= 47,404 cf

Plug-Flow detention time= 273.4 min calculated for 57,126 cf (57% of inflow)
 Center-of-Mass det. time= 138.6 min (1,084.4 - 945.9)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 291.30' | 143,915 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=2.77 cfs @ 15.43 hrs HW=294.07' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 2.77 cfs @ 1.30 fps)

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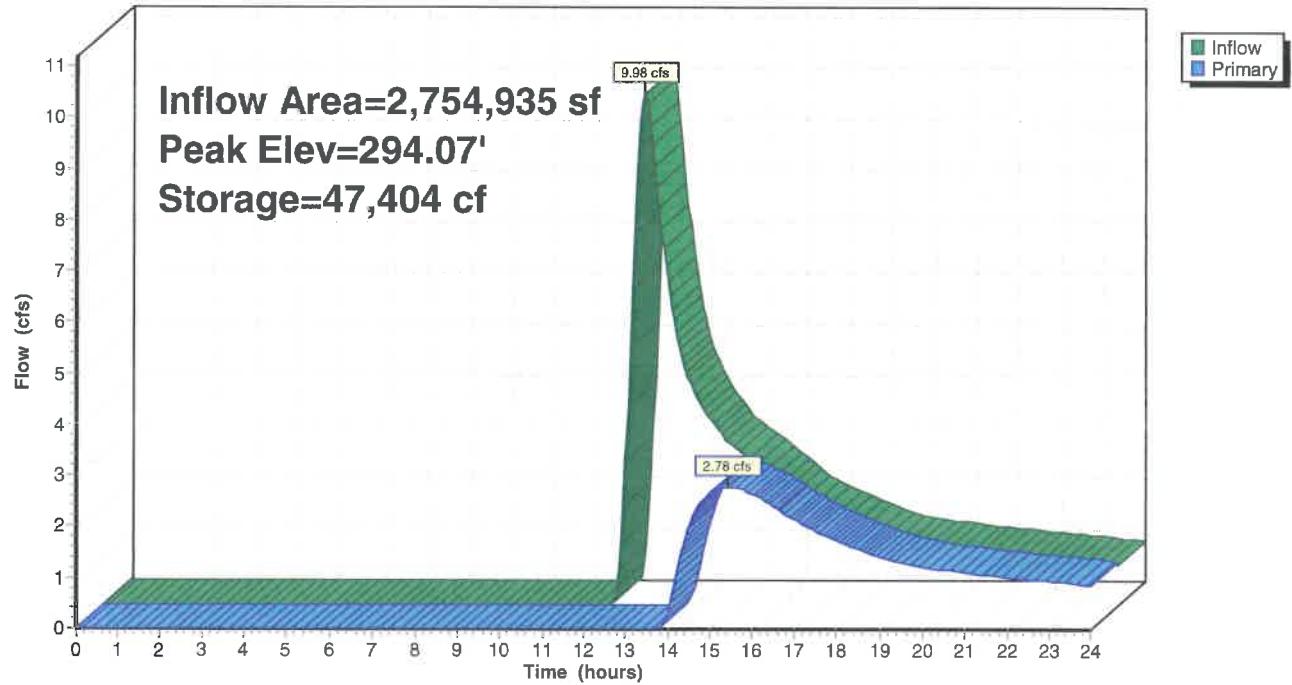
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Pond 7P: Vernal Pool and Wetland

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Summary for Pond 8P: Road Berm

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth > 0.30" for 2 YR event
 Inflow = 0.85 cfs @ 12.54 hrs, Volume= 8,039 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 310.46' @ 24.00 hrs Surf.Area= 23,736 sf Storage= 8,027 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 310.00' | 256,496 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 312.50' | 8.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)

↑1=Orifice/Grate (Controls 0.00 cfs)

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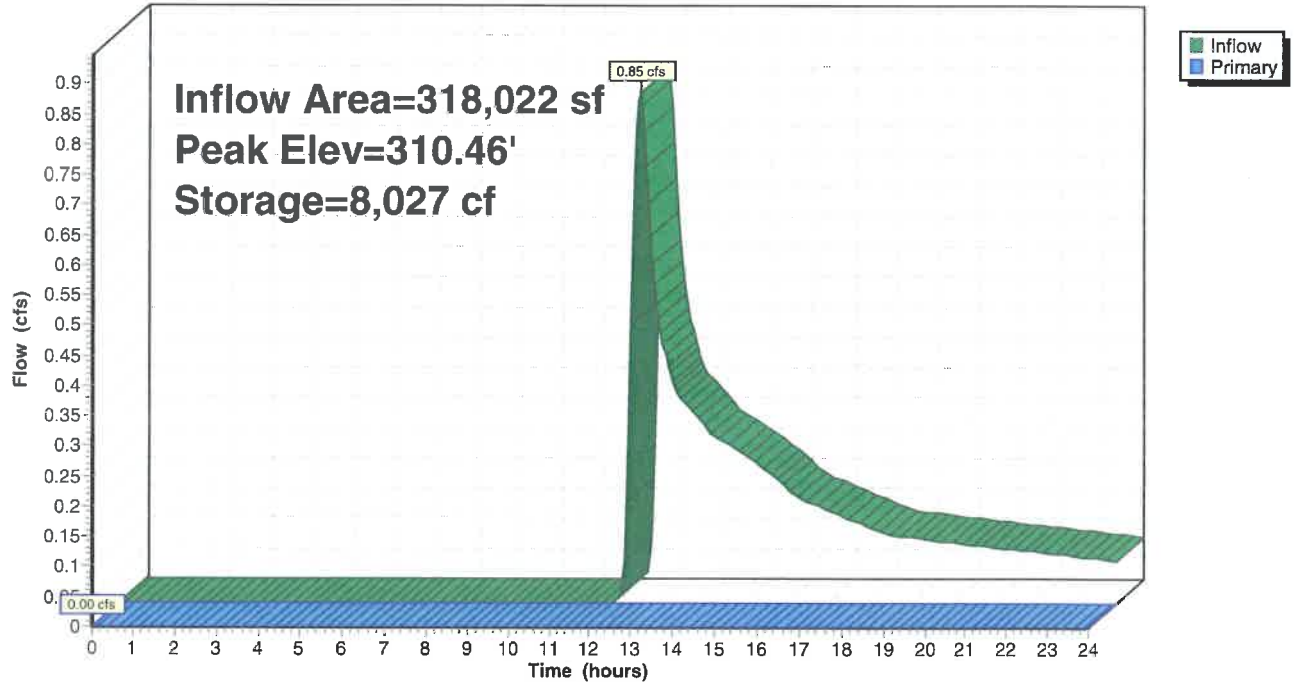
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Pond 8P: Road Berm

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Summary for Subcatchment 1S: To Culvert

Runoff = 10.78 cfs @ 12.43 hrs, Volume= 61,489 cf, Depth> 1.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

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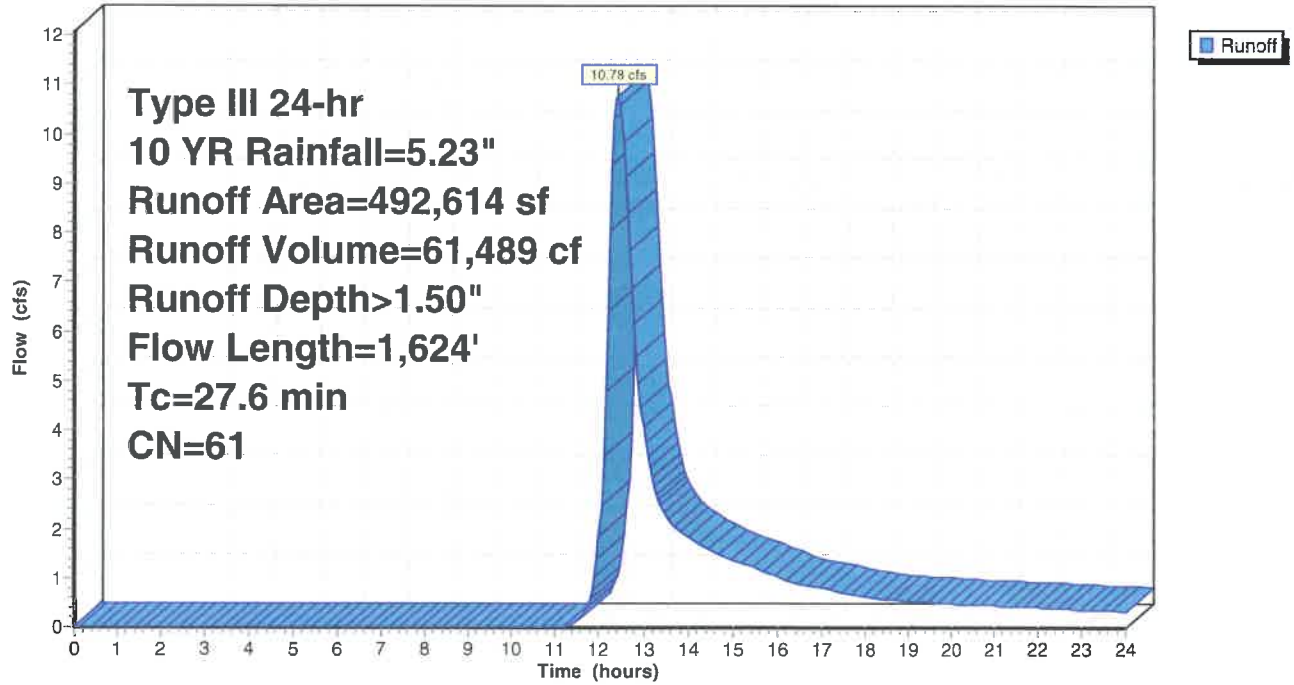
00454 - PROPOSED Conditions
Type III 24-hr 10 YR Rainfall=5.23"

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Subcatchment 1S: To Culvert

Hydrograph



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Summary for Subcatchment 2S: To Proposed Culvert

Runoff = 41.58 cfs @ 12.70 hrs, Volume= 309,492 cf, Depth> 1.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 45.8 | 2,588 | Total | | | |

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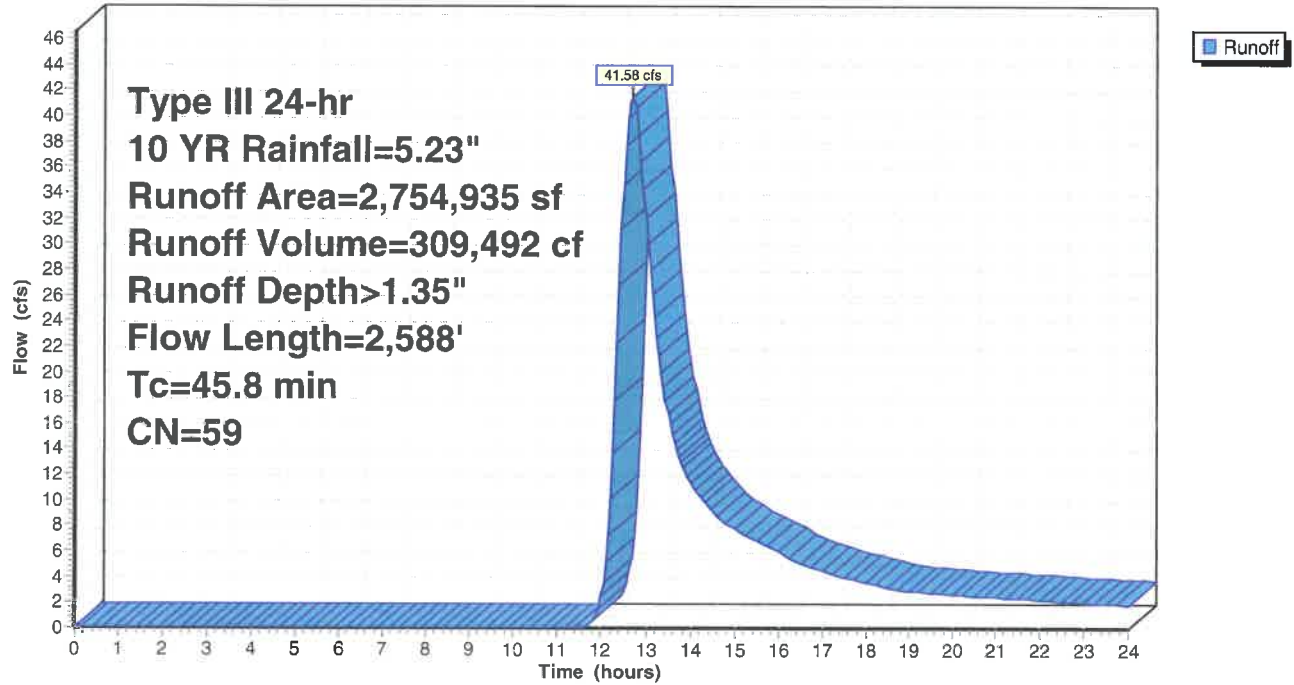
00454 - PROPOSED Conditions
Type III 24-hr 10 YR Rainfall=5.23"

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Subcatchment 2S: To Proposed Culvert

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Type III 24-hr 10 YR Rainfall=5.23"

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Summary for Subcatchment 3S: To 8" pipes

Runoff = 5.01 cfs @ 12.37 hrs, Volume= 28,868 cf, Depth> 1.09"

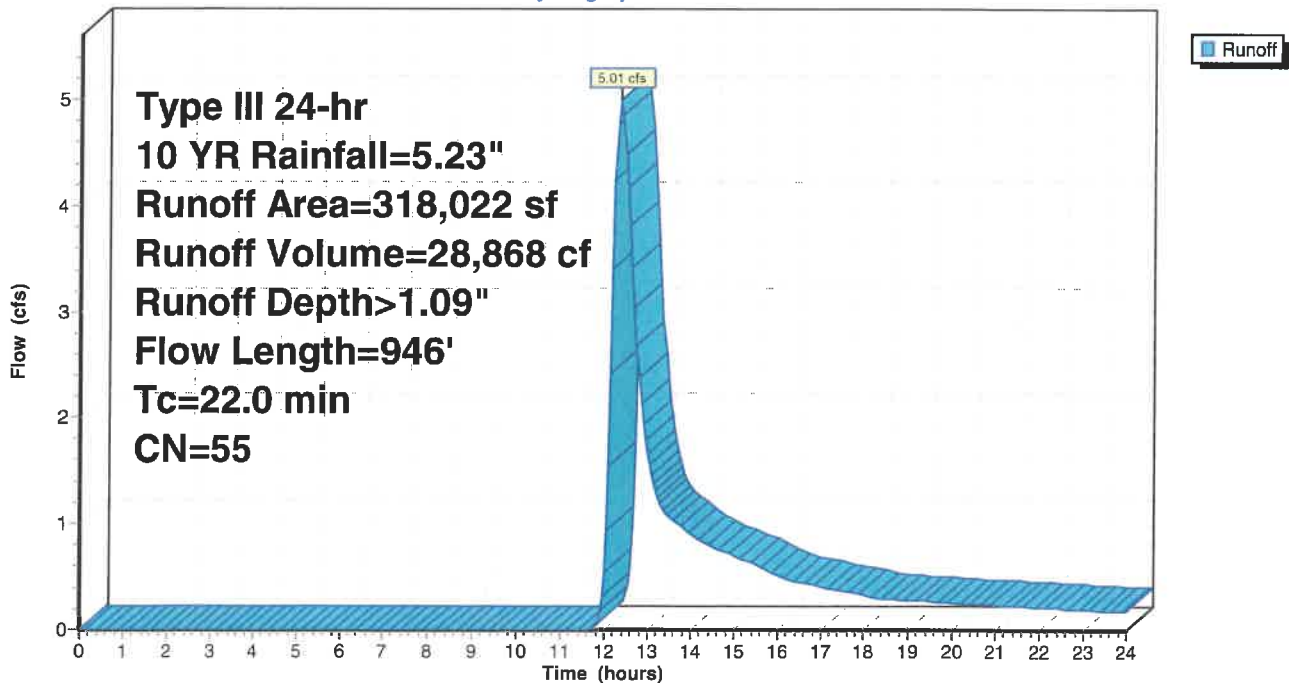
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 318,022 | 55 | Woods, Good, HSG B |
| 318,022 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.22" |
| 14.9 | 896 | 0.0401 | 1.00 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 22.0 | 946 | Total | | | |

Subcatchment 3S: To 8" pipes

Hydrograph



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Summary for Subcatchment 4S: To CB 1

Runoff = 1.41 cfs @ 12.09 hrs, Volume= 4,704 cf, Depth> 4.31"

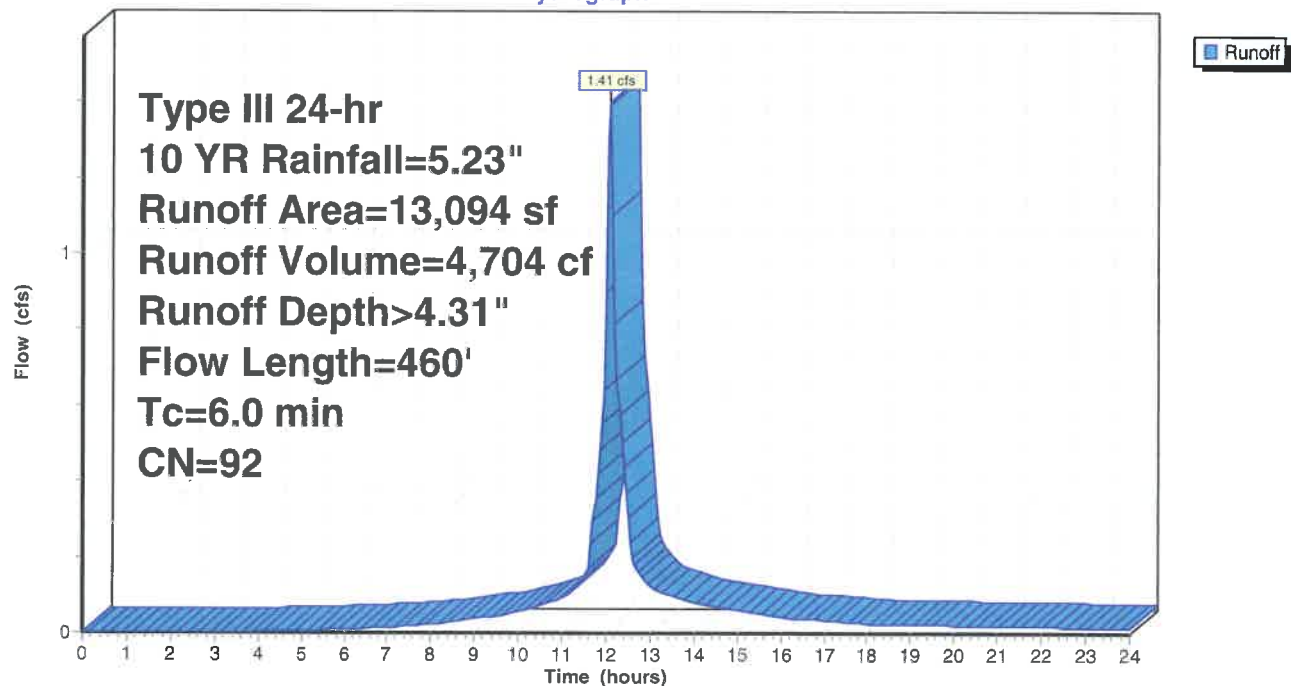
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 11,086 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,008 | 61 | >75% Grass cover, Good, HSG B |
| 13,094 | 92 | Weighted Average |
| 2,008 | | 15.34% Pervious Area |
| 11,086 | | 84.66% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 4S: To CB 1

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Summary for Subcatchment 5S: To CB 2

Runoff = 1.28 cfs @ 12.09 hrs, Volume= 4,488 cf, Depth> 4.76"

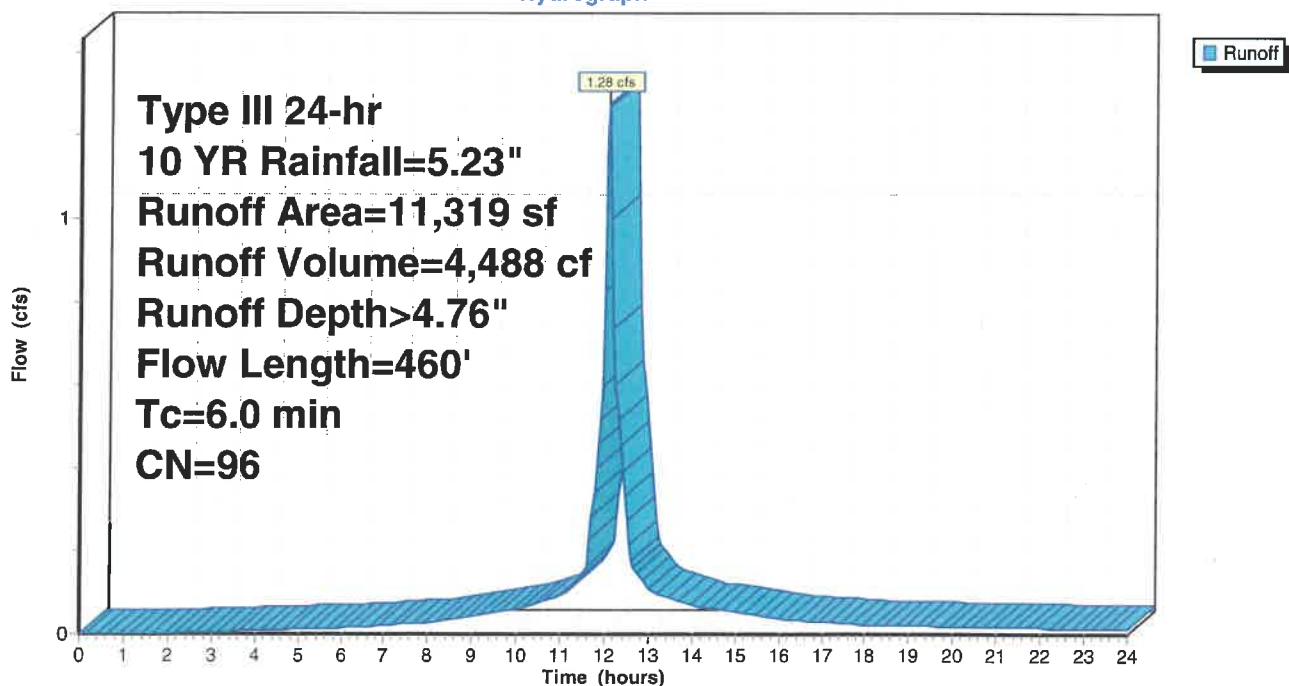
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 10,719 | 98 | Paved roads w/curbs & sewers, HSG B |
| 600 | 61 | >75% Grass cover, Good, HSG B |
| 11,319 | 96 | Weighted Average |
| 600 | | 5.30% Pervious Area |
| 10,719 | | 94.70% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 5S: To CB 2

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Summary for Subcatchment 6S: To CB 3

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 1,039 cf, Depth> 3.99"

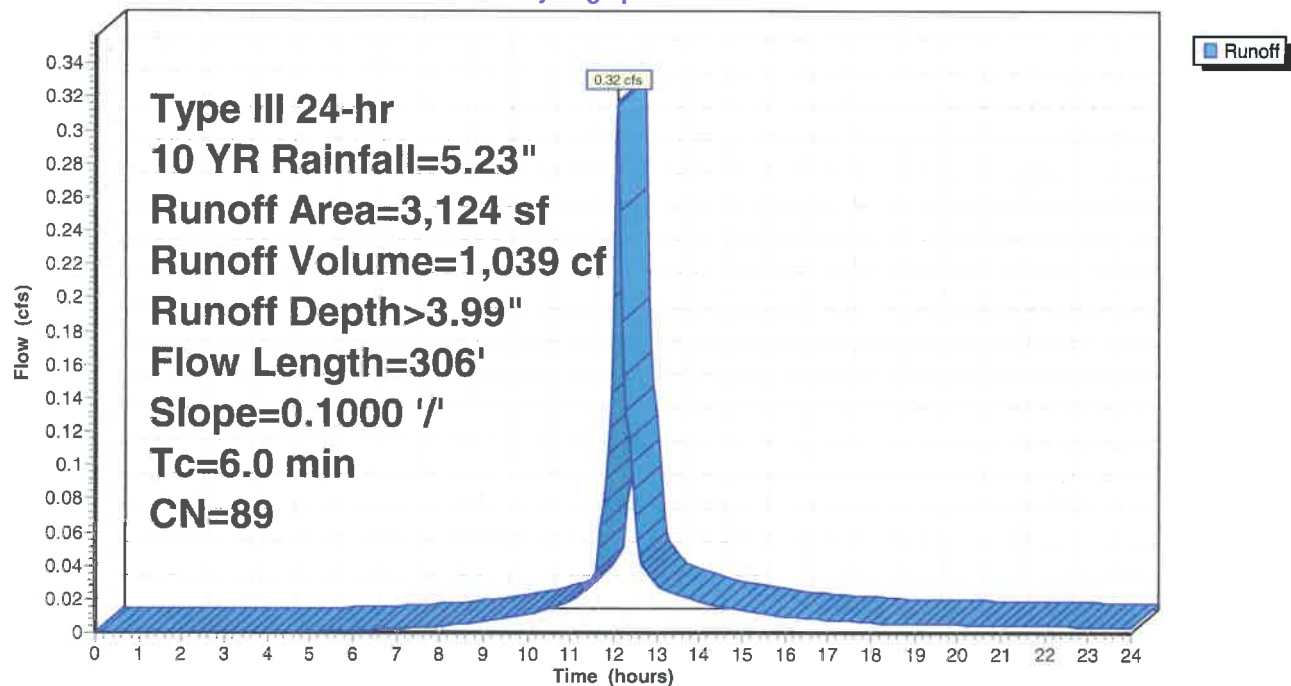
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,343 | 98 | Paved roads w/curbs & sewers, HSG B |
| 781 | 61 | >75% Grass cover, Good, HSG B |
| 3,124 | 89 | Weighted Average |
| 781 | | 25.00% Pervious Area |
| 2,343 | | 75.00% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 6S: To CB 3

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Summary for Subcatchment 7S: To CB 4

Runoff = 0.26 cfs @ 12.09 hrs, Volume= 848 cf, Depth> 3.99"

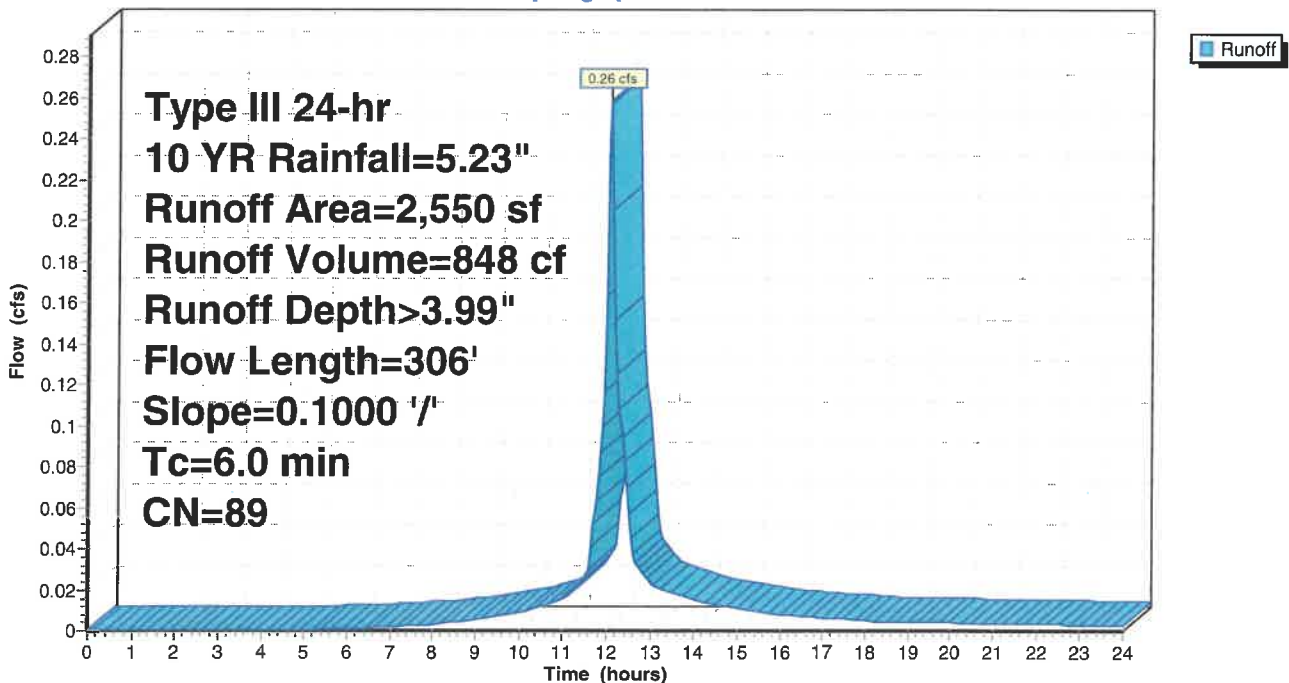
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 1,913 | 98 | Paved roads w/curbs & sewers, HSG B |
| 637 | 61 | >75% Grass cover, Good, HSG B |
| 2,550 | 89 | Weighted Average |
| 637 | | 24.98% Pervious Area |
| 1,913 | | 75.02% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 7S: To CB 4

Hydrograph



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Summary for Subcatchment 8S: To CB 6

Runoff = 0.35 cfs @ 12.09 hrs, Volume= 1,132 cf, Depth> 3.68"

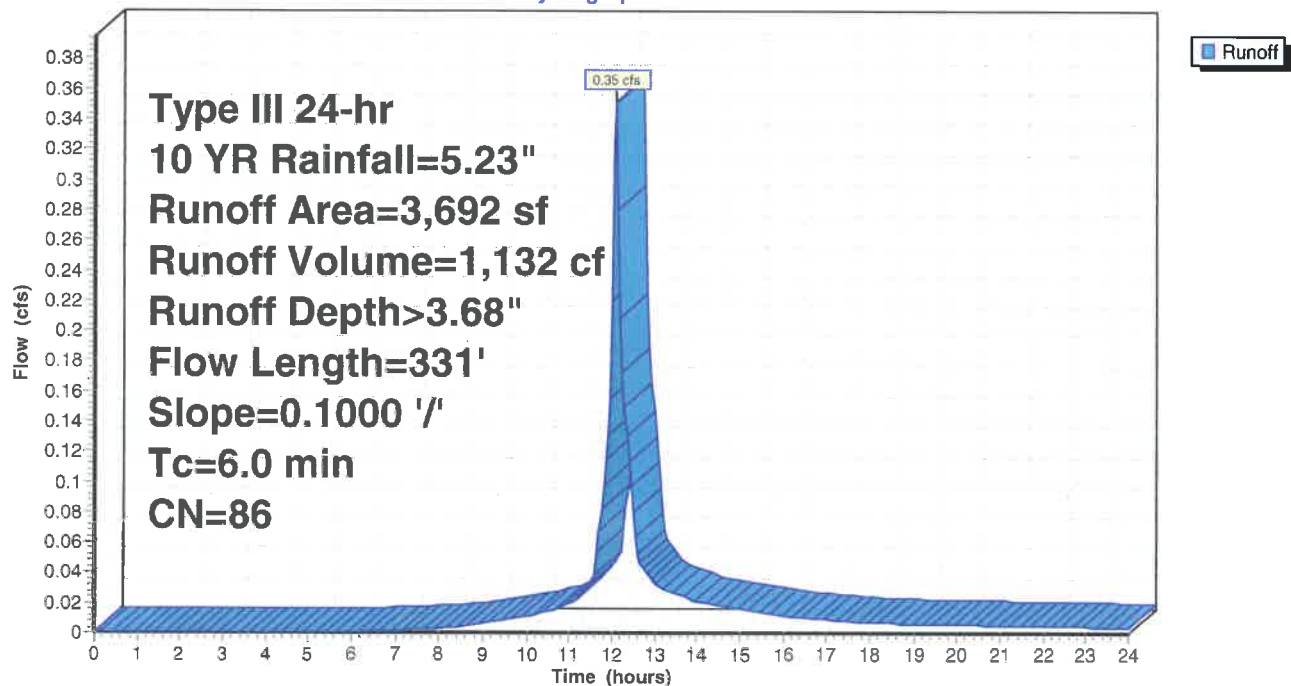
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,493 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,199 | 61 | >75% Grass cover, Good, HSG B |
| 3,692 | 86 | Weighted Average |
| 1,199 | | 32.48% Pervious Area |
| 2,493 | | 67.52% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 281 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 331 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 8S: To CB 6

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Summary for Subcatchment 9S: To CB 5

Runoff = 0.38 cfs @ 12.09 hrs, Volume= 1,288 cf, Depth> 4.42"

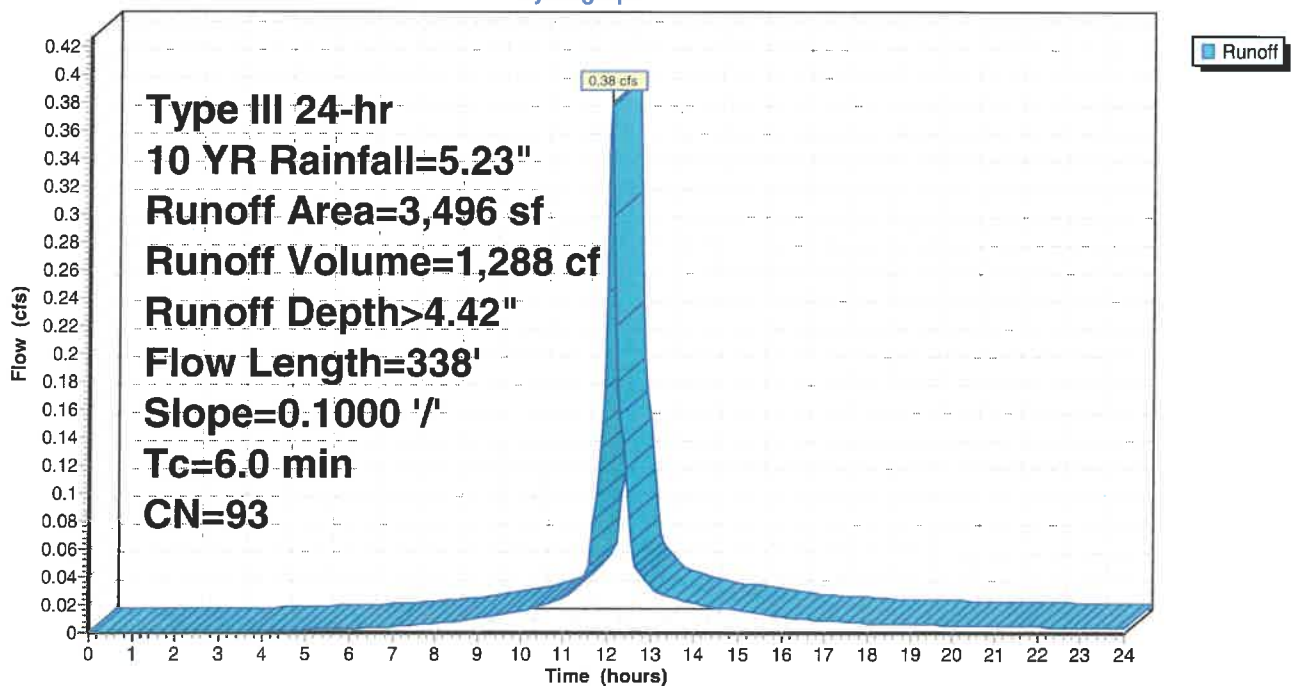
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 3,000 | 98 | Paved roads w/curbs & sewers, HSG B |
| 496 | 61 | >75% Grass cover, Good, HSG B |
| 3,496 | 93 | Weighted Average |
| 496 | | 14.19% Pervious Area |
| 3,000 | | 85.81% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 288 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 338 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 9S: To CB 5

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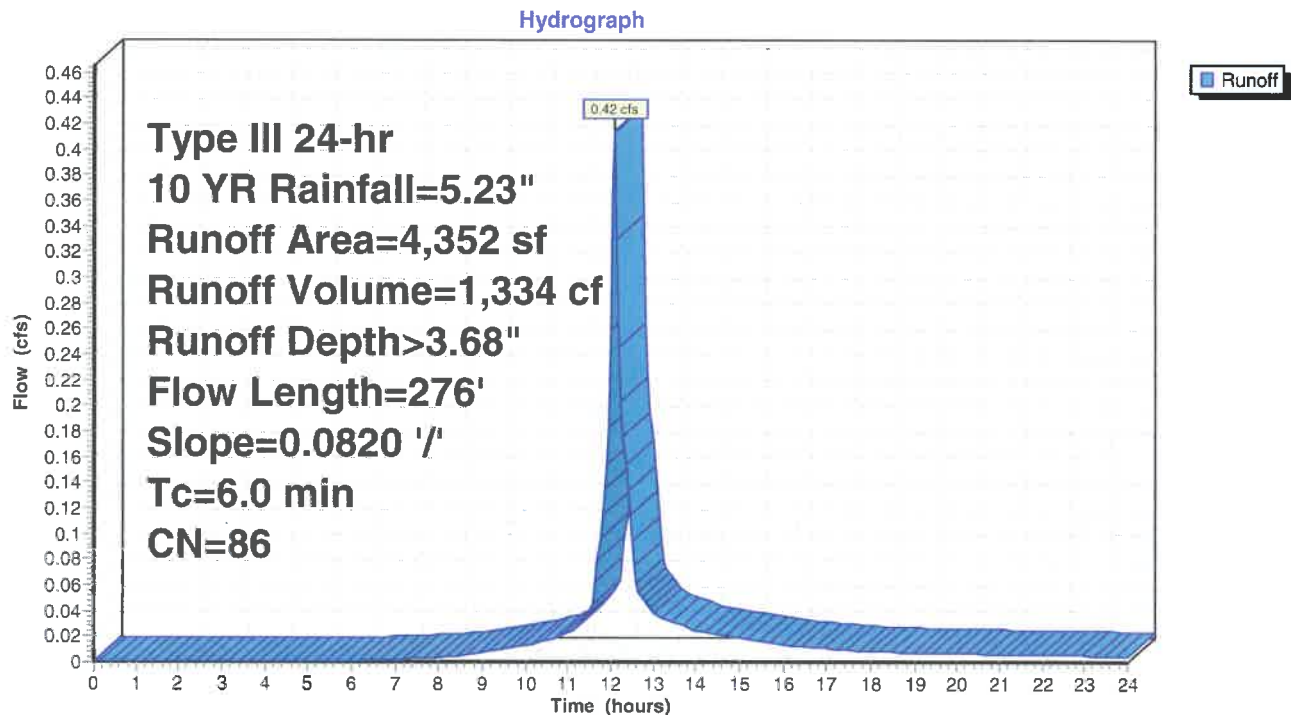
Summary for Subcatchment 10S: To CB 7

Runoff = 0.42 cfs @ 12.09 hrs, Volume= 1,334 cf, Depth> 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,363 | 61 | >75% Grass cover, Good, HSG B |
| 4,352 | 86 | Weighted Average |
| 1,363 | | 31.32% Pervious Area |
| 2,989 | | 68.68% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.6 | 226 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.0 | 276 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 10S: To CB 7

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Summary for Subcatchment 11S: To CB 8

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 1,320 cf, Depth> 3.88"

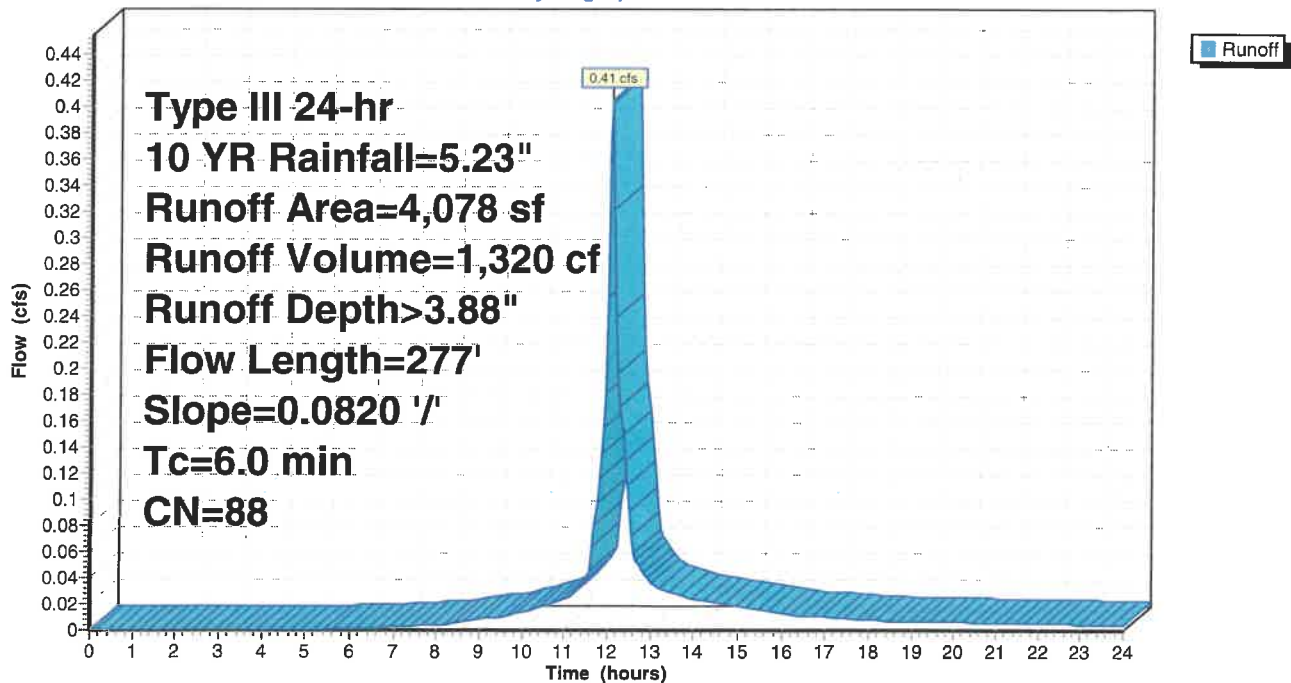
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,089 | 61 | >75% Grass cover, Good, HSG B |
| 4,078 | 88 | Weighted Average |
| 1,089 | | 26.70% Pervious Area |
| 2,989 | | 73.30% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 227 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 277 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 11S: To CB 8

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Summary for Subcatchment 12S: To CB 9

Runoff = 1.23 cfs @ 12.09 hrs, Volume= 3,928 cf, Depth> 3.48"

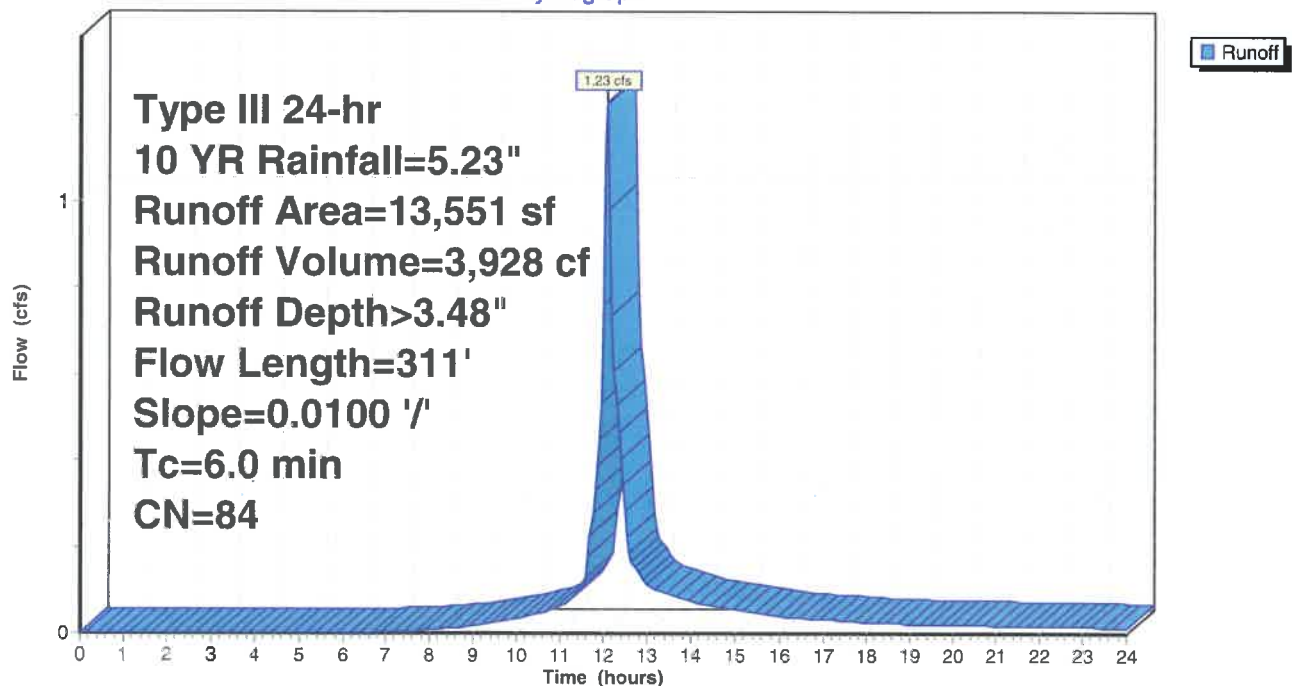
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| 5,218 | 61 | >75% Grass cover, Good, HSG B |
| 13,551 | 84 | Weighted Average |
| 5,218 | | 38.51% Pervious Area |
| 8,333 | | 61.49% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.1 | 261 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.0 | 311 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 12S: To CB 9

Hydrograph



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Summary for Subcatchment 13S: To CB 10

Runoff = 1.28 cfs @ 12.09 hrs, Volume= 4,047 cf, Depth> 3.28"

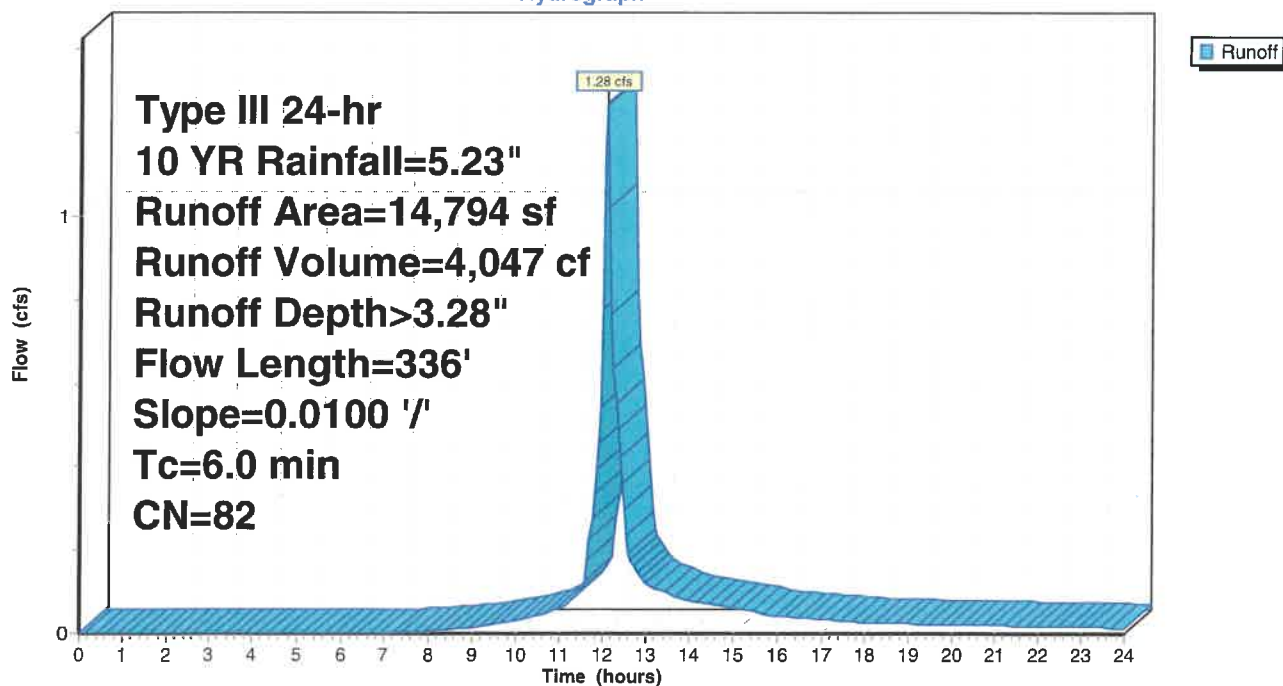
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| 6,461 | 61 | >75% Grass cover, Good, HSG B |
| 14,794 | 82 | Weighted Average |
| 6,461 | | 43.67% Pervious Area |
| 8,333 | | 56.33% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.3 | 286 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.2 | 336 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 13S: To CB 10

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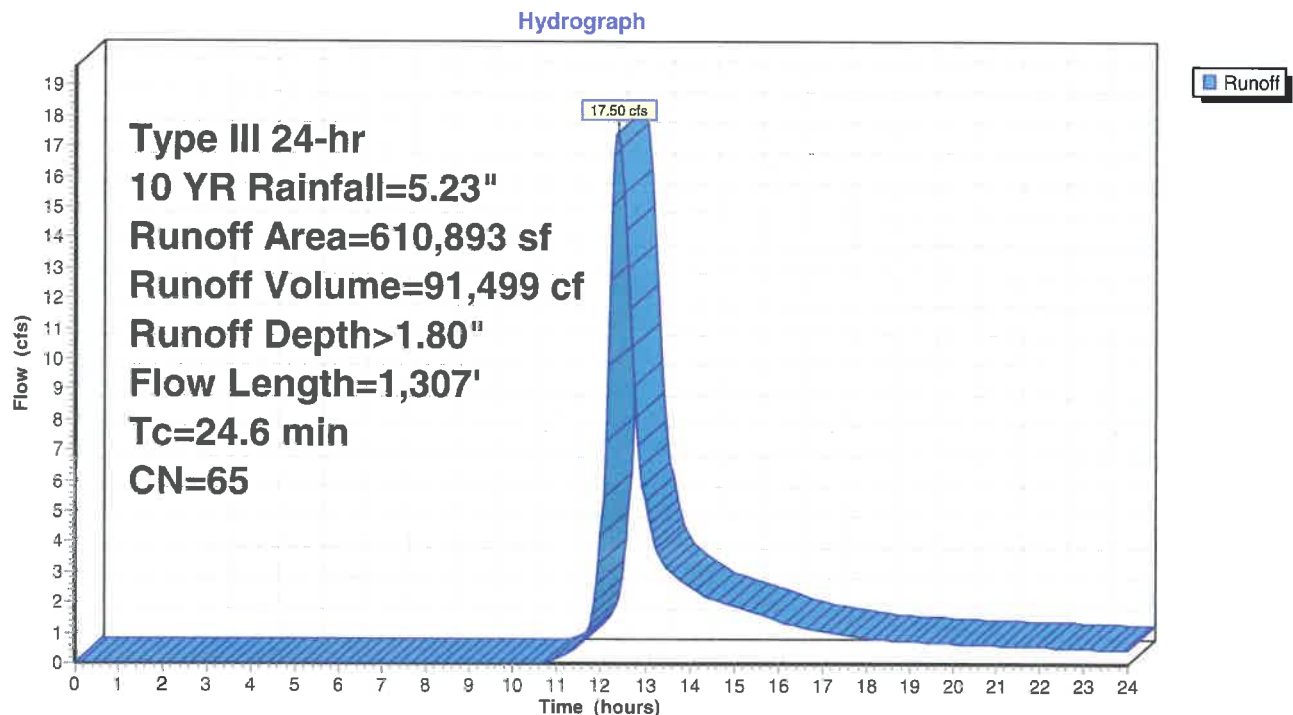
Summary for Subcatchment 14S: To Wetland

Runoff = 17.50 cfs @ 12.37 hrs, Volume= 91,499 cf, Depth> 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 326,808 | 55 | Woods, Good, HSG B |
| 260,659 | 77 | Woods, Good, HSG D |
| 23,426 | 61 | >75% Grass cover, Good, HSG B |
| 610,893 | 65 | Weighted Average |
| 610,893 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 7.9 | 648 | 0.0750 | 1.37 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 6.9 | 609 | 0.0200 | 1.47 | 1.76 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 24.6 | 1,307 | Total | | | |

Subcatchment 14S: To Wetland

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Summary for Subcatchment 15S: TO BASIN

Runoff = 2.31 cfs @ 12.21 hrs, Volume= 10,752 cf, Depth> 1.16"

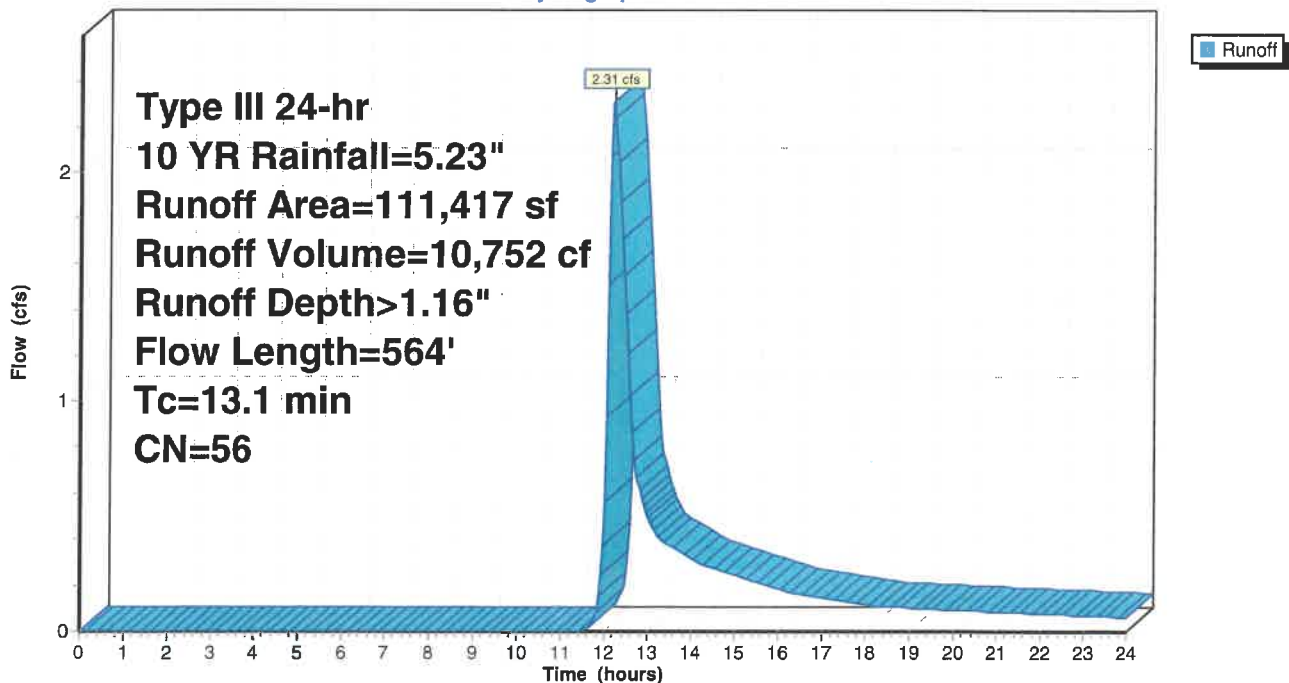
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 89,974 | 55 | Woods, Good, HSG B |
| 21,443 | 61 | >75% Grass cover, Good, HSG B |
| 111,417 | 56 | Weighted Average |
| 111,417 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.8 | 50 | 0.0620 | 0.11 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 5.1 | 467 | 0.0931 | 1.53 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.2 | 47 | 0.3190 | 3.95 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 13.1 | 564 | Total | | | |

Subcatchment 15S: TO BASIN

Hydrograph



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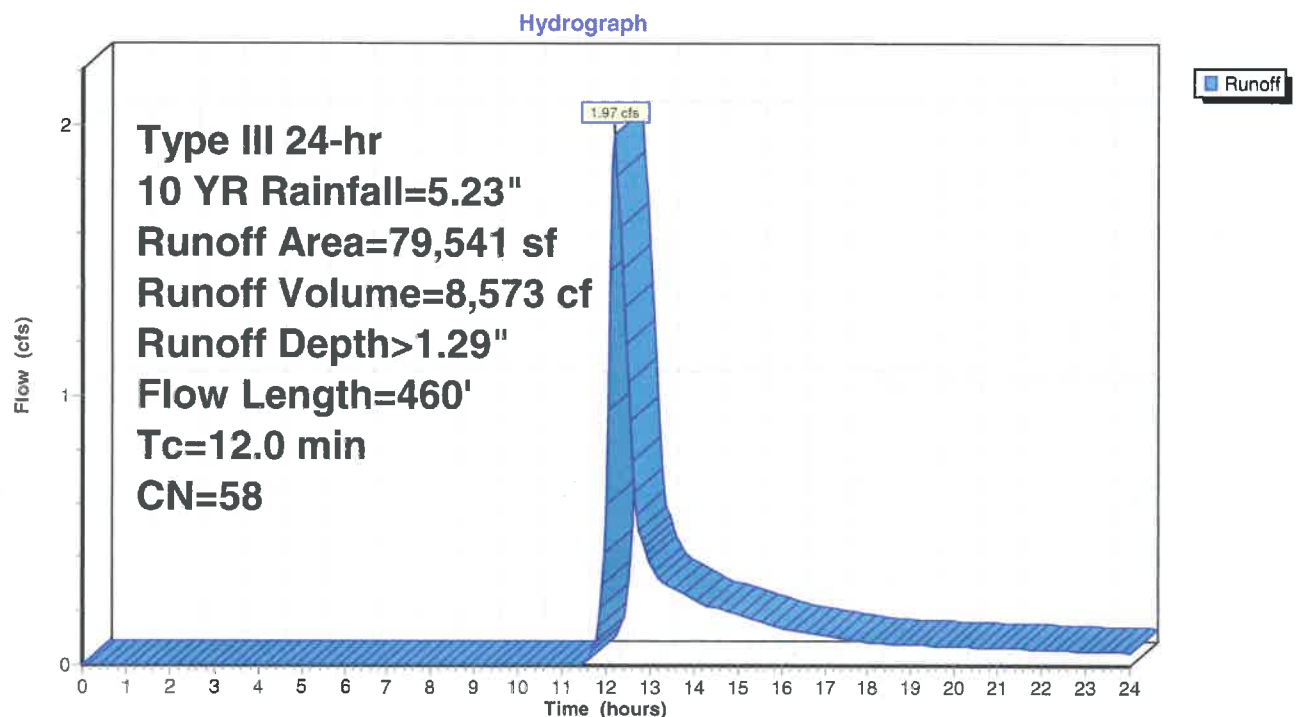
Summary for Subcatchment 16S: TO BASIN

Runoff = 1.97 cfs @ 12.19 hrs, Volume= 8,573 cf, Depth> 1.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 35,427 | 61 | >75% Grass cover, Good, HSG B |
| 44,114 | 55 | Woods, Good, HSG B |
| 79,541 | 58 | Weighted Average |
| 79,541 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 4.8 | 391 | 0.0735 | 1.36 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.1 | 19 | 0.3150 | 5.05 | | Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps |
| 12.0 | 460 | Total | | | |

Subcatchment 16S: TO BASIN

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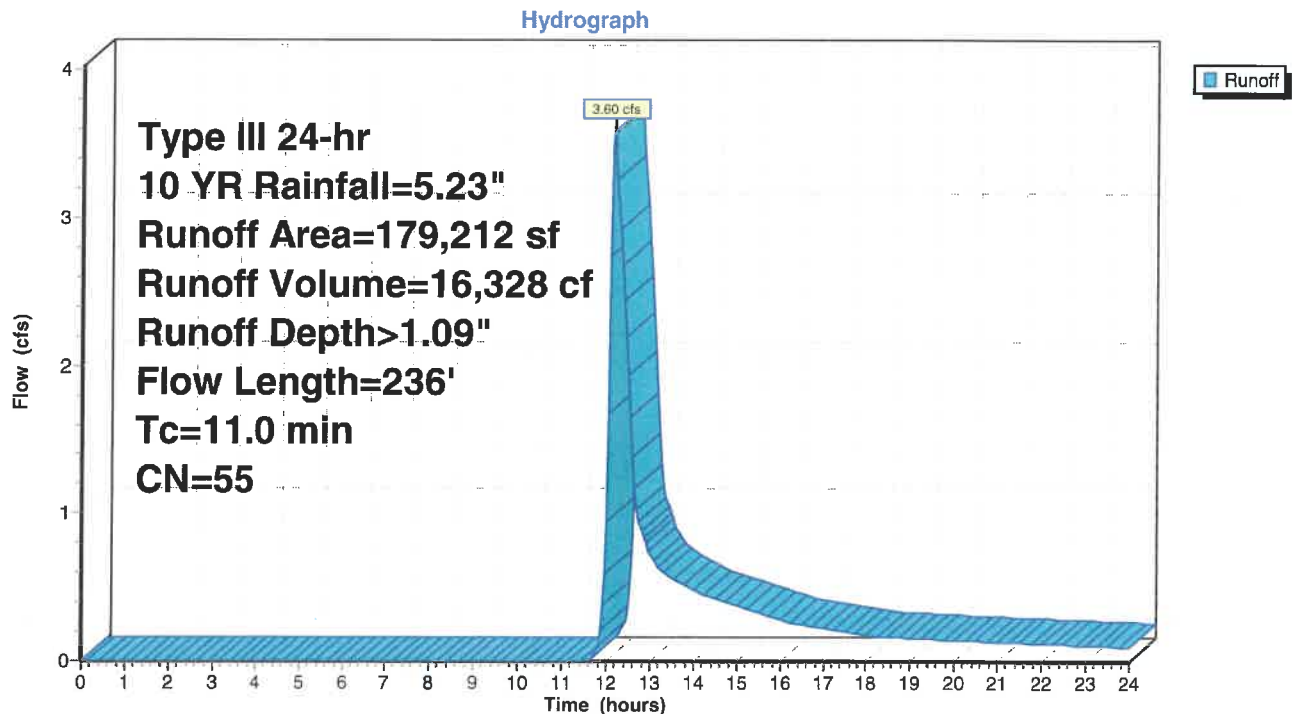
Summary for Subcatchment 17S: To Off Site

Runoff = 3.60 cfs @ 12.18 hrs, Volume= 16,328 cf, Depth> 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 179,212 | 55 | Woods, Good, HSG B |
| 179,212 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 8.5 | 50 | 0.0500 | 0.10 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.5 | 186 | 0.0600 | 1.22 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 11.0 | 236 | Total | | | |

Subcatchment 17S: To Off Site

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Summary for Subcatchment 19S: Area To Basin3

Runoff = 1.16 cfs @ 12.10 hrs, Volume= 3,933 cf, Depth> 1.51"

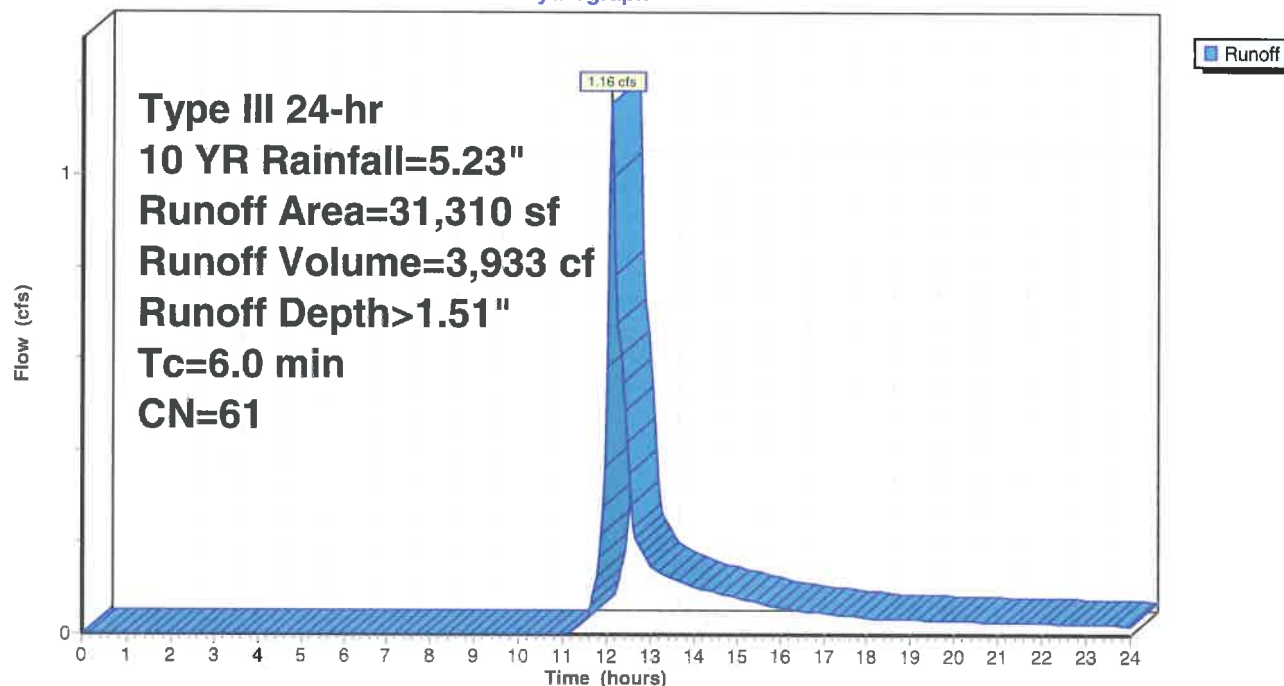
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 10 YR Rainfall=5.23"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 31,310 | 61 | >75% Grass cover, Good, HSG B |
| 31,310 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|-------------------|
| 6.0 | | | | | Direct Entry, a-b |

Subcatchment 19S: Area To Basin3

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Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 1.50" for 10 YR event
Inflow = 10.78 cfs @ 12.43 hrs, Volume= 61,489 cf
Outflow = 10.77 cfs @ 12.43 hrs, Volume= 61,476 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.41 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 2.91 fps, Avg. Travel Time= 0.3 min

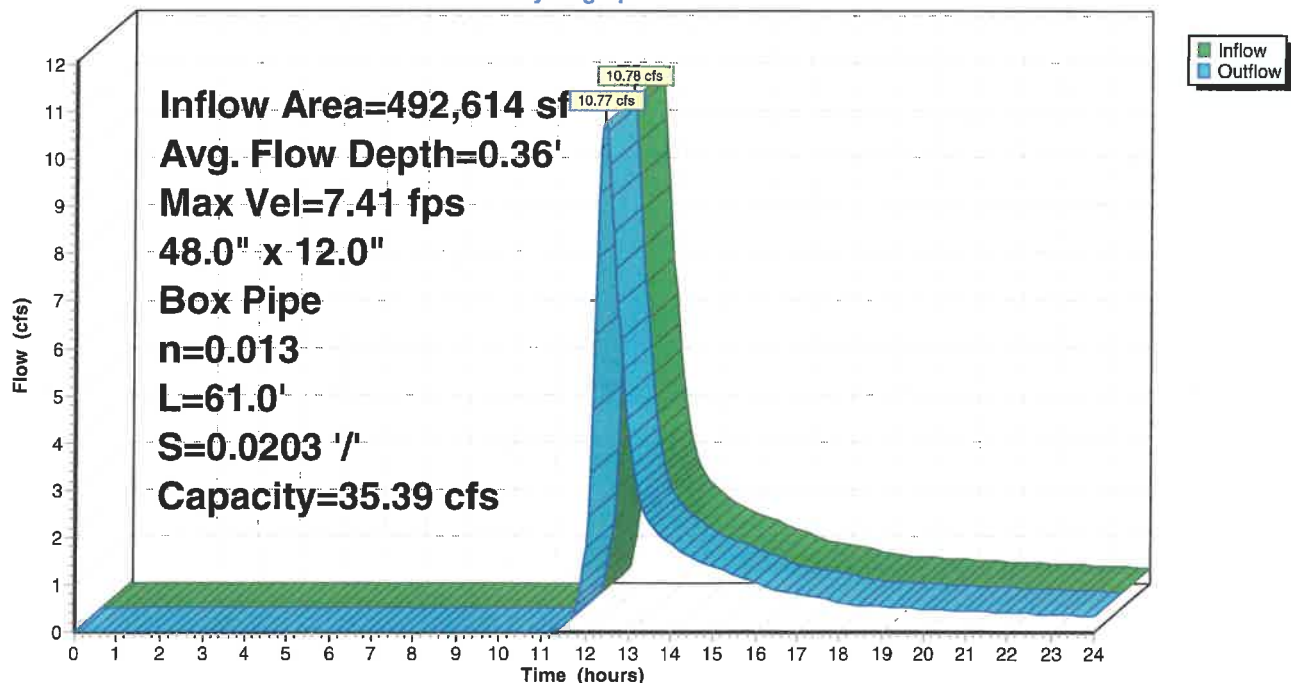
Peak Storage= 89 cf @ 12.43 hrs
Average Depth at Peak Storage= 0.36' , Surface Width= 4.00'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe
n= 0.013 Concrete, trowel finish
Length= 61.0' Slope= 0.0203 '/'
Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

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Summary for Reach 2R: New Box Cumvert

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 1.15" for 10 YR event
Inflow = 25.27 cfs @ 13.17 hrs, Volume= 263,929 cf
Outflow = 25.26 cfs @ 13.18 hrs, Volume= 263,904 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 10.04 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 5.42 fps, Avg. Travel Time= 0.1 min

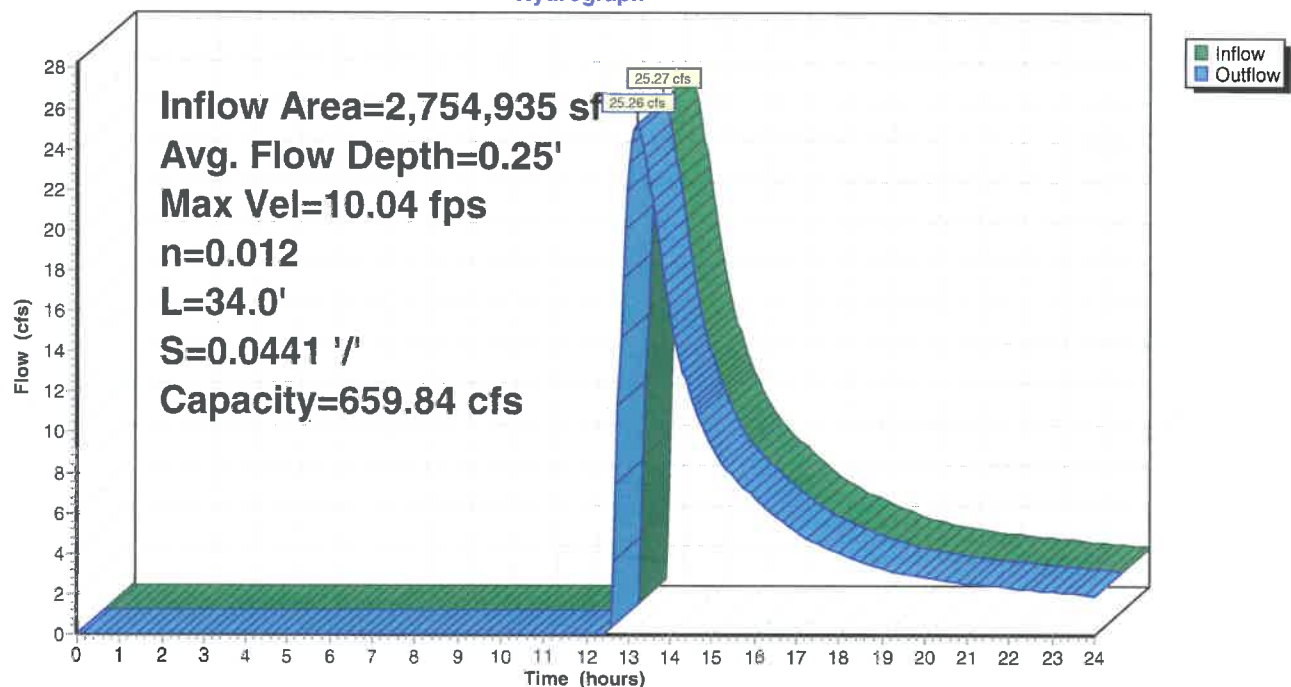
Peak Storage= 86 cf @ 13.18 hrs
Average Depth at Peak Storage= 0.25' , Surface Width= 10.00'
Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 659.84 cfs

10.00' x 2.00' deep channel, n= 0.012 Concrete pipe, finished
Length= 34.0' Slope= 0.0441 '/'
Inlet Invert= 294.00', Outlet Invert= 292.50'



Reach 2R: New Box Cumvert

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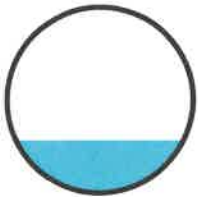
Summary for Reach 3R: CB1 to DMH 1

Inflow Area = 13,094 sf, 84.66% Impervious, Inflow Depth > 4.31" for 10 YR event
Inflow = 1.41 cfs @ 12.09 hrs, Volume= 4,704 cf
Outflow = 1.41 cfs @ 12.09 hrs, Volume= 4,704 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.68 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.54 fps, Avg. Travel Time= 0.0 min

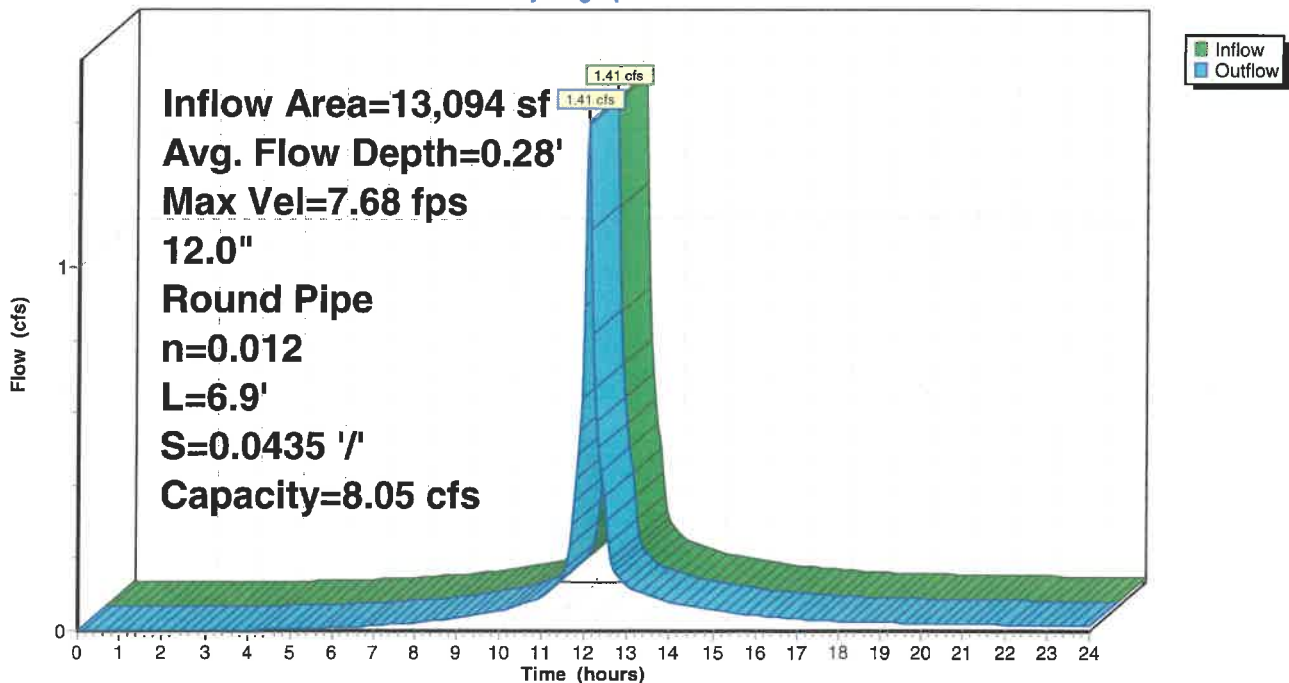
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.28', Surface Width= 0.90'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 3R: CB1 to DMH 1

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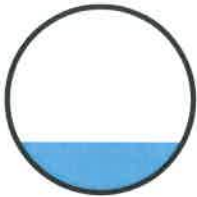
Summary for Reach 4R: CB2 to DMH 1

Inflow Area = 11,319 sf, 94.70% Impervious, Inflow Depth > 4.76" for 10 YR event
Inflow = 1.28 cfs @ 12.09 hrs, Volume= 4,488 cf
Outflow = 1.28 cfs @ 12.09 hrs, Volume= 4,488 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.48 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.47 fps, Avg. Travel Time= 0.0 min

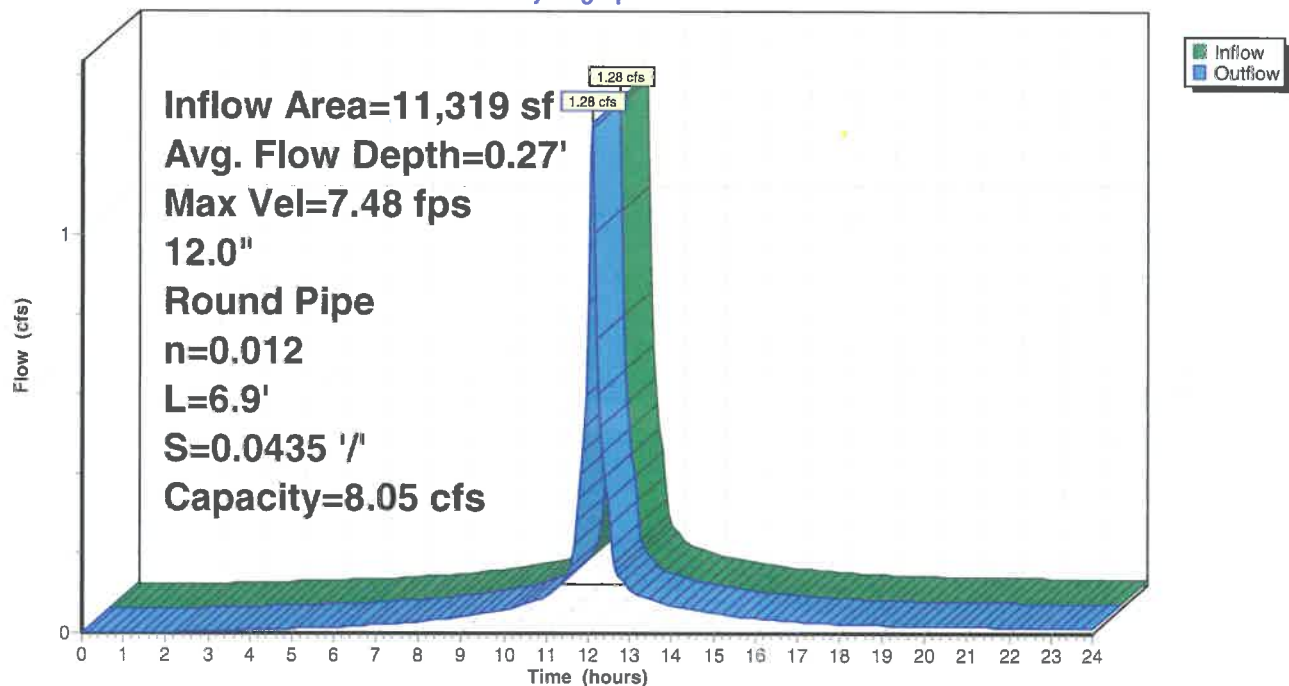
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.27' , Surface Width= 0.89'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 4R: CB2 to DMH 1

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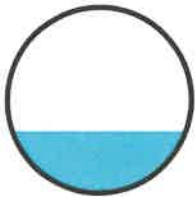
Summary for Reach 5R: DMH1 to DMH 2

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 4.52" for 10 YR event
Inflow = 2.68 cfs @ 12.09 hrs, Volume= 9,192 cf
Outflow = 2.66 cfs @ 12.09 hrs, Volume= 9,190 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.24 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 1.70 fps, Avg. Travel Time= 0.7 min

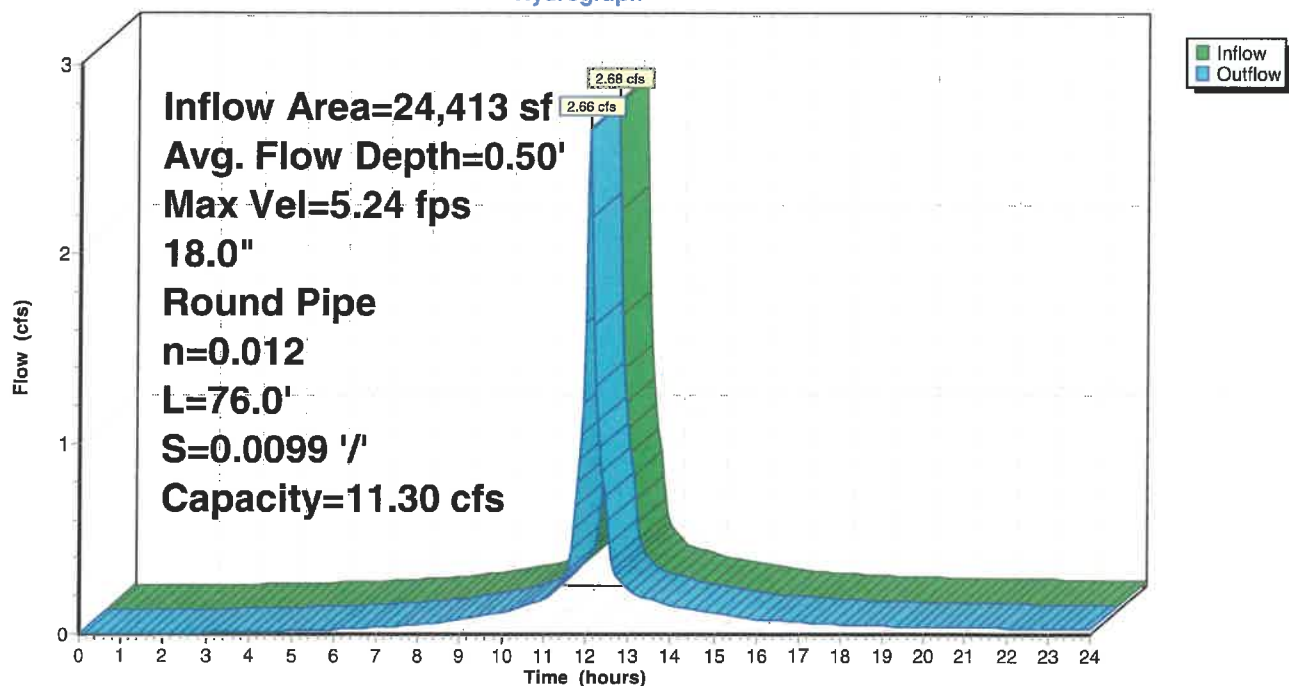
Peak Storage= 39 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.50' , Surface Width= 1.41'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 11.30 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 76.0' Slope= 0.0099 '/'
Inlet Invert= 291.75', Outlet Invert= 291.00'



Reach 5R: DMH1 to DMH 2

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Summary for Reach 6R: CB3 to DMH 2

Inflow Area = 3,124 sf, 75.00% Impervious, Inflow Depth > 3.99" for 10 YR event
Inflow = 0.32 cfs @ 12.09 hrs, Volume= 1,039 cf
Outflow = 0.32 cfs @ 12.09 hrs, Volume= 1,039 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.11 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.72 fps, Avg. Travel Time= 0.1 min

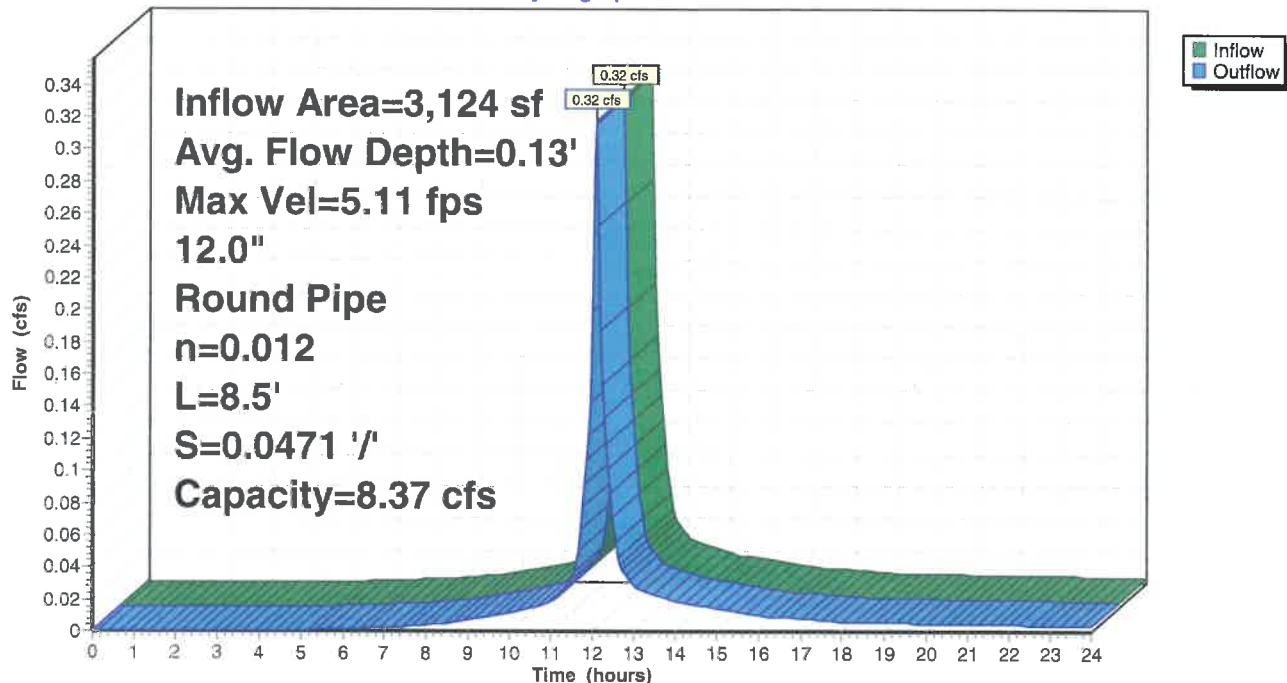
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.13' , Surface Width= 0.68'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 8.5' Slope= 0.0471 '/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 6R: CB3 to DMH 2

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Summary for Reach 7R: CB4 to DMH 2

Inflow Area = 2,550 sf, 75.02% Impervious, Inflow Depth > 3.99" for 10 YR event
Inflow = 0.26 cfs @ 12.09 hrs, Volume= 848 cf
Outflow = 0.26 cfs @ 12.09 hrs, Volume= 848 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.04 fps, Min. Travel Time= 0.1 min

Avg. Velocity = 1.36 fps, Avg. Travel Time= 0.2 min

Peak Storage= 1 cf @ 12.09 hrs

Average Depth at Peak Storage= 0.14', Surface Width= 0.69'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 6.52 cfs

12.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

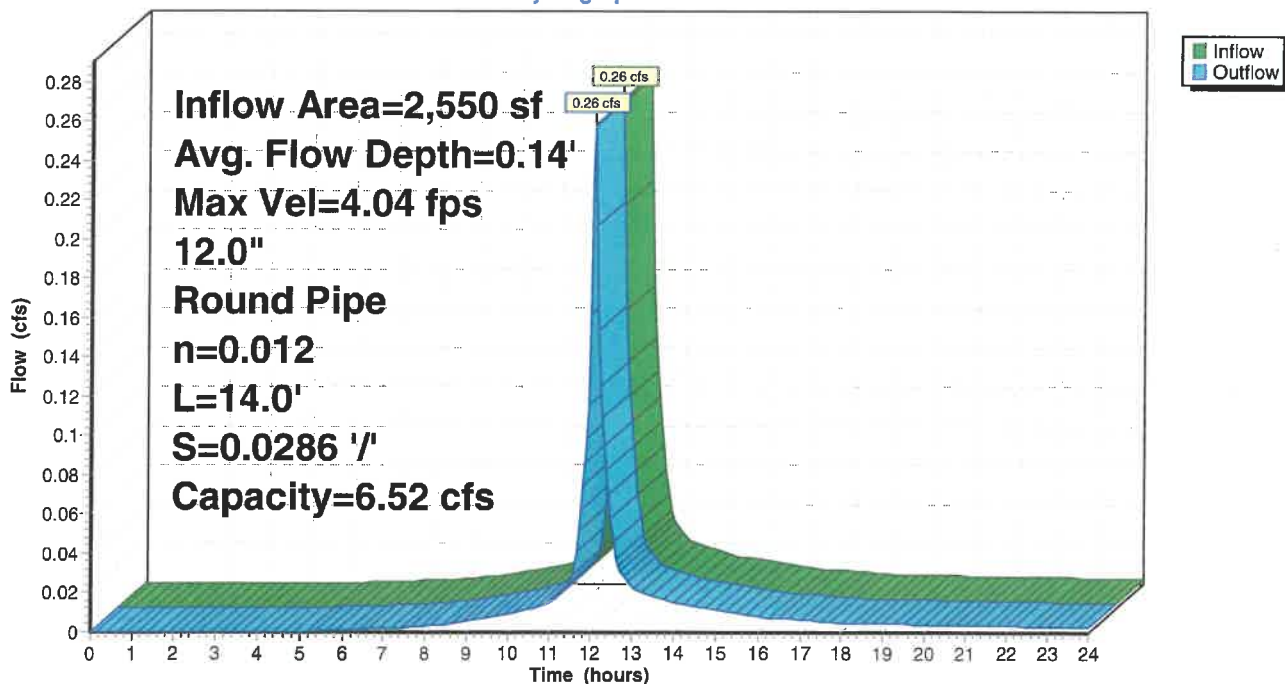
Length= 14.0' Slope= 0.0286 1'

Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 7R: CB4 to DMH 2

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Summary for Reach 8R: DMH 2 TO DMH 7

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 4.02" for 10 YR event
Inflow = 1.29 cfs @ 12.10 hrs, Volume= 4,305 cf
Outflow = 1.25 cfs @ 12.12 hrs, Volume= 4,302 cf, Atten= 4%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.46 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 1.77 fps, Avg. Travel Time= 2.3 min

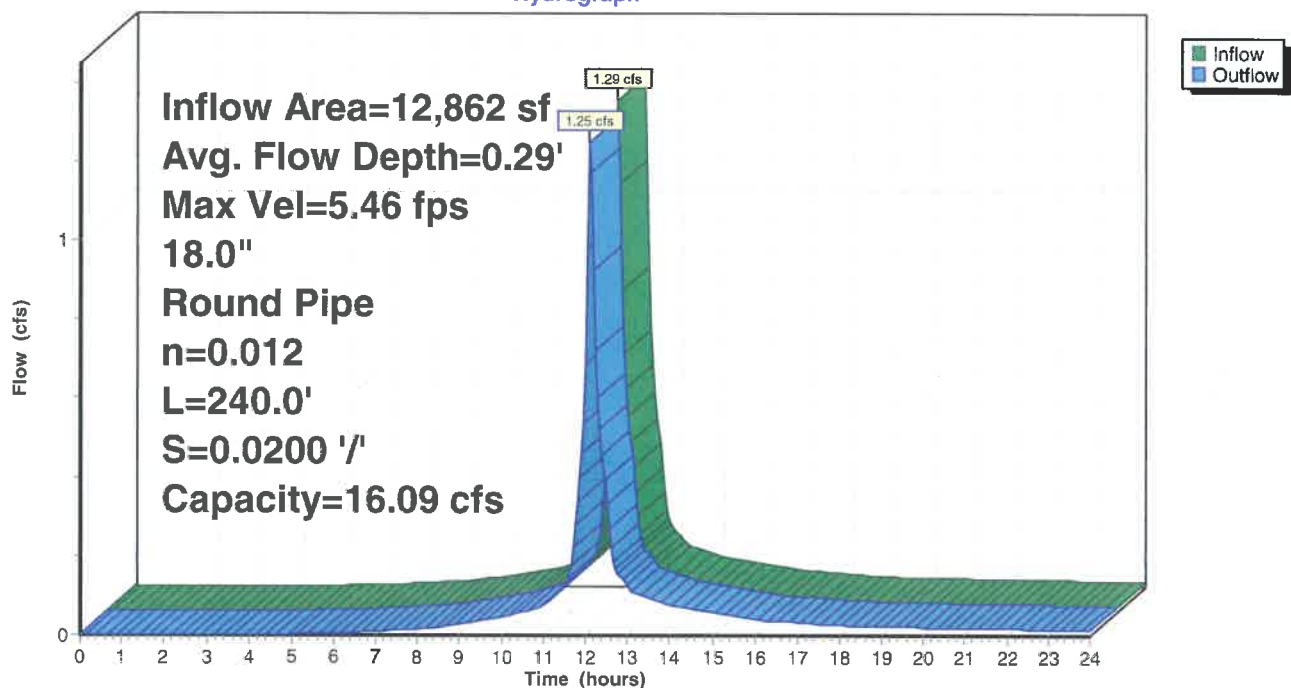
Peak Storage= 57 cf @ 12.11 hrs
Average Depth at Peak Storage= 0.29' , Surface Width= 1.18'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 16.09 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0200 '/'
Inlet Invert= 294.80', Outlet Invert= 290.00'



Reach 8R: DMH 2 TO DMH 7

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Summary for Reach 9R: DMH 3 to DMH 2

Inflow Area = 7,188 sf, 76.42% Impervious, Inflow Depth > 4.04" for 10 YR event
Inflow = 0.73 cfs @ 12.09 hrs, Volume= 2,420 cf
Outflow = 0.72 cfs @ 12.10 hrs, Volume= 2,419 cf, Atten= 2%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 7.91 fps, Min. Travel Time= 0.4 min

Avg. Velocity = 2.56 fps, Avg. Travel Time= 1.4 min

Peak Storage= 19 cf @ 12.10 hrs

Average Depth at Peak Storage= 0.18', Surface Width= 0.76'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

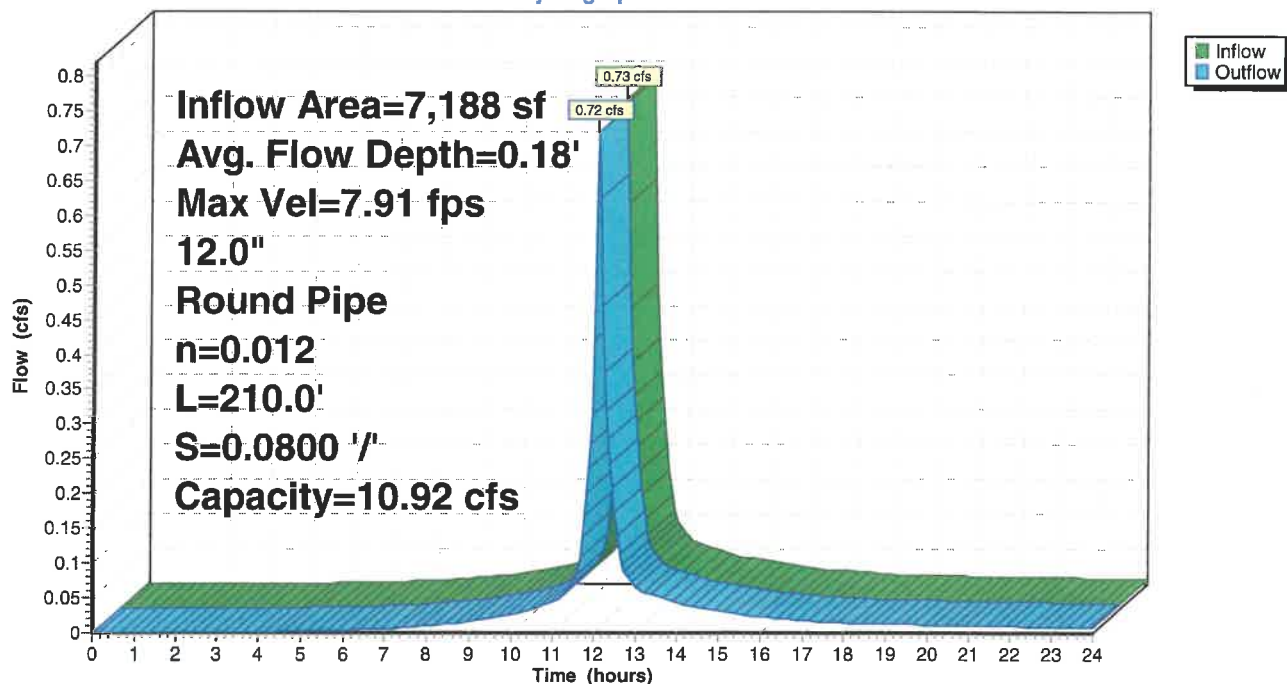
Length= 210.0' Slope= 0.0800 1/'

Inlet Invert= 315.40', Outlet Invert= 298.60'



Reach 9R: DMH 3 to DMH 2

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Summary for Reach 10R: CB5 to DMH 3

Inflow Area = 3,692 sf, 67.52% Impervious, Inflow Depth > 3.68" for 10 YR event
Inflow = 0.35 cfs @ 12.09 hrs, Volume= 1,132 cf
Outflow = 0.35 cfs @ 12.09 hrs, Volume= 1,132 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.15 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.75 fps, Avg. Travel Time= 0.1 min

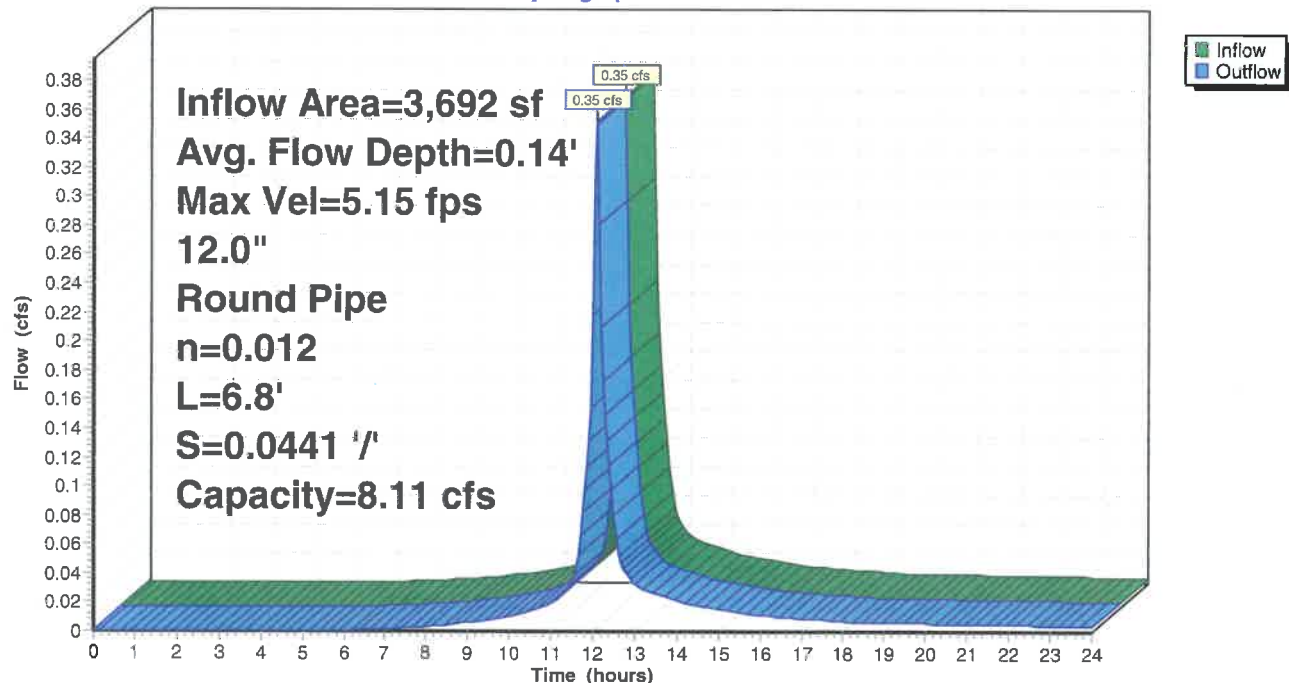
Peak Storage= 0 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.14', Surface Width= 0.70'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.11 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.8' Slope= 0.0441 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 10R: CB5 to DMH 3

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Summary for Reach 11R: CB6 to DMH 3

Inflow Area = 3,496 sf, 85.81% Impervious, Inflow Depth > 4.42" for 10 YR event
Inflow = 0.38 cfs @ 12.09 hrs, Volume= 1,288 cf
Outflow = 0.38 cfs @ 12.09 hrs, Volume= 1,288 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.15 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.37 fps, Avg. Travel Time= 0.2 min

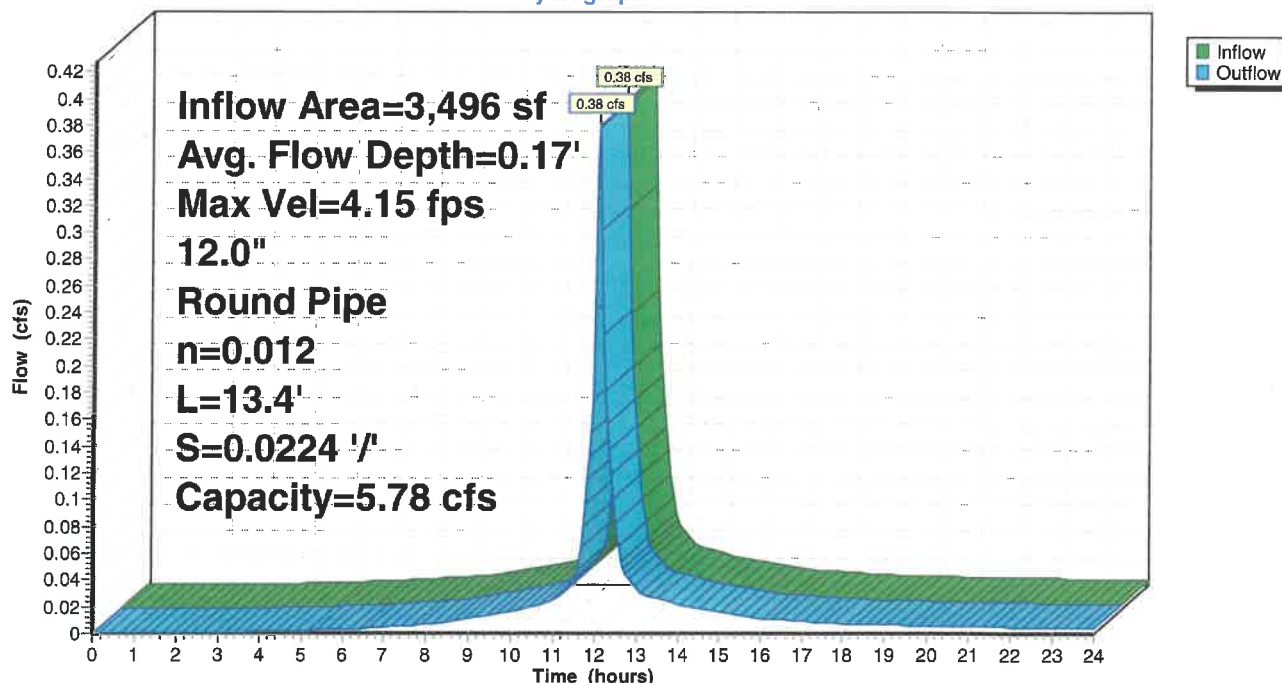
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.17' , Surface Width= 0.76'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.78 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0224 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 11R: CB6 to DMH 3

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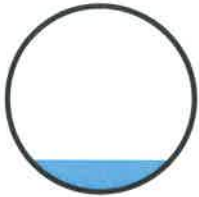
Summary for Reach 12R: DMH 7 TO BASIN

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 4.01" for 10 YR event
Inflow = 1.25 cfs @ 12.12 hrs, Volume= 4,302 cf
Outflow = 1.21 cfs @ 12.14 hrs, Volume= 4,300 cf, Atten= 3%, Lag= 1.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.11 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 2.03 fps, Avg. Travel Time= 2.0 min

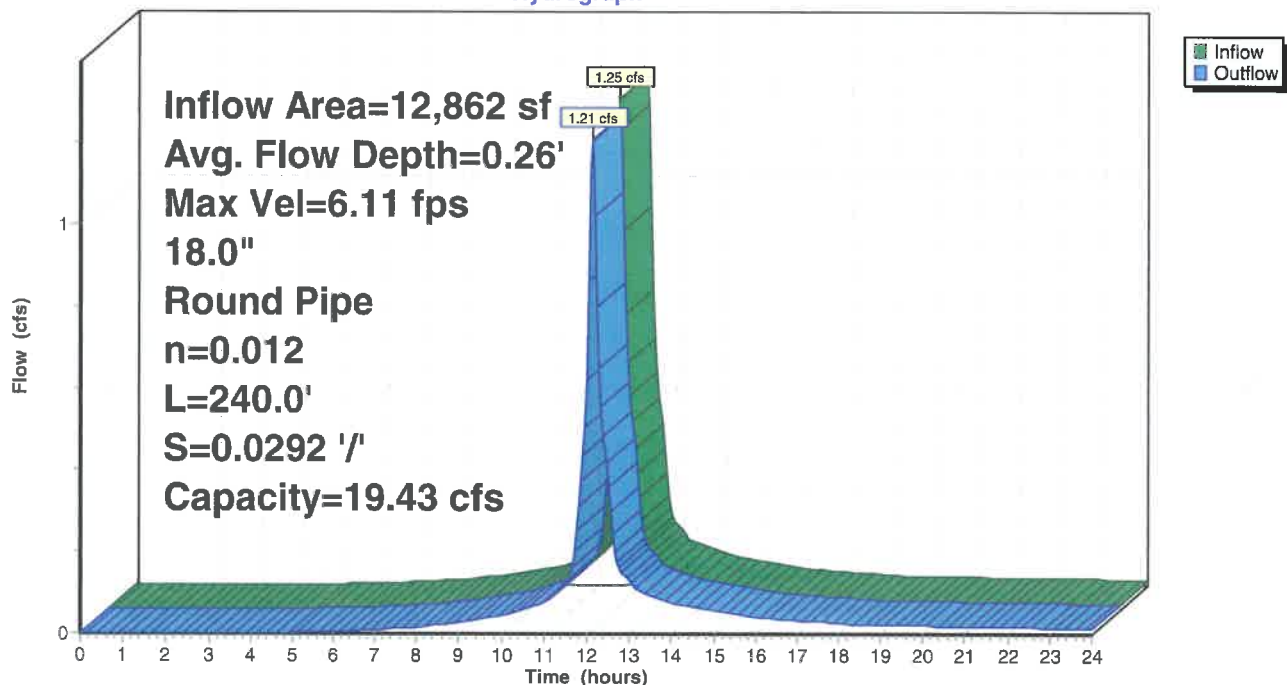
Peak Storage= 48 cf @ 12.13 hrs
Average Depth at Peak Storage= 0.26' , Surface Width= 1.13'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 19.43 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0292 '/'
Inlet Invert= 290.00', Outlet Invert= 283.00'



Reach 12R: DMH 7 TO BASIN

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Summary for Reach 13R: CB7 TO DMH 4

Inflow Area = 4,352 sf, 68.68% Impervious, Inflow Depth > 3.68" for 10 YR event
Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,334 cf
Outflow = 0.41 cfs @ 12.09 hrs, Volume= 1,334 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 1.69 fps, Avg. Travel Time= 0.1 min

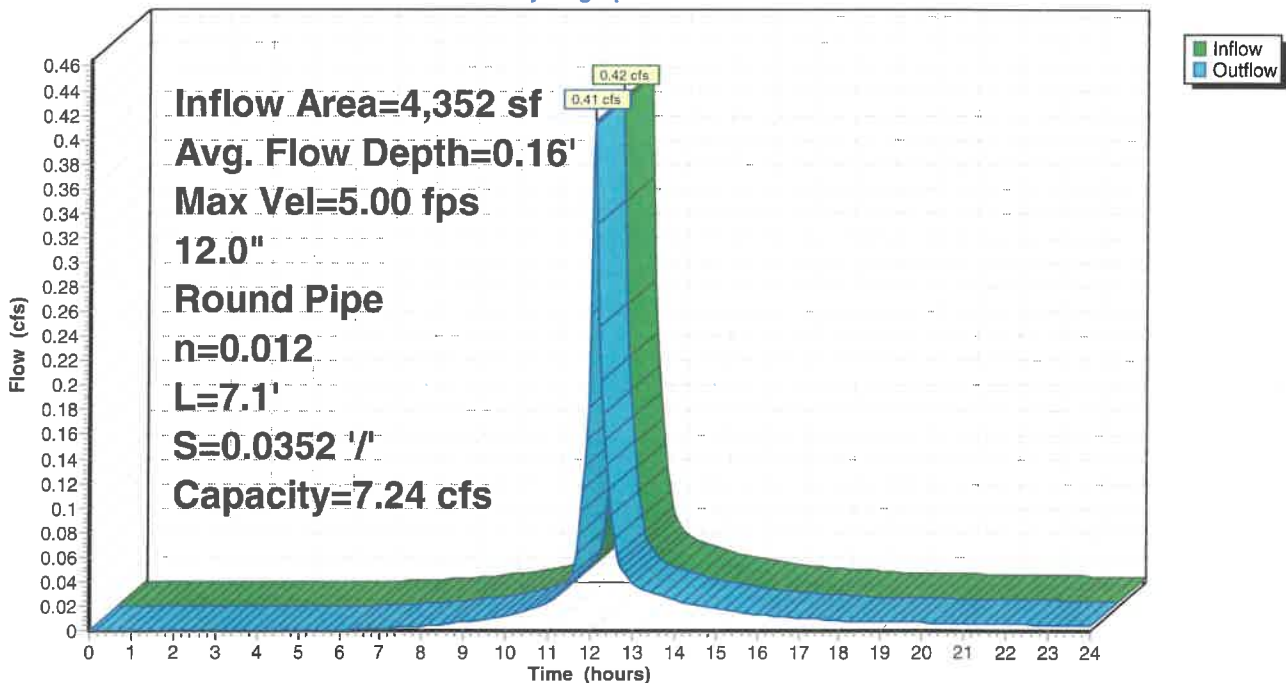
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.16', Surface Width= 0.74'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.24 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.1' Slope= 0.0352 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 13R: CB7 TO DMH 4

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Summary for Reach 14R: CB 8 TO DMH 4

Inflow Area = 4,078 sf, 73.30% Impervious, Inflow Depth > 3.88" for 10 YR event
Inflow = 0.41 cfs @ 12.09 hrs, Volume= 1,320 cf
Outflow = 0.41 cfs @ 12.09 hrs, Volume= 1,320 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.97 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.33 fps, Avg. Travel Time= 0.2 min

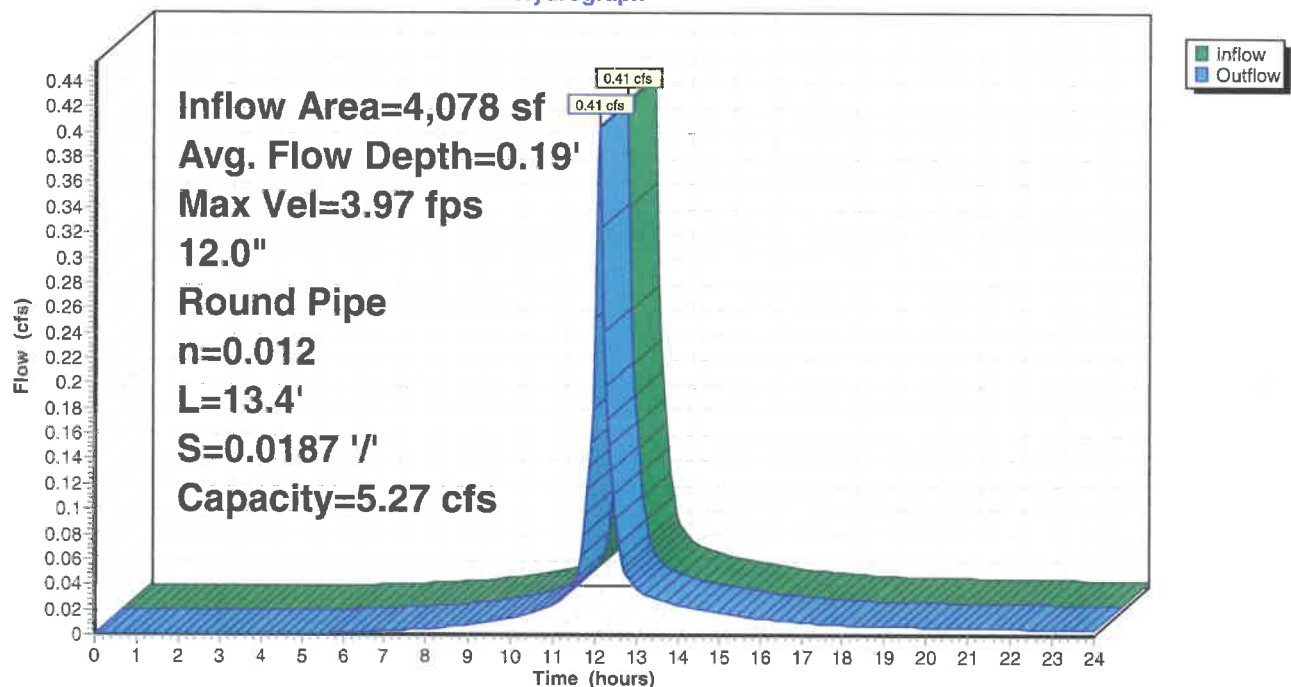
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.19' , Surface Width= 0.78'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.27 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0187 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 14R: CB 8 TO DMH 4

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Summary for Reach 15R: DMH 4 TO DMH 5

Inflow Area = 8,430 sf, 70.91% Impervious, Inflow Depth > 3.78" for 10 YR event
Inflow = 0.82 cfs @ 12.09 hrs, Volume= 2,655 cf
Outflow = 0.82 cfs @ 12.10 hrs, Volume= 2,654 cf, Atten= 1%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.18 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.73 fps, Avg. Travel Time= 0.6 min

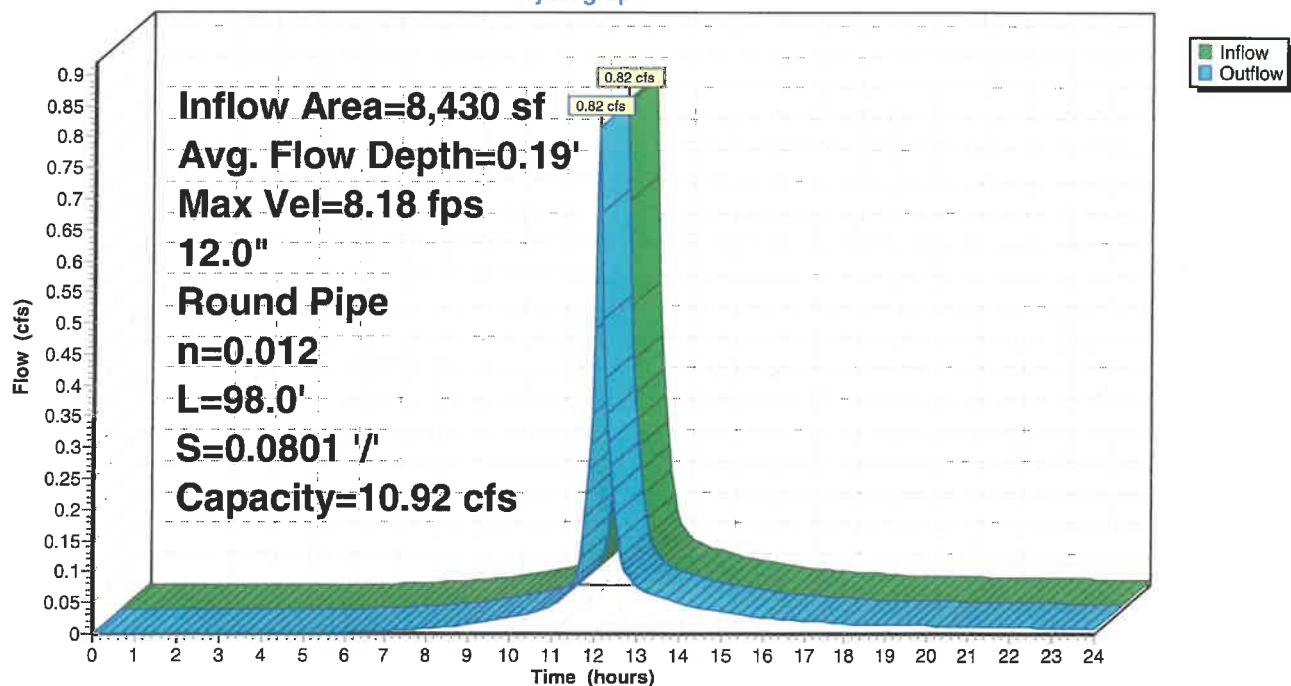
Peak Storage= 10 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.19', Surface Width= 0.78'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 98.0' Slope= 0.0801 '/'
Inlet Invert= 316.20', Outlet Invert= 308.35'



Reach 15R: DMH 4 TO DMH 5

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Summary for Reach 16R: DMH 5 TO DMH 6

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 3.47" for 10 YR event
Inflow = 3.30 cfs @ 12.10 hrs, Volume= 10,628 cf
Outflow = 3.18 cfs @ 12.12 hrs, Volume= 10,620 cf, Atten= 4%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.64 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 1.90 fps, Avg. Travel Time= 2.1 min

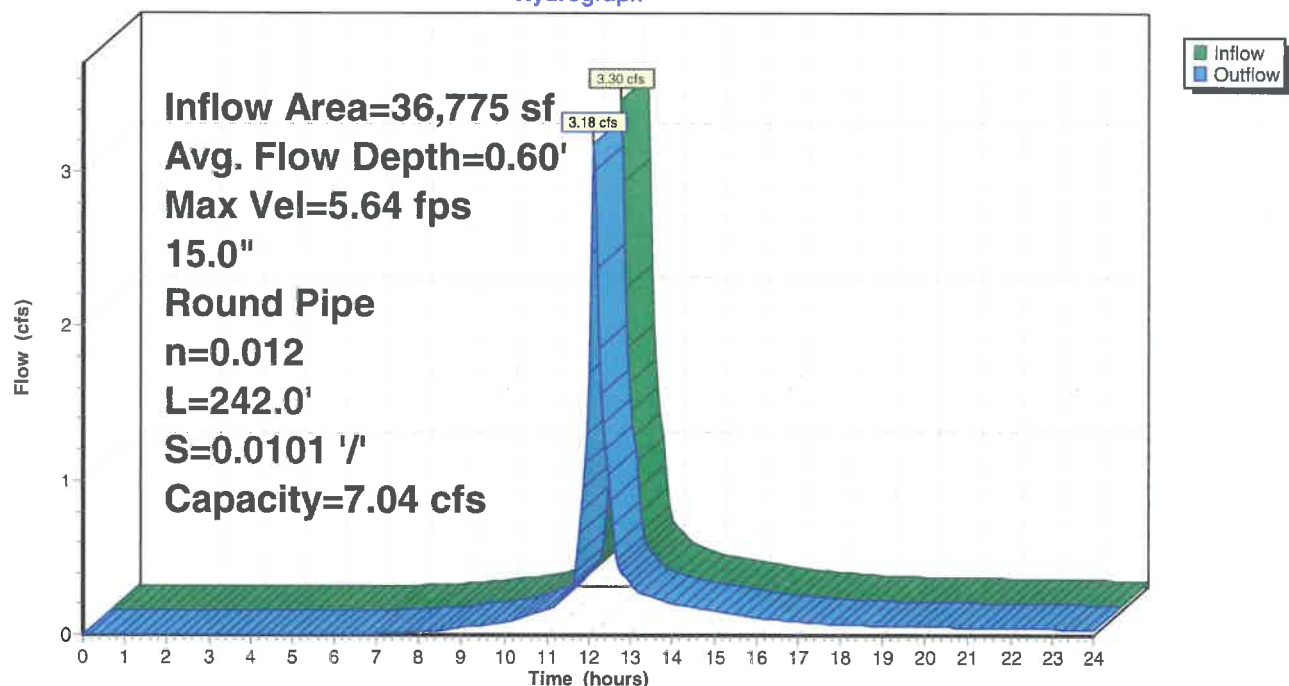
Peak Storage= 141 cf @ 12.11 hrs
Average Depth at Peak Storage= 0.60' , Surface Width= 1.25'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.04 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 242.0' Slope= 0.0101 1/'
Inlet Invert= 308.10', Outlet Invert= 305.65'



Reach 16R: DMH 5 TO DMH 6

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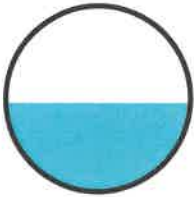
Summary for Reach 17R: DMH 6 TO DMH 5

Inflow Area = 28,345 sf, 58.80% Impervious, Inflow Depth > 3.38" for 10 YR event
Inflow = 2.51 cfs @ 12.09 hrs, Volume= 7,975 cf
Outflow = 2.48 cfs @ 12.10 hrs, Volume= 7,974 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.79 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.37 fps, Avg. Travel Time= 0.7 min

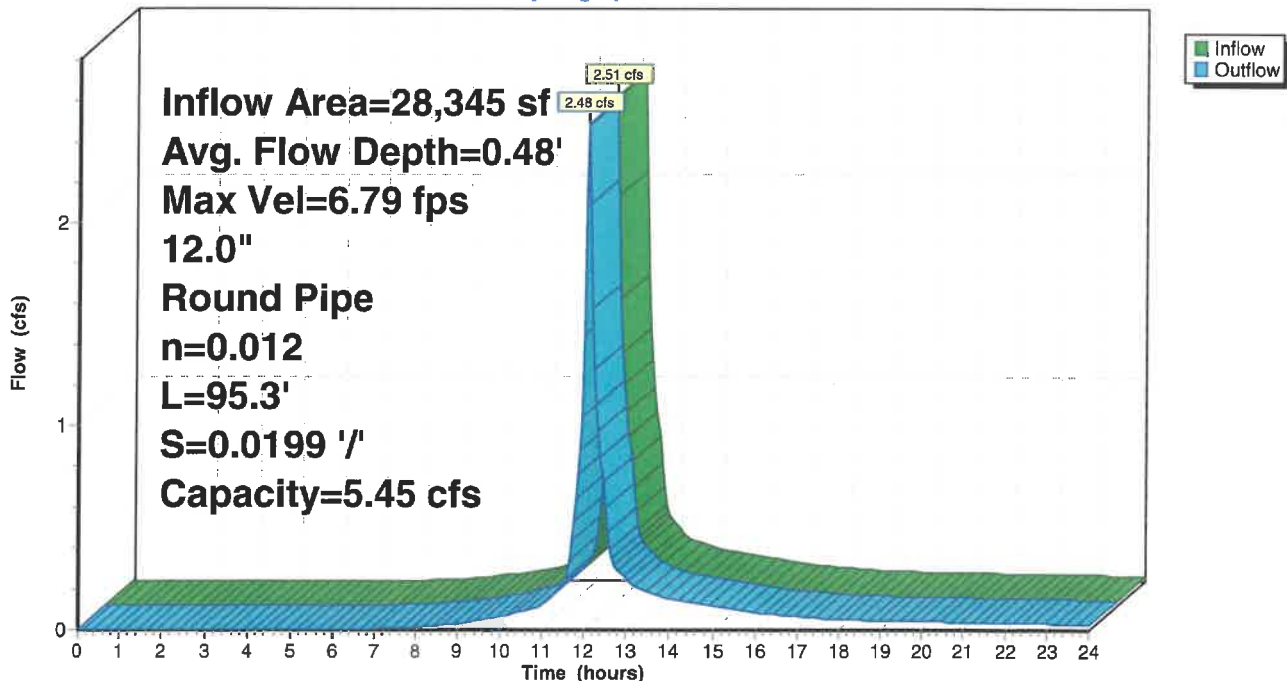
Peak Storage= 35 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.48' , Surface Width= 1.00'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.45 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 95.3' Slope= 0.0199 '/
Inlet Invert= 310.25', Outlet Invert= 308.35'



Reach 17R: DMH 6 TO DMH 5

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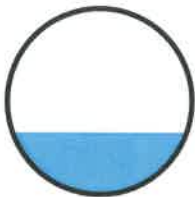
Summary for Reach 18R: CB 10 TO DMH 6

Inflow Area = 14,794 sf, 56.33% Impervious, Inflow Depth > 3.28" for 10 YR event
Inflow = 1.28 cfs @ 12.09 hrs, Volume= 4,047 cf
Outflow = 1.27 cfs @ 12.09 hrs, Volume= 4,047 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.60 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.96 fps, Avg. Travel Time= 0.1 min

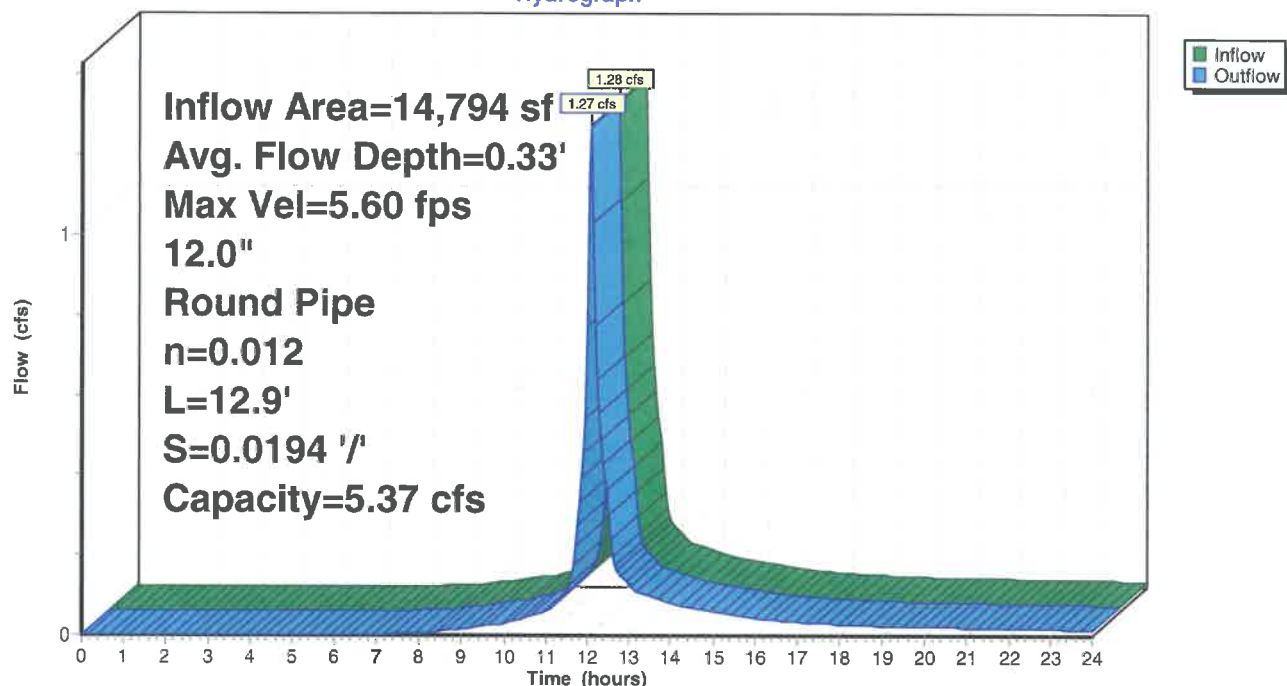
Peak Storage= 3 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.33' , Surface Width= 0.94'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 12.9' Slope= 0.0194 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 18R: CB 10 TO DMH 6

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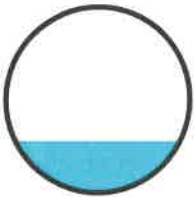
Summary for Reach 19R: CB 9 TO DMH 6

Inflow Area = 13,551 sf, 61.49% Impervious, Inflow Depth > 3.48" for 10 YR event
Inflow = 1.23 cfs @ 12.09 hrs, Volume= 3,928 cf
Outflow = 1.23 cfs @ 12.09 hrs, Volume= 3,928 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.90 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.37 fps, Avg. Travel Time= 0.0 min

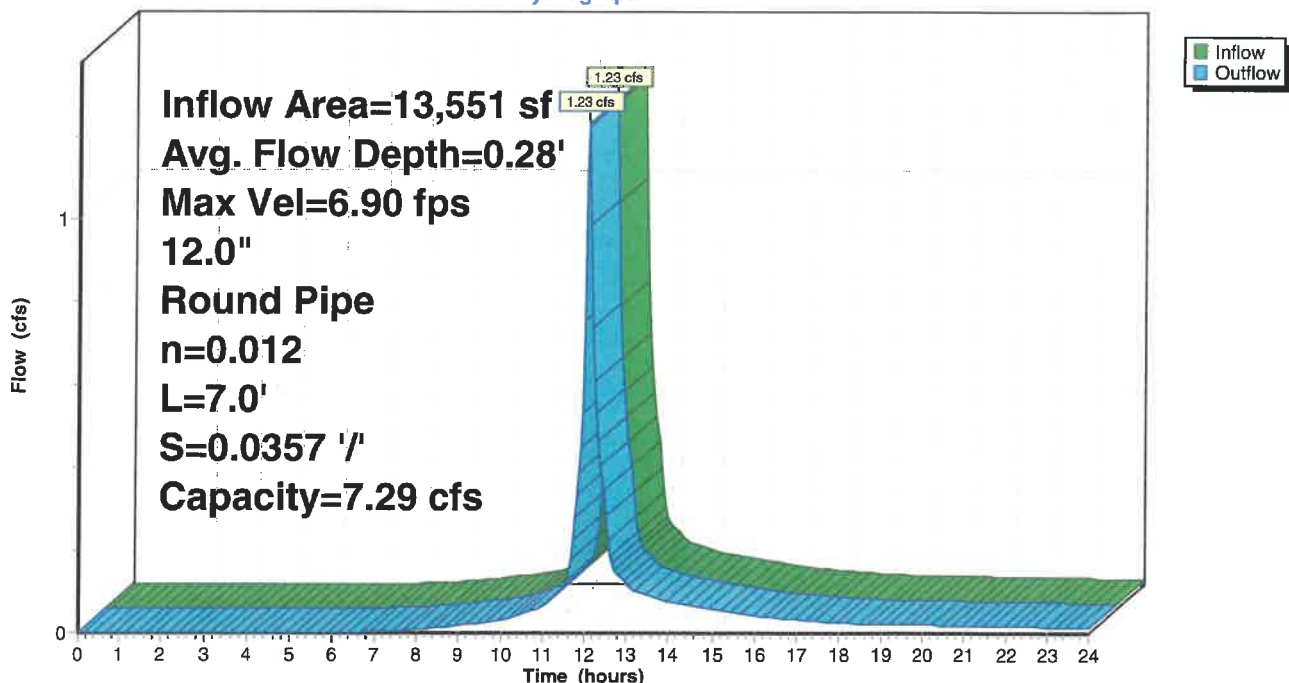
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.28', Surface Width= 0.90'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.29 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.0' Slope= 0.0357 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 19R: CB 9 TO DMH 6

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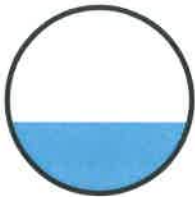
Summary for Reach 20R: DMH 6 to Basin

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 3.47" for 10 YR event
Inflow = 3.18 cfs @ 12.12 hrs, Volume= 10,620 cf
Outflow = 3.17 cfs @ 12.12 hrs, Volume= 10,620 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.34 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 2.48 fps, Avg. Travel Time= 0.2 min

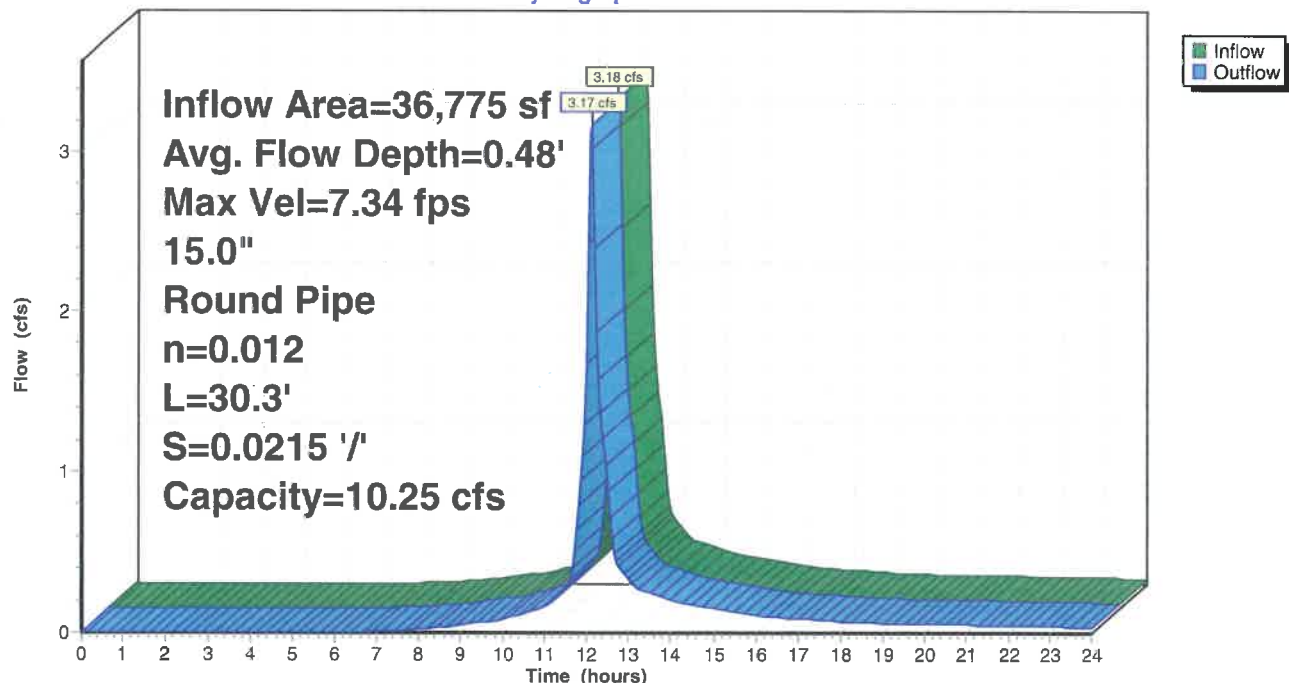
Peak Storage= 13 cf @ 12.12 hrs
Average Depth at Peak Storage= 0.48', Surface Width= 1.22'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 10.25 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 30.3' Slope= 0.0215 '/'
Inlet Invert= 305.65', Outlet Invert= 305.00'



Reach 20R: DMH 6 to Basin

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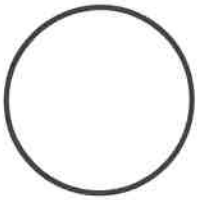
Summary for Reach 21R: 2-8" DI

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth = 0.00" for 10 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

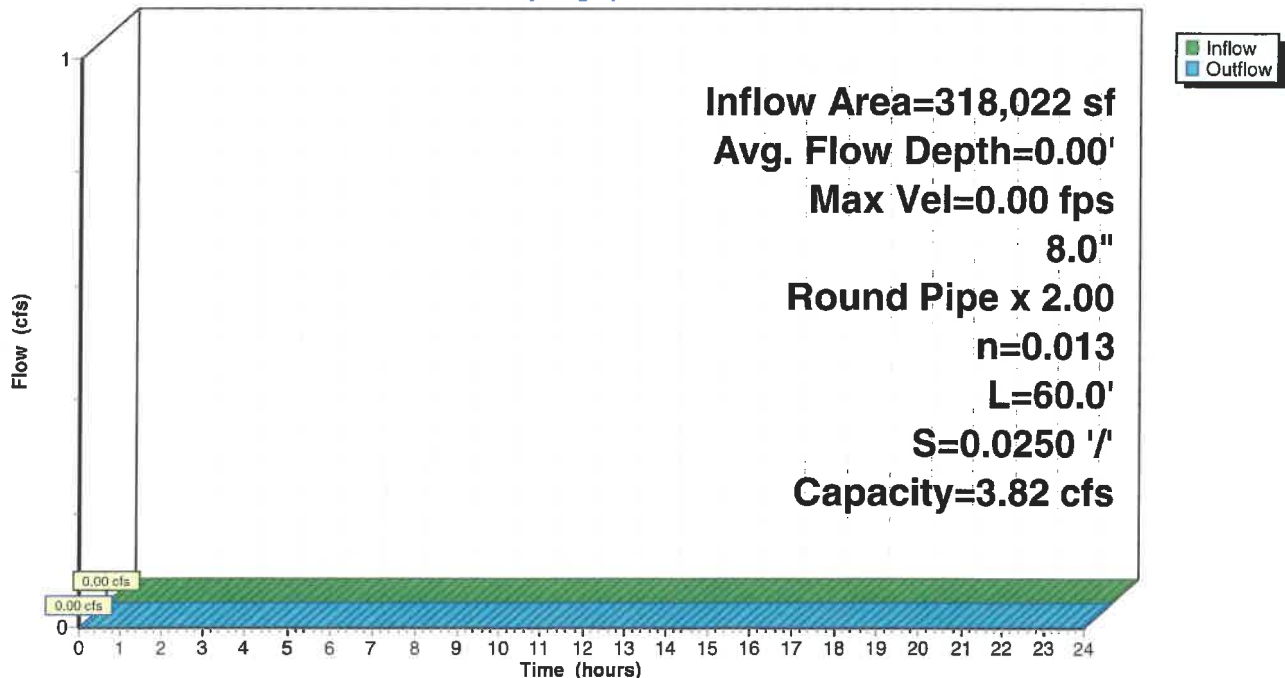
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.67' Flow Area= 0.7 sf, Capacity= 3.82 cfs

A factor of 2.00 has been applied to the storage and discharge capacity
8.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 60.0' Slope= 0.0250 '/'
Inlet Invert= 312.50', Outlet Invert= 311.00'



Reach 21R: 2-8" DI

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Summary for Reach 22R: Basin 2 Outfall

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth = 0.61" for 10 YR event
Inflow = 0.42 cfs @ 13.37 hrs, Volume= 5,900 cf
Outflow = 0.42 cfs @ 13.37 hrs, Volume= 5,900 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.97 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.48 fps, Avg. Travel Time= 0.4 min

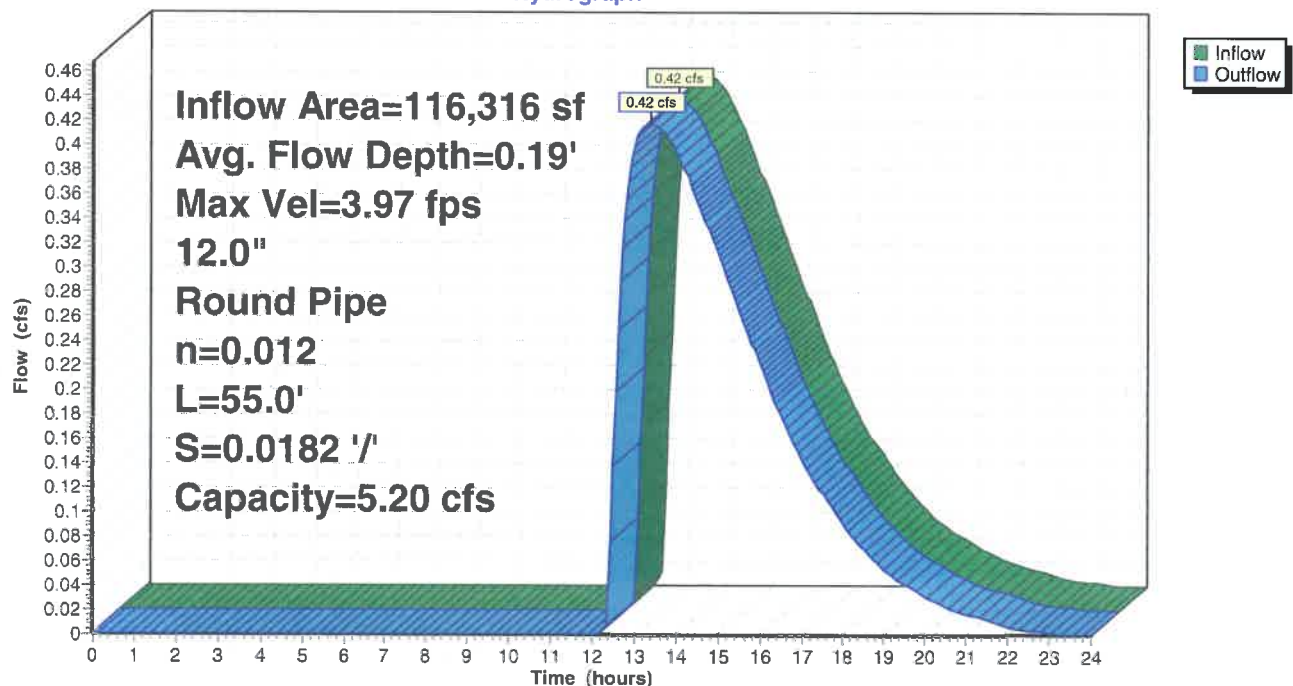
Peak Storage= 6 cf @ 13.37 hrs
Average Depth at Peak Storage= 0.19' , Surface Width= 0.79'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.20 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 55.0' Slope= 0.0182 '/'
Inlet Invert= 304.00', Outlet Invert= 303.00'



Reach 22R: Basin 2 Outfall

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Summary for Reach 23R: Basin 1 Outfall

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 0.42" for 10 YR event
Inflow = 0.22 cfs @ 14.50 hrs, Volume= 4,311 cf
Outflow = 0.22 cfs @ 14.51 hrs, Volume= 4,311 cf, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.40 fps, Min. Travel Time= 0.2 min
Avg. Velocity= 2.43 fps, Avg. Travel Time= 0.3 min

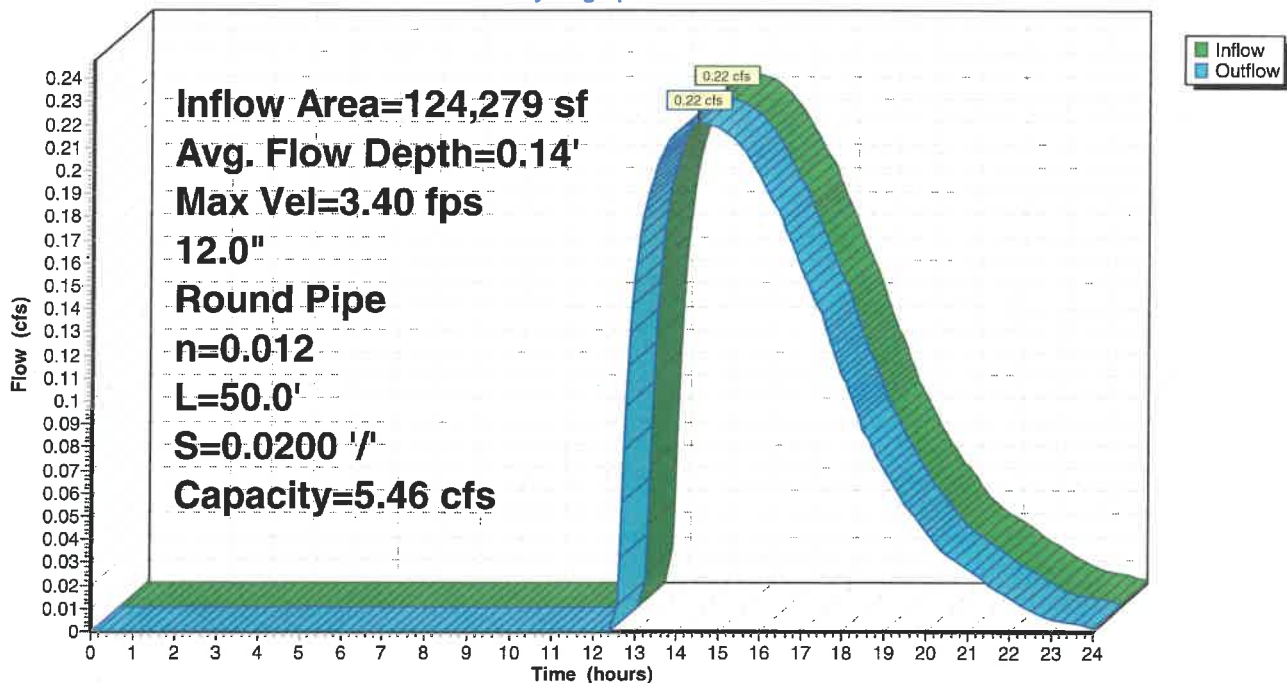
Peak Storage= 3 cf @ 14.50 hrs
Average Depth at Peak Storage= 0.14', Surface Width= 0.69'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.46 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 50.0' Slope= 0.0200 '/'
Inlet Invert= 281.00', Outlet Invert= 280.00'



Reach 23R: Basin 1 Outfall

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Summary for Reach 24R: Basin 3 Outfall

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 1.32" for 10 YR event
Inflow = 0.73 cfs @ 12.57 hrs, Volume= 6,126 cf
Outflow = 0.74 cfs @ 12.56 hrs, Volume= 6,125 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.00 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.73 fps, Avg. Travel Time= 0.3 min

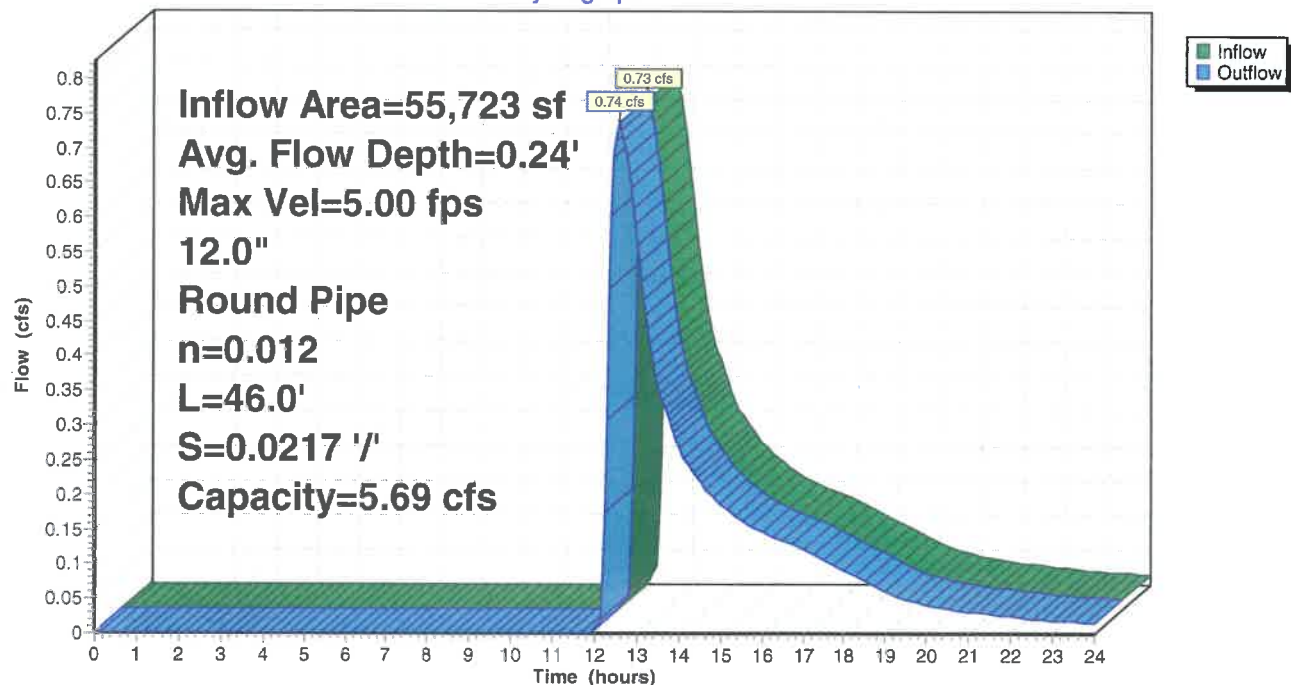
Peak Storage= 7 cf @ 12.56 hrs
Average Depth at Peak Storage= 0.24' , Surface Width= 0.86'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.69 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 46.0' Slope= 0.0217 '/'
Inlet Invert= 289.00', Outlet Invert= 288.00'



Reach 24R: Basin 3 Outfall

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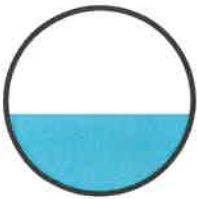
Summary for Reach 25R: 24" Bypass Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 1.50" for 10 YR event
Inflow = 10.77 cfs @ 12.43 hrs, Volume= 61,476 cf
Outflow = 10.76 cfs @ 12.44 hrs, Volume= 61,465 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.39 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 4.04 fps, Avg. Travel Time= 0.3 min

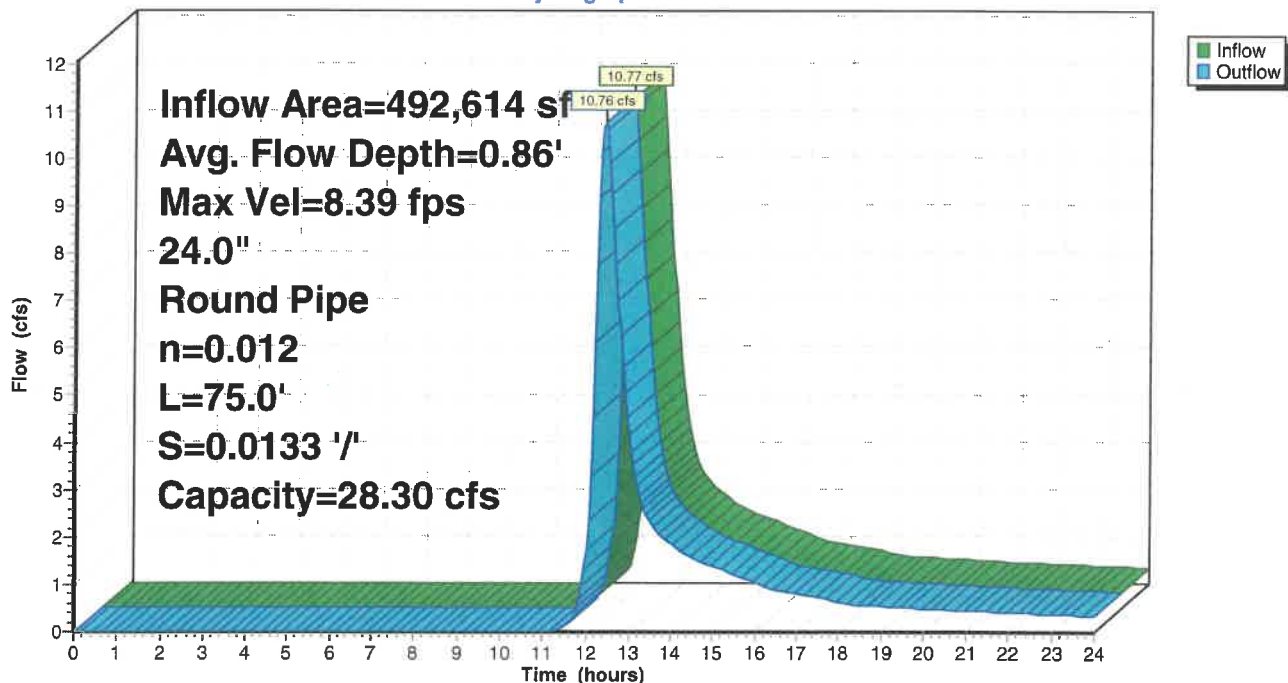
Peak Storage= 96 cf @ 12.43 hrs
Average Depth at Peak Storage= 0.86' , Surface Width= 1.98'
Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 28.30 cfs

24.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 75.0' Slope= 0.0133 '/'
Inlet Invert= 292.50', Outlet Invert= 291.50'



Reach 25R: 24" Bypass Culvert

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Summary for Reach EV1: Road Berm

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 1.15" for 10 YR event
Inflow = 25.27 cfs @ 13.17 hrs, Volume= 263,946 cf
Outflow = 25.27 cfs @ 13.17 hrs, Volume= 263,929 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.36 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.85 fps, Avg. Travel Time= 0.1 min

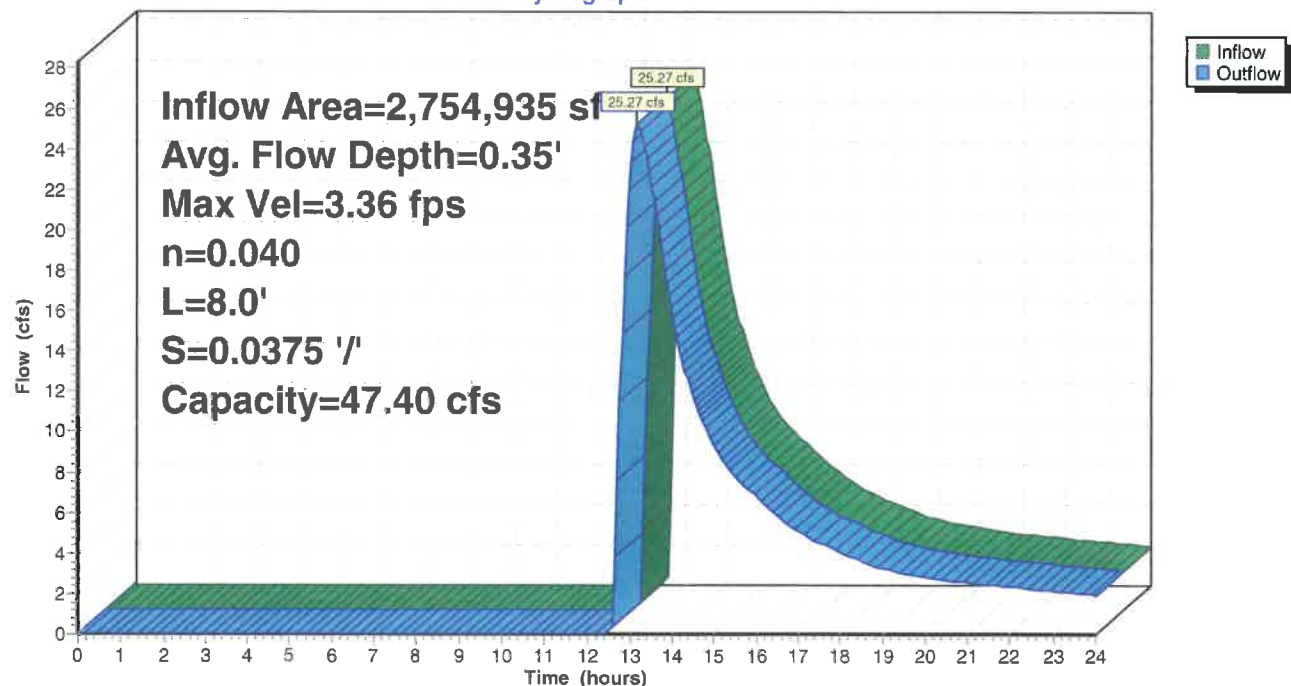
Peak Storage= 60 cf @ 13.17 hrs
Average Depth at Peak Storage= 0.35', Surface Width= 23.46'
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides
Side Slope Z-value= 5.0 '/' Top Width= 25.00'
Length= 8.0' Slope= 0.0375 '/'
Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: Road Berm

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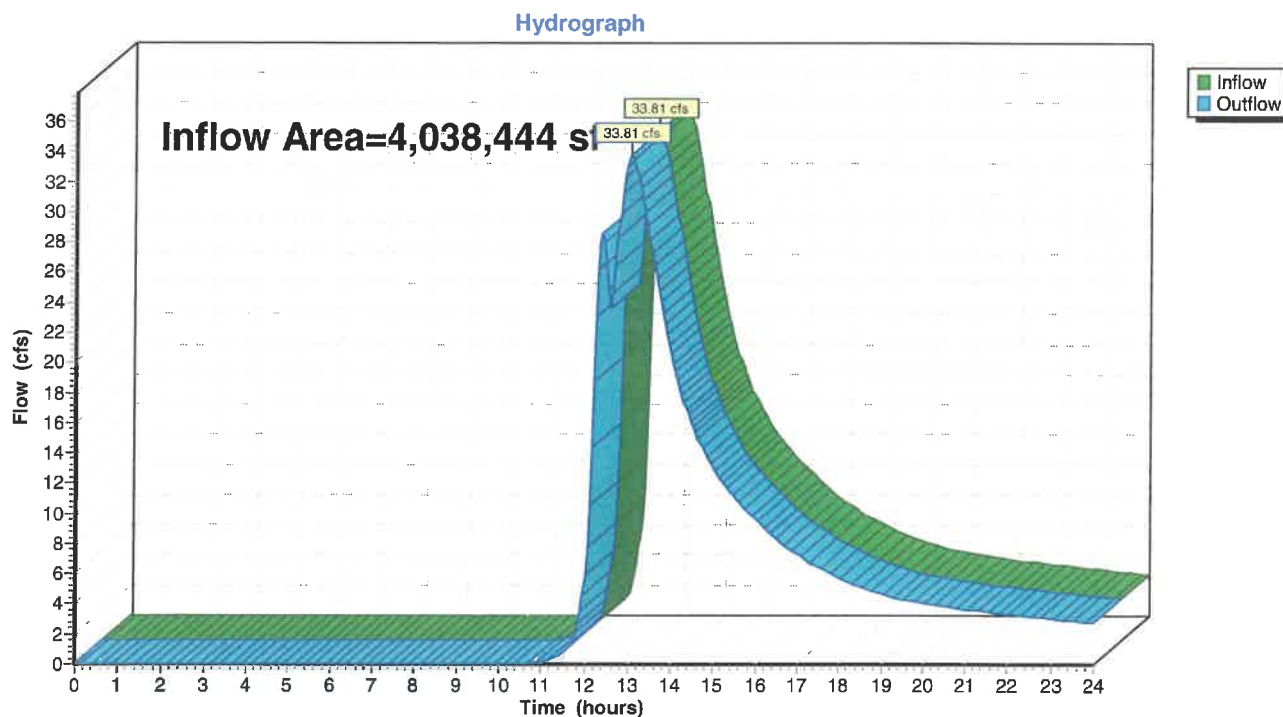
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Summary for Reach EV2: To Wetland

Inflow Area = 4,038,444 sf, 4.14% Impervious, Inflow Depth > 1.27" for 10 YR event
Inflow = 33.81 cfs @ 13.07 hrs, Volume= 427,304 cf
Outflow = 33.81 cfs @ 13.07 hrs, Volume= 427,304 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV2: To Wetland



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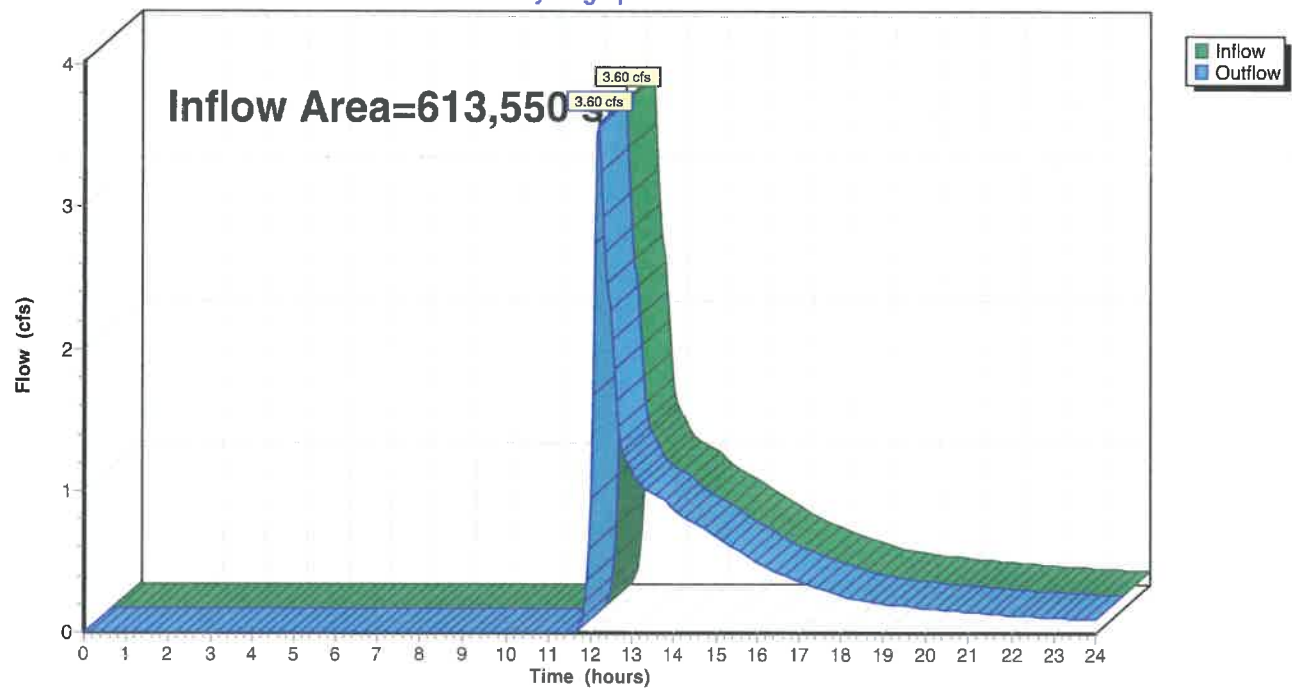
Summary for Reach EV3: To Offsite

Inflow Area = 613,550 sf, 3.69% Impervious, Inflow Depth > 0.43" for 10 YR event
Inflow = 3.60 cfs @ 12.18 hrs, Volume= 22,228 cf
Outflow = 3.60 cfs @ 12.18 hrs, Volume= 22,228 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV3: To Offsite

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Summary for Pond 1P: Forebay

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 4.01" for 10 YR event
 Inflow = 1.21 cfs @ 12.14 hrs, Volume= 4,300 cf
 Outflow = 1.23 cfs @ 12.17 hrs, Volume= 2,804 cf, Atten= 0%, Lag= 2.1 min
 Primary = 1.23 cfs @ 12.17 hrs, Volume= 2,804 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 282.59' @ 12.17 hrs Surf.Area= 1,292 sf Storage= 1,593 cf

Plug-Flow detention time= 169.9 min calculated for 2,798 cf (65% of inflow)
 Center-of-Mass det. time= 73.8 min (868.1 - 794.3)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 281.00' | 3,838 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 281.00 | 737 | 101.0 | 0 | 0 | 737 |
| 282.00 | 1,071 | 120.0 | 899 | 899 | 1,089 |
| 284.00 | 1,908 | 158.0 | 2,939 | 3,838 | 1,974 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 282.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=1.09 cfs @ 12.17 hrs HW=282.58' (Free Discharge)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 1.09 cfs @ 0.68 fps)

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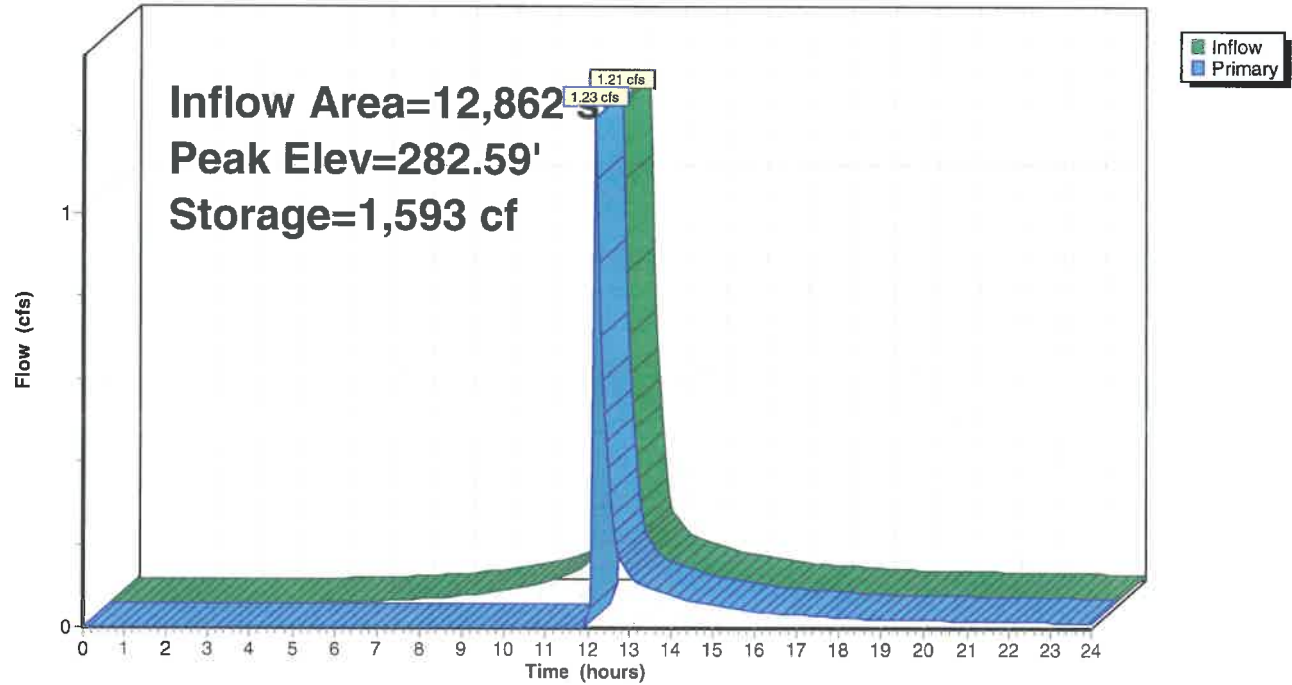
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Pond 1P: Forebay

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Summary for Pond 2P: Basin 1

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 1.31" for 10 YR event
 Inflow = 3.39 cfs @ 12.20 hrs, Volume= 13,556 cf
 Outflow = 0.34 cfs @ 14.50 hrs, Volume= 9,438 cf, Atten= 90%, Lag= 138.3 min
 Discarded = 0.12 cfs @ 14.50 hrs, Volume= 5,127 cf
 Primary = 0.22 cfs @ 14.50 hrs, Volume= 4,311 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 282.44' @ 14.50 hrs Surf.Area= 4,771 sf Storage= 6,111 cf

Plug-Flow detention time= 243.9 min calculated for 9,418 cf (69% of inflow)
 Center-of-Mass det. time= 137.7 min (1,022.7 - 885.0)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 281.00' | 51,257 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 281.00 | 3,708 | 232.0 | 0 | 0 | 3,708 |
| 282.00 | 4,433 | 251.0 | 4,065 | 4,065 | 4,477 |
| 284.00 | 6,052 | 288.0 | 10,443 | 14,508 | 6,153 |
| 286.00 | 7,890 | 326.0 | 13,901 | 28,410 | 8,109 |
| 288.00 | 9,968 | 364.0 | 17,818 | 46,227 | 10,306 |
| 288.50 | 10,153 | 374.0 | 5,030 | 51,257 | 10,922 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 287.50' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 281.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 281.00' | 12.0" Round Culvert L= 44.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 281.00' / 280.00' S= 0.0227 ' S= 0.0227 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 282.00' | 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 282.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 283.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 286.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.12 cfs @ 14.50 hrs HW=282.44' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=0.22 cfs @ 14.50 hrs HW=282.44' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 0.22 cfs of 3.24 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.22 cfs @ 2.54 fps)

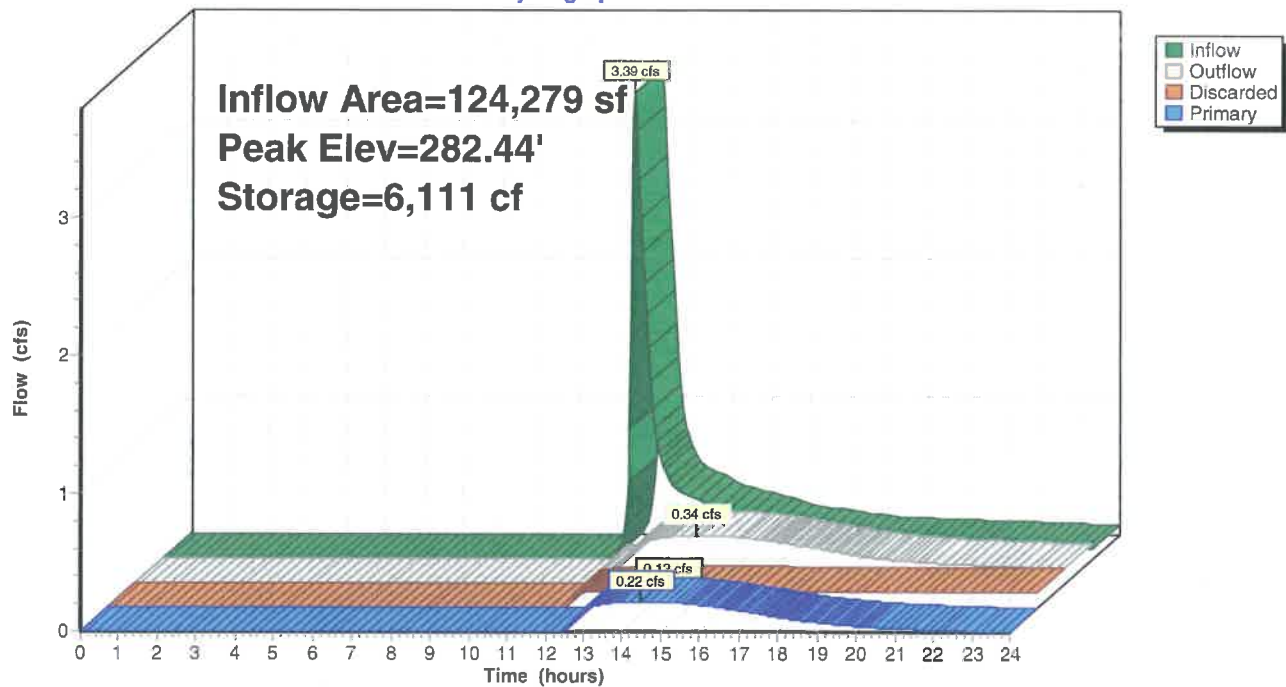
↳ **5=Orifice/Grate** (Controls 0.00 cfs)

↳ **6=Orifice/Grate** (Controls 0.00 cfs)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 2P: Basin 1

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Summary for Pond 3P: Forebay

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 3.47" for 10 YR event
 Inflow = 3.17 cfs @ 12.12 hrs, Volume= 10,620 cf
 Outflow = 3.10 cfs @ 12.14 hrs, Volume= 8,408 cf, Atten= 2%, Lag= 1.4 min
 Primary = 3.10 cfs @ 12.14 hrs, Volume= 8,408 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.66' @ 12.14 hrs Surf.Area= 1,879 sf Storage= 2,498 cf

Plug-Flow detention time= 119.1 min calculated for 8,390 cf (79% of inflow)
 Center-of-Mass det. time= 42.7 min (853.3 - 810.6)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 10,058 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 1,160 | 129.0 | 0 | 0 | 1,160 |
| 306.00 | 2,048 | 166.0 | 3,166 | 3,166 | 2,077 |
| 308.00 | 3,164 | 204.0 | 5,172 | 8,338 | 3,256 |
| 308.50 | 3,725 | 227.0 | 1,720 | 10,058 | 4,052 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 305.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=3.04 cfs @ 12.14 hrs HW=305.66' (Free Discharge)

↑ **1=Broad-Crested Rectangular Weir** (Weir Controls 3.04 cfs @ 0.97 fps)

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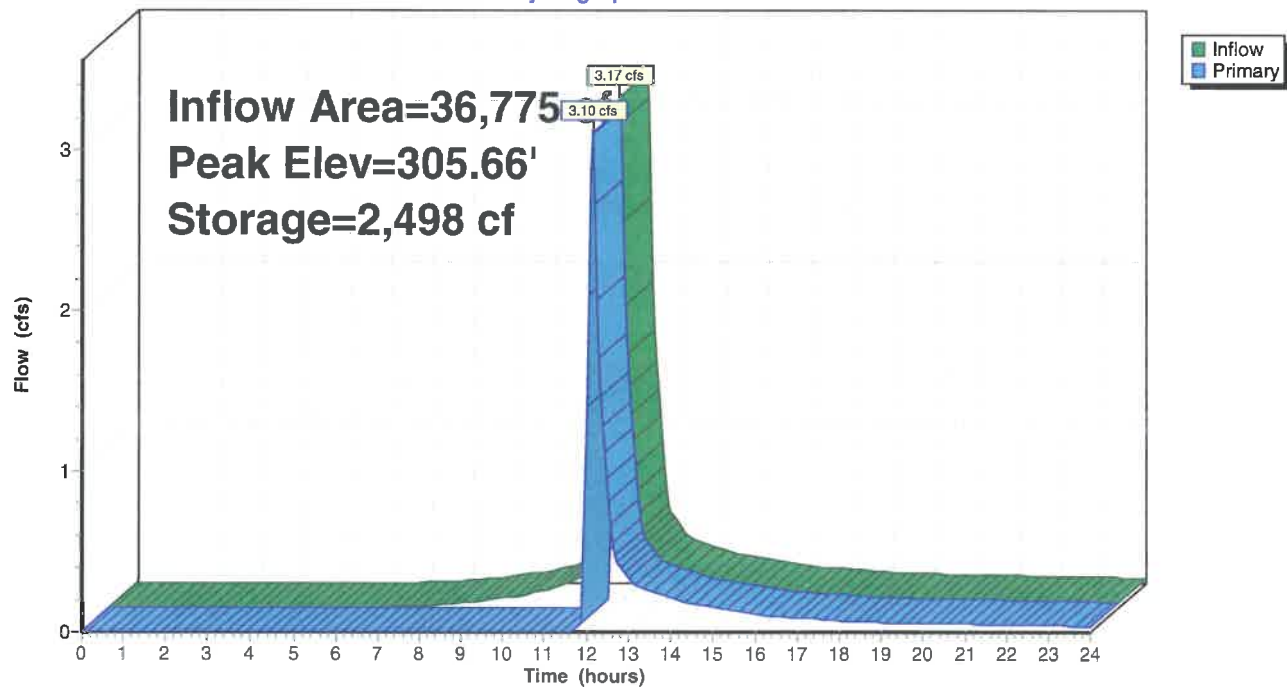
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Pond 3P: Forebay

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Summary for Pond 4P: Basin 2

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 1.75" for 10 YR event
 Inflow = 4.99 cfs @ 12.16 hrs, Volume= 16,980 cf
 Outflow = 0.57 cfs @ 13.37 hrs, Volume= 12,128 cf, Atten= 89%, Lag= 72.7 min
 Discarded = 0.15 cfs @ 13.37 hrs, Volume= 6,229 cf
 Primary = 0.42 cfs @ 13.37 hrs, Volume= 5,900 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.44' @ 13.37 hrs Surf.Area= 5,862 sf Storage= 7,458 cf

Plug-Flow detention time= 223.0 min calculated for 12,103 cf (71% of inflow)
 Center-of-Mass det. time= 124.0 min (991.8 - 867.8)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 30,313 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 4,504 | 302.0 | 0 | 0 | 4,504 |
| 306.00 | 6,434 | 340.0 | 10,881 | 10,881 | 6,549 |
| 308.00 | 8,590 | 378.0 | 14,972 | 25,853 | 8,836 |
| 308.50 | 9,256 | 397.0 | 4,460 | 30,313 | 10,023 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 307.70' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 304.00' | 1.100 in/hr Exfiltration over Surface area |
| #3 | Primary | 304.00' | 12.0" Round Culvert L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 304.00' / 303.00' S= 0.0286 ' S= 0.0286 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 305.00' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 305.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 306.00' | 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

Discarded OutFlow Max=0.15 cfs @ 13.37 hrs HW=305.44' (Free Discharge)

2=Exfiltration (Exfiltration Controls 0.15 cfs)
Primary OutFlow Max=0.42 cfs @ 13.37 hrs HW=305.44' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
3=Culvert (Passes 0.42 cfs of 3.24 cfs potential flow)
4=Orifice/Grate (Orifice Controls 0.42 cfs @ 2.27 fps)
5=Orifice/Grate (Controls 0.00 cfs)
6=Orifice/Grate (Controls 0.00 cfs)

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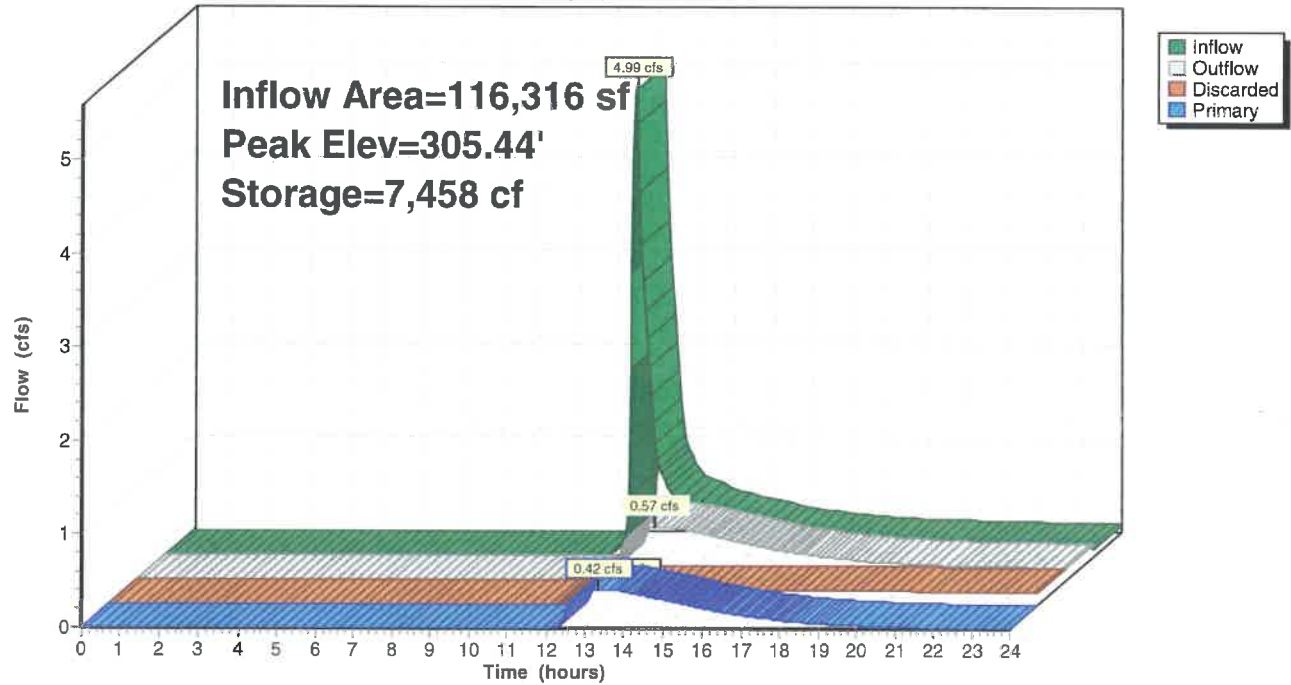
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Pond 4P: Basin 2

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Summary for Pond 5P: Forebay

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 4.52" for 10 YR event
 Inflow = 2.66 cfs @ 12.09 hrs, Volume= 9,190 cf
 Outflow = 2.24 cfs @ 12.17 hrs, Volume= 5,335 cf, Atten= 16%, Lag= 4.4 min
 Primary = 2.24 cfs @ 12.17 hrs, Volume= 5,335 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 291.33' @ 12.17 hrs Surf.Area= 2,318 sf Storage= 4,142 cf

Plug-Flow detention time= 205.4 min calculated for 5,335 cf (58% of inflow)
 Center-of-Mass det. time= 98.8 min (870.6 - 771.8)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 289.00' | 5,821 cf | Forebay (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 289.00 | 1,270 | 132.0 | 0 | 0 | 1,270 |
| 290.00 | 1,710 | 153.0 | 1,485 | 1,485 | 1,767 |
| 291.00 | 2,145 | 172.0 | 1,923 | 3,408 | 2,285 |
| 292.00 | 2,692 | 191.0 | 2,413 | 5,821 | 2,863 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 291.20' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=2.04 cfs @ 12.17 hrs HW=291.32' (Free Discharge)

↑ **1=Broad-Crested Rectangular Weir** (Weir Controls 2.04 cfs @ 0.84 fps)

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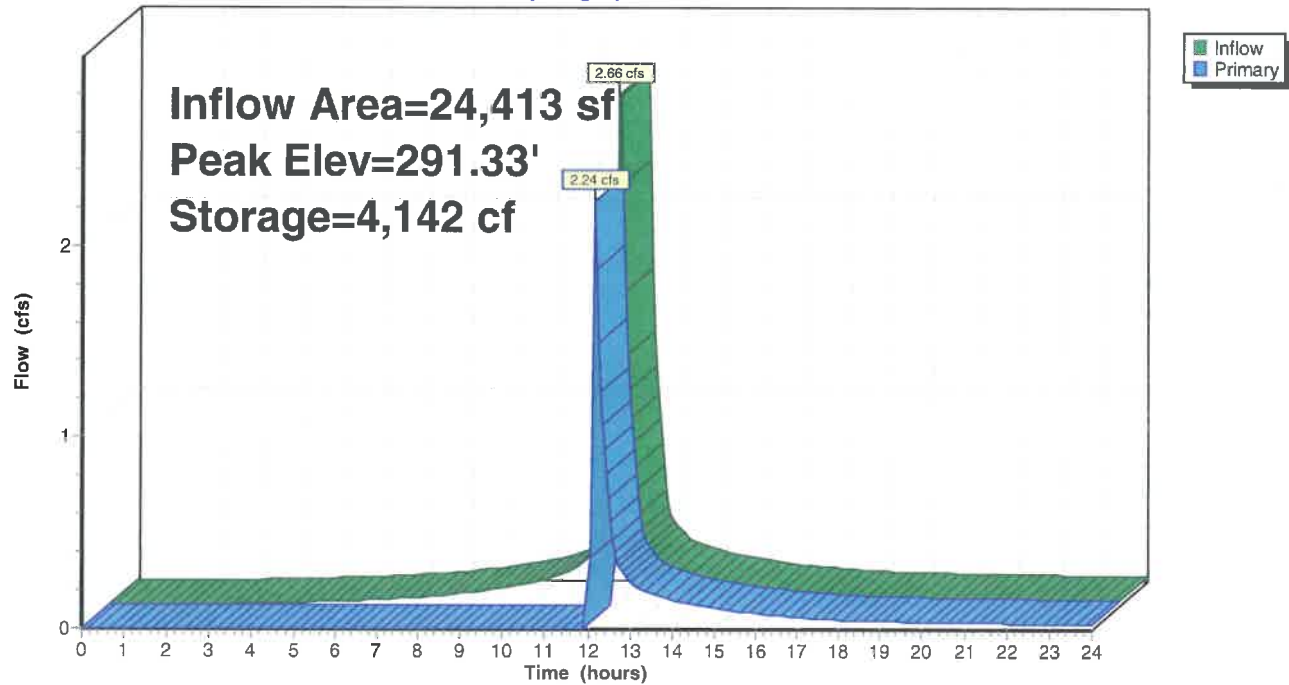
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Pond 5P: Forebay

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Summary for Pond 6P: Basin 3

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 2.00" for 10 YR event
 Inflow = 3.18 cfs @ 12.16 hrs, Volume= 9,268 cf
 Outflow = 0.79 cfs @ 12.57 hrs, Volume= 8,034 cf, Atten= 75%, Lag= 24.6 min
 Discarded = 0.06 cfs @ 12.57 hrs, Volume= 1,908 cf
 Primary = 0.73 cfs @ 12.57 hrs, Volume= 6,126 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 291.04' @ 12.57 hrs Surf.Area= 2,090 sf Storage= 2,932 cf

Plug-Flow detention time= 123.6 min calculated for 8,034 cf (87% of inflow)
 Center-of-Mass det. time= 63.6 min (933.3 - 869.7)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 289.00' | 12,306 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 289.00 | 840 | 185.0 | 0 | 0 | 840 |
| 290.00 | 1,423 | 204.0 | 1,119 | 1,119 | 1,459 |
| 291.00 | 2,065 | 223.0 | 1,734 | 2,853 | 2,139 |
| 292.00 | 2,763 | 242.0 | 2,406 | 5,258 | 2,880 |
| 293.00 | 3,517 | 260.0 | 3,132 | 8,391 | 3,642 |
| 294.00 | 4,328 | 279.0 | 3,915 | 12,306 | 4,500 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 293.25' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 289.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 289.00' | 12.0" Round Culvert L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 289.00' / 288.00' S= 0.0222 ' S= 0.0222 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 290.00' | 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 290.50' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 291.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 292.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.06 cfs @ 12.57 hrs HW=291.04' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.73 cfs @ 12.57 hrs HW=291.04' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 0.73 cfs of 4.14 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.23 cfs @ 4.60 fps)

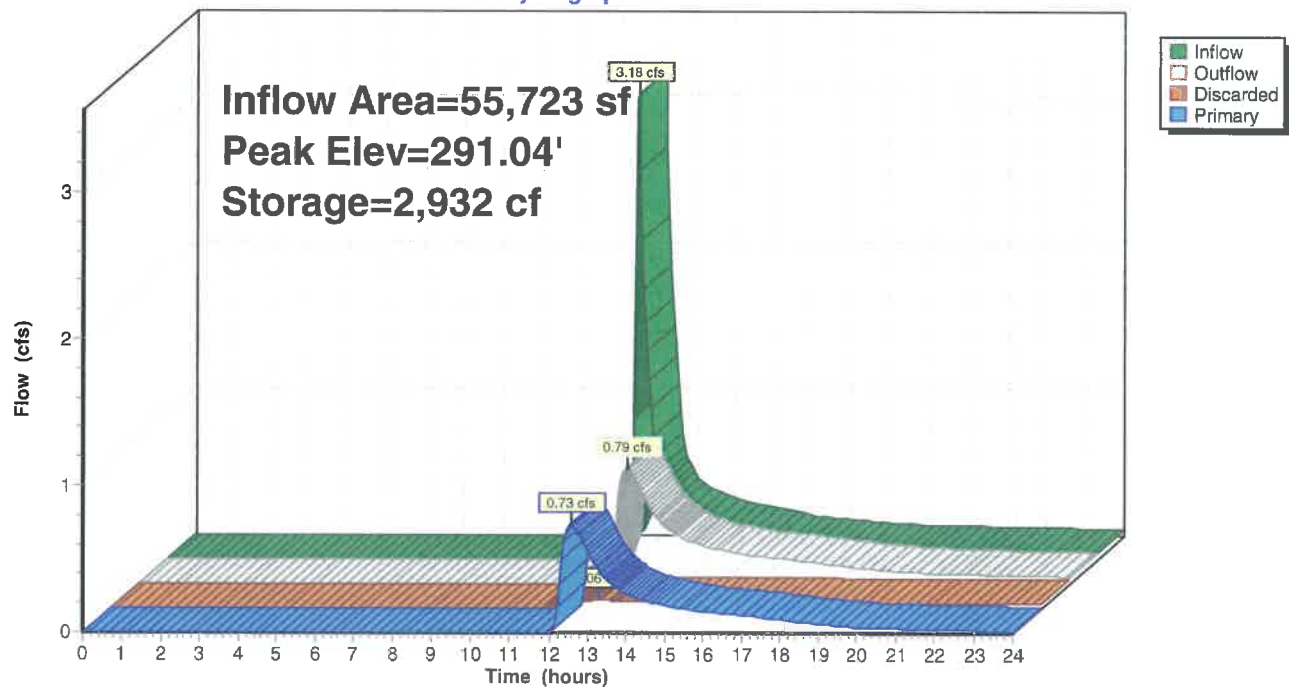
↳ **5=Orifice/Grate** (Orifice Controls 0.51 cfs @ 2.58 fps)

↳ **6=Orifice/Grate** (Controls 0.00 cfs)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 6P: Basin 3

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Summary for Pond 7P: Vernal Pool and Wetland

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 1.35" for 10 YR event
 Inflow = 41.58 cfs @ 12.70 hrs, Volume= 309,492 cf
 Outflow = 25.27 cfs @ 13.17 hrs, Volume= 263,946 cf, Atten= 39%, Lag= 28.3 min
 Primary = 25.27 cfs @ 13.17 hrs, Volume= 263,946 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 294.91' @ 13.17 hrs Surf.Area= 45,996 sf Storage= 81,001 cf

Plug-Flow detention time= 110.1 min calculated for 263,946 cf (85% of inflow)
 Center-of-Mass det. time= 47.5 min (951.8 - 904.3)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 291.30' | 143,915 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=25.23 cfs @ 13.17 hrs HW=294.91' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 25.23 cfs @ 2.83 fps)

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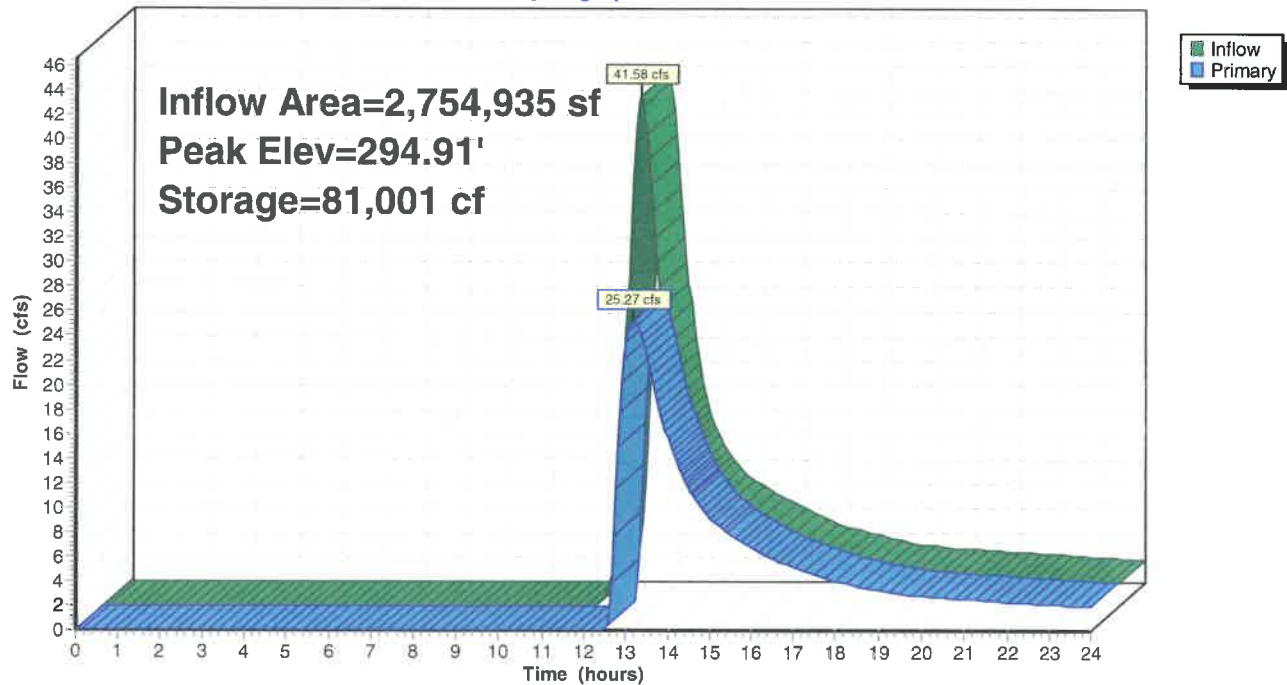
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Pond 7P: Vernal Pool and Wetland

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Summary for Pond 8P: Road Berm

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth > 1.09" for 10 YR event
 Inflow = 5.01 cfs @ 12.37 hrs, Volume= 28,868 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 311.10' @ 24.00 hrs Surf.Area= 41,347 sf Storage= 28,847 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 310.00' | 256,496 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 312.50' | 8.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)

1=Orifice/Grate (Controls 0.00 cfs)

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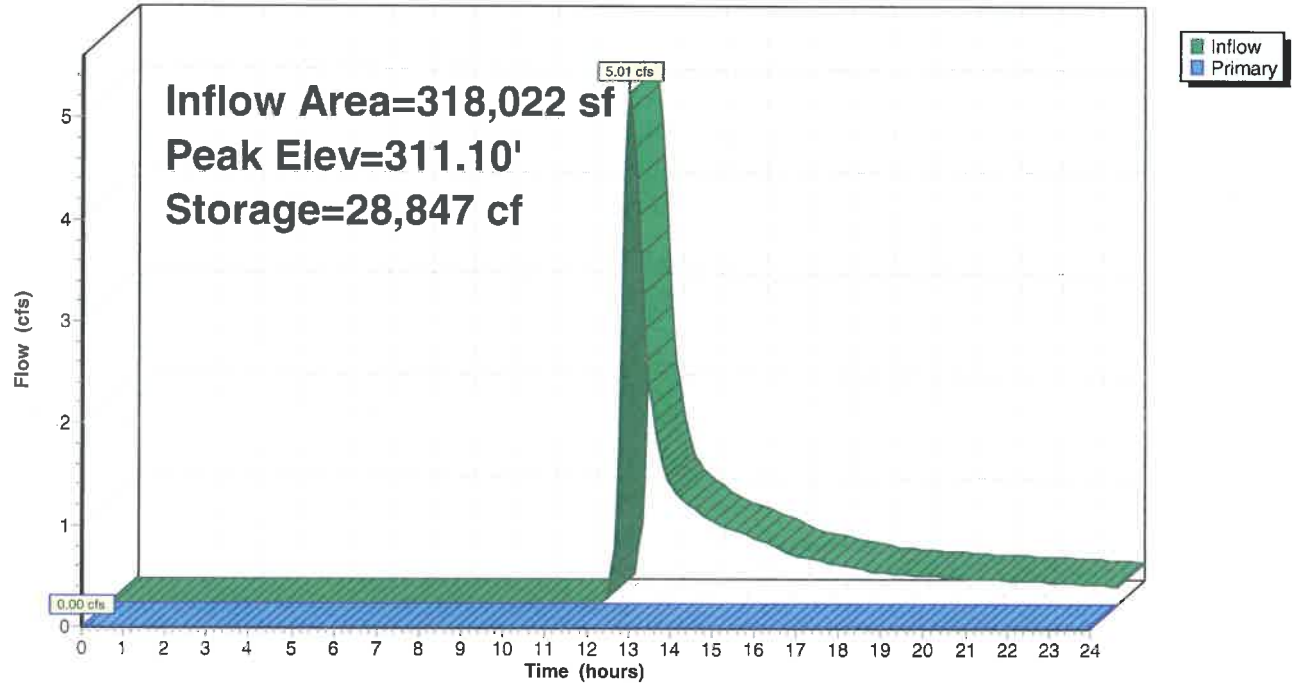
00454 - PROPOSED Conditions
Type III 24-hr 10 YR Rainfall=5.23"

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Pond 8P: Road Berm

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 1S: To Culvert

Runoff = 16.88 cfs @ 12.41 hrs, Volume= 92,300 cf, Depth> 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

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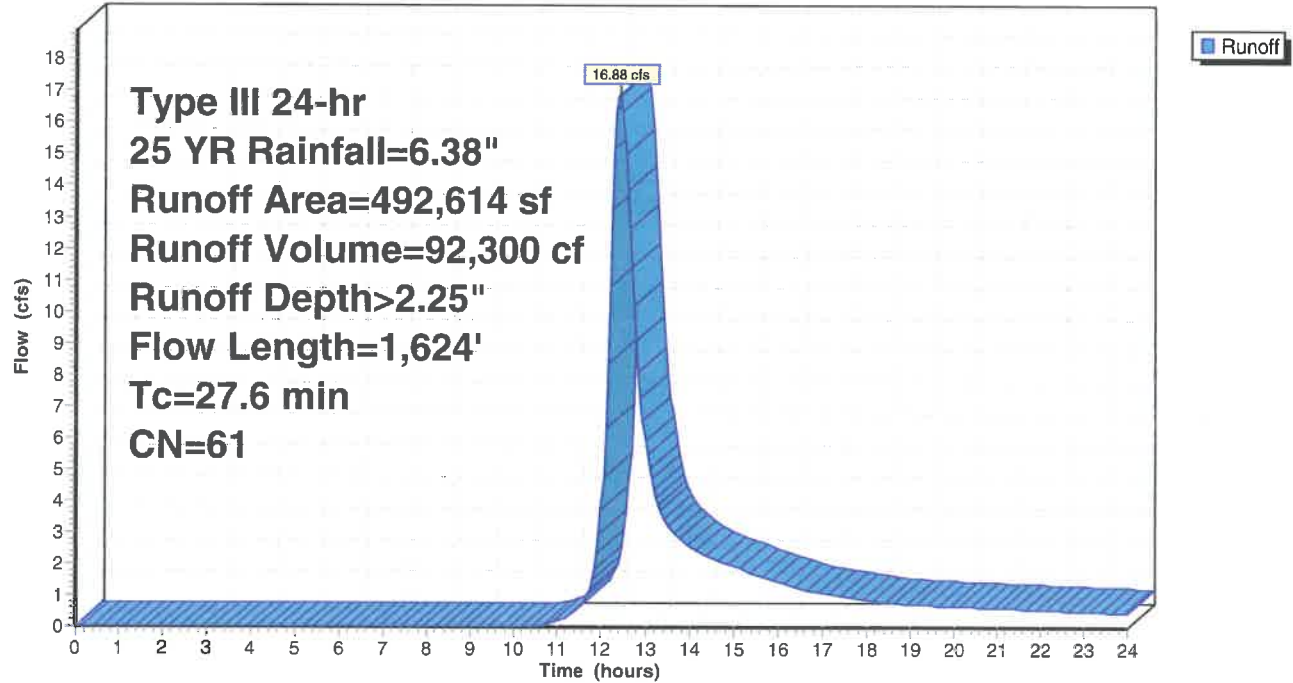
00454 - PROPOSED Conditions
Type III 24-hr 25 YR Rainfall=6.38"

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Subcatchment 1S: To Culvert

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 2S: To Proposed Culvert

Runoff = 66.94 cfs @ 12.68 hrs, Volume= 472,904 cf, Depth> 2.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 45.8 | 2,588 | Total | | | |

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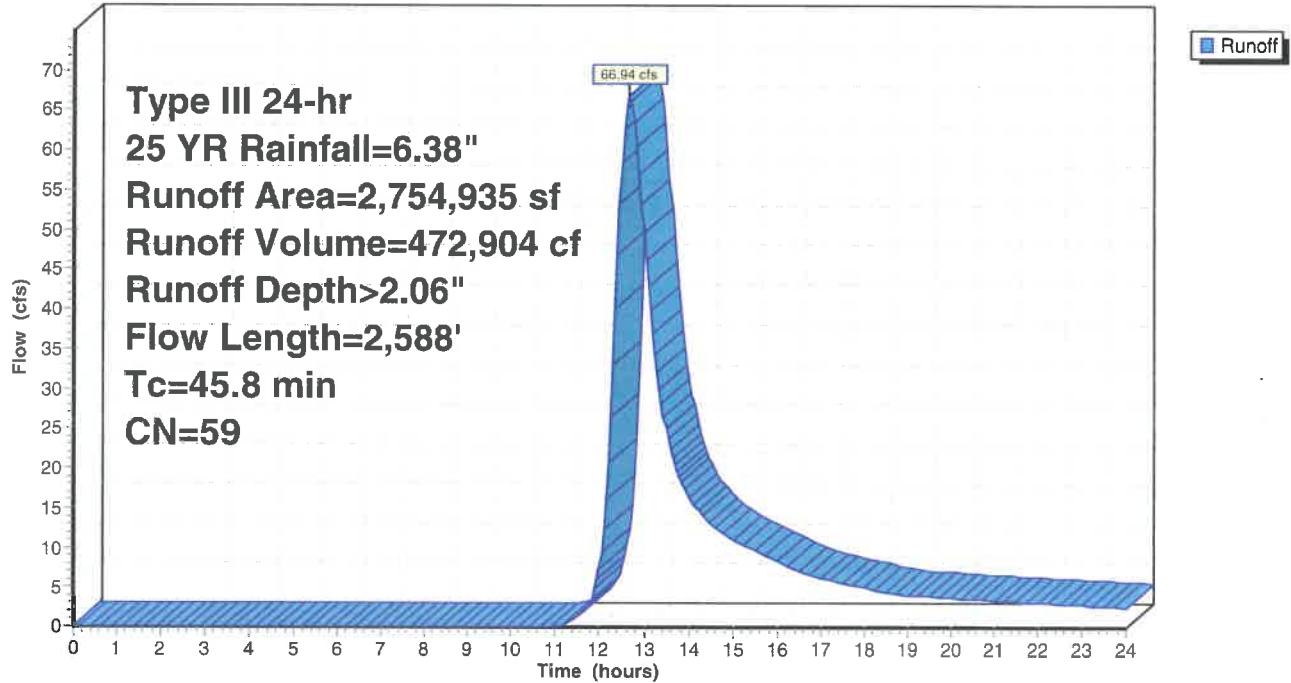
00454 - PROPOSED Conditions
Type III 24-hr 25 YR Rainfall=6.38"

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Subcatchment 2S: To Proposed Culvert

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Summary for Subcatchment 3S: To 8" pipes

Runoff = 8.70 cfs @ 12.34 hrs, Volume= 45,859 cf, Depth> 1.73"

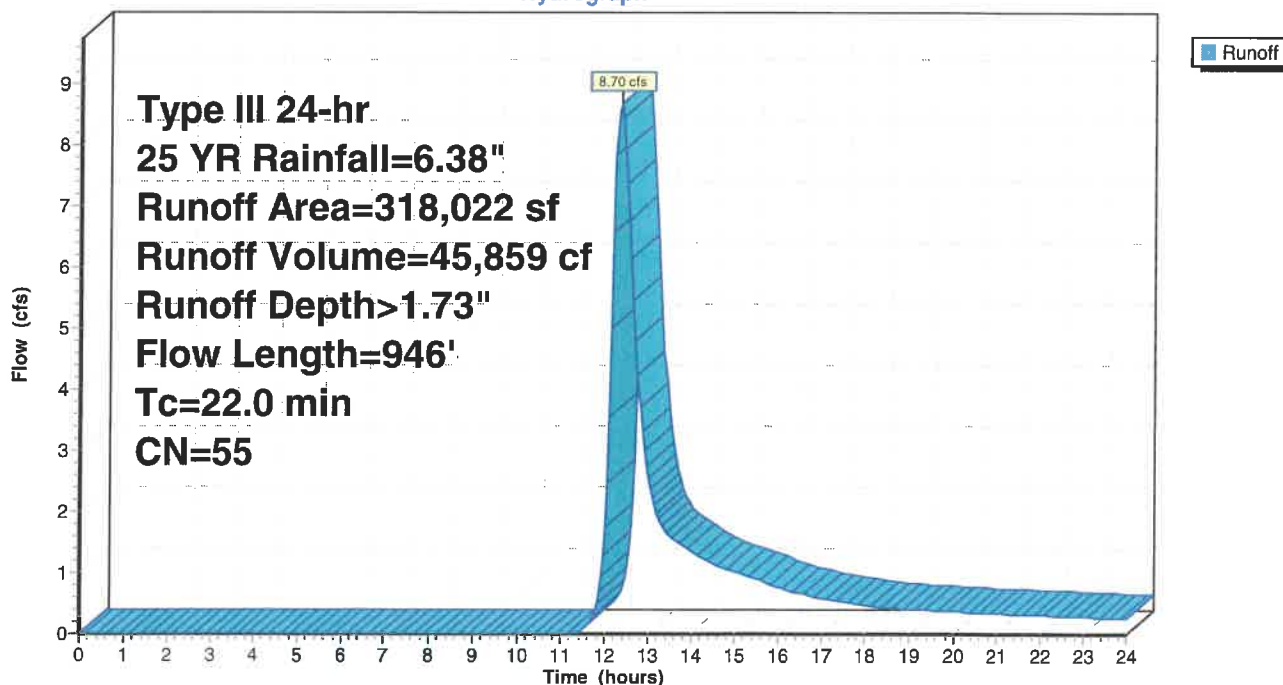
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 318,022 | 55 | Woods, Good, HSG B |
| 318,022 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.22" |
| 14.9 | 896 | 0.0401 | 1.00 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 22.0 | 946 | Total | | | |

Subcatchment 3S: To 8" pipes

Hydrograph



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Summary for Subcatchment 4S: To CB 1

Runoff = 1.75 cfs @ 12.09 hrs, Volume= 5,936 cf, Depth> 5.44"

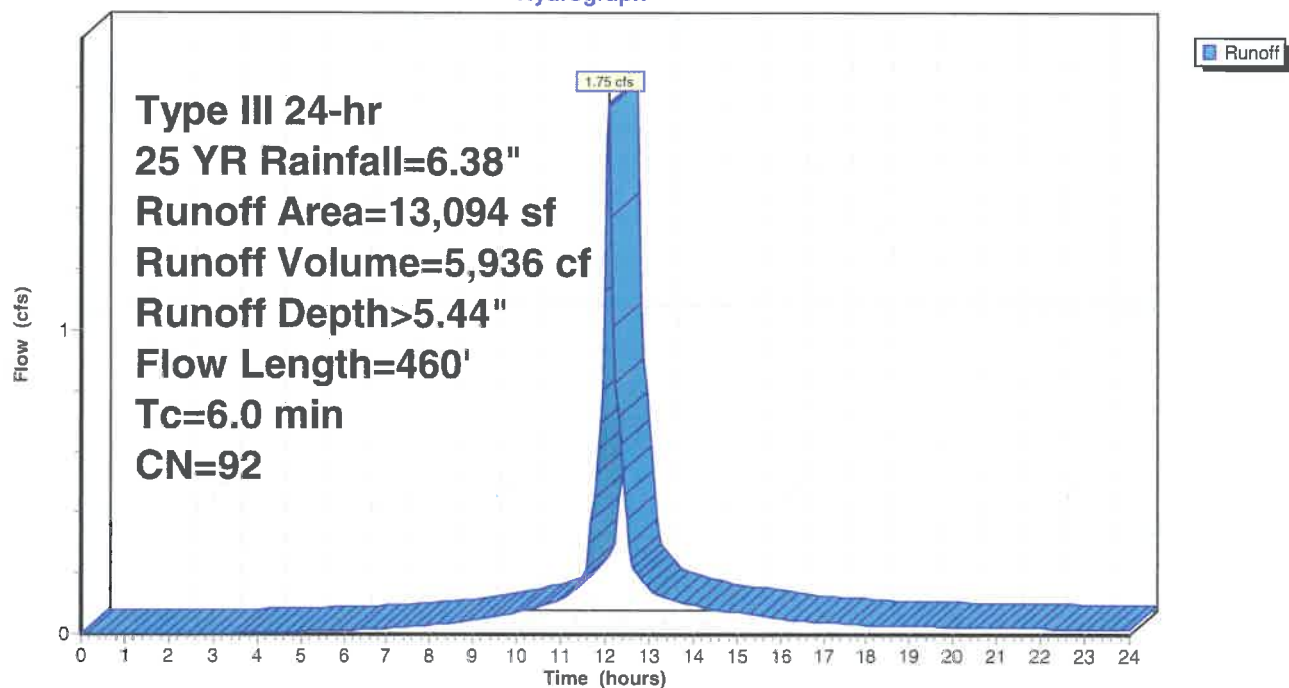
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 11,086 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,008 | 61 | >75% Grass cover, Good, HSG B |
| 13,094 | 92 | Weighted Average |
| 2,008 | | 15.34% Pervious Area |
| 11,086 | | 84.66% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 4S: To CB 1

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 5S: To CB 2

Runoff = 1.57 cfs @ 12.09 hrs, Volume= 5,567 cf, Depth> 5.90"

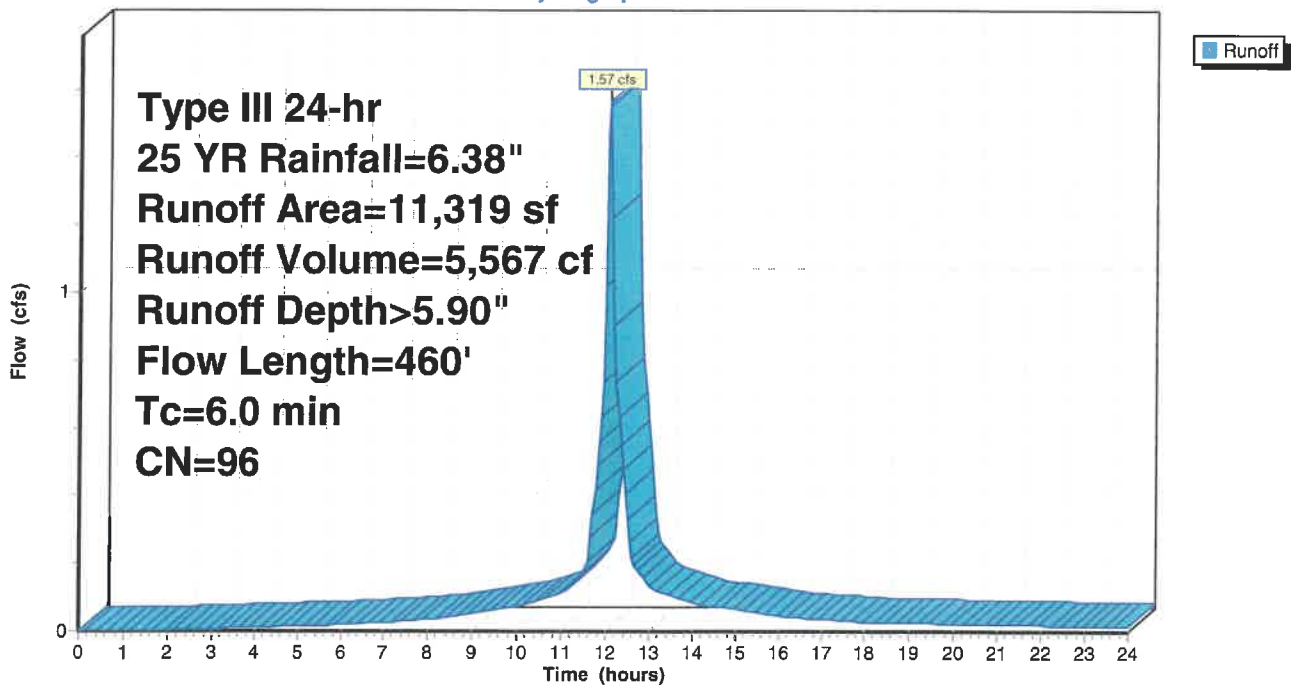
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 10,719 | 98 | Paved roads w/curbs & sewers, HSG B |
| 600 | 61 | >75% Grass cover, Good, HSG B |
| 11,319 | 96 | Weighted Average |
| 600 | | 5.30% Pervious Area |
| 10,719 | | 94.70% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 5S: To CB 2

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Summary for Subcatchment 6S: To CB 3

Runoff = 0.40 cfs @ 12.09 hrs, Volume= 1,328 cf, Depth> 5.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

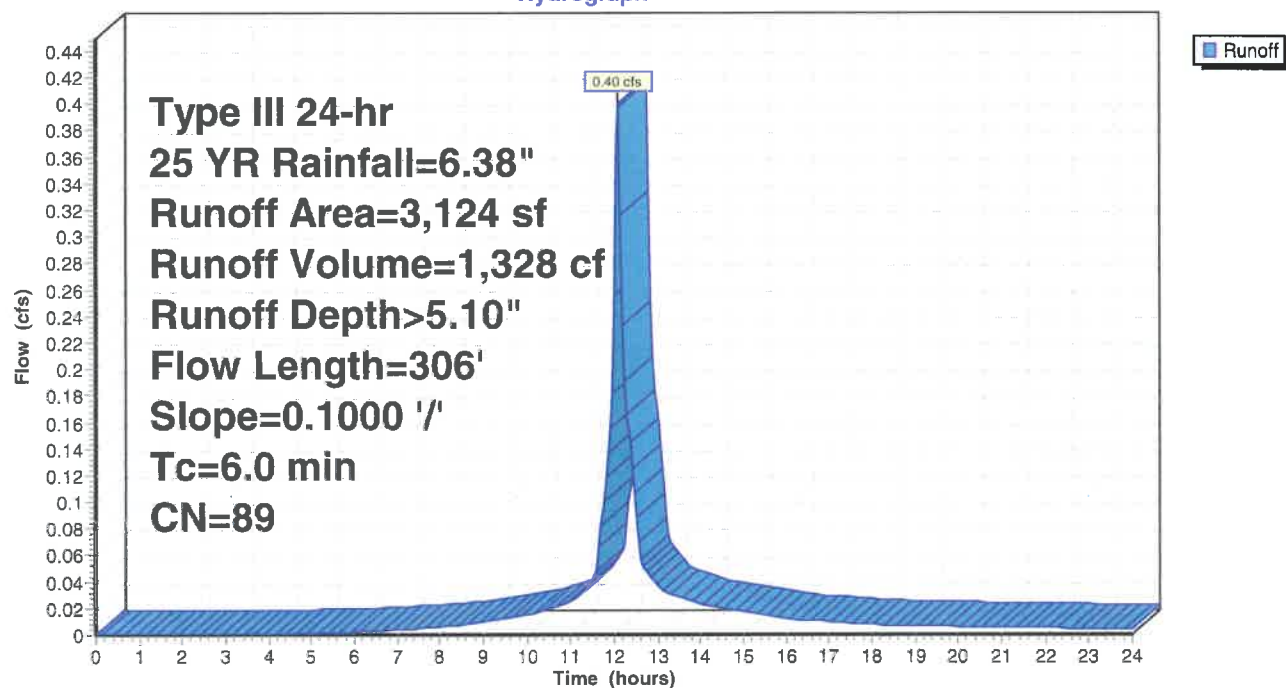
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,343 | 98 | Paved roads w/curbs & sewers, HSG B |
| 781 | 61 | >75% Grass cover, Good, HSG B |
| 3,124 | 89 | Weighted Average |
| 781 | | 25.00% Pervious Area |
| 2,343 | | 75.00% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 6S: To CB 3

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Summary for Subcatchment 7S: To CB 4

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,084 cf, Depth> 5.10"

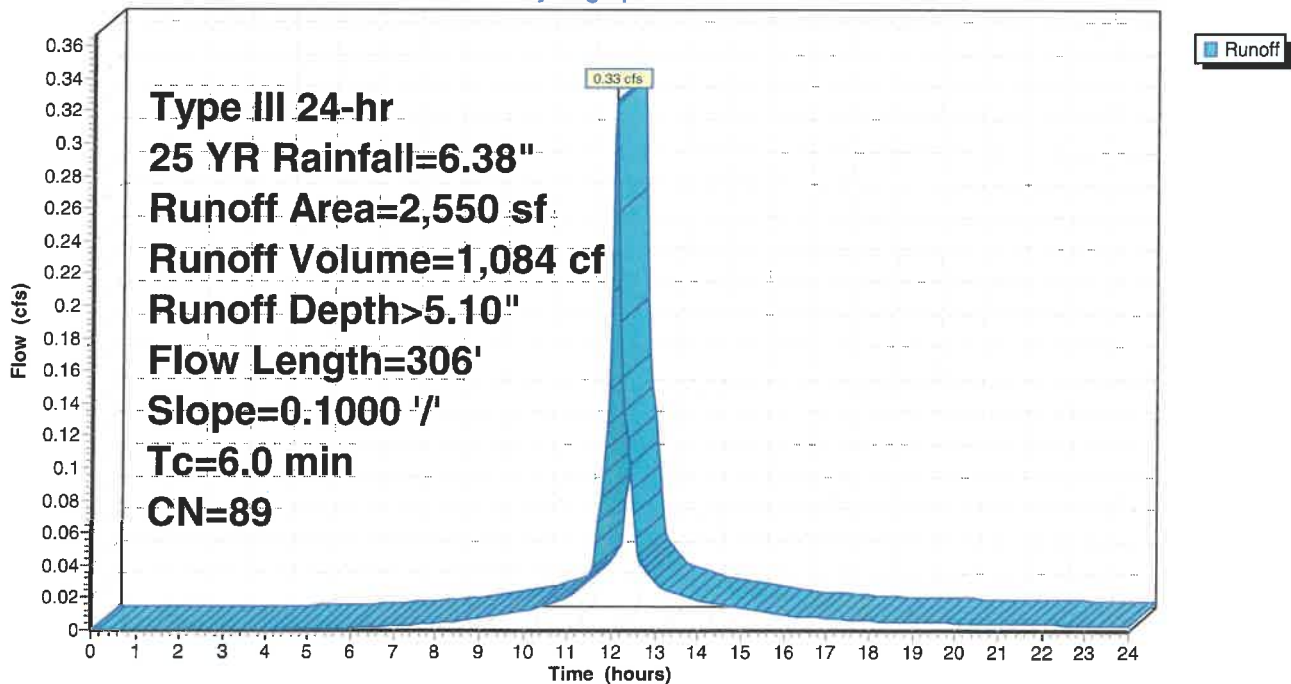
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 1,913 | 98 | Paved roads w/curbs & sewers, HSG B |
| 637 | 61 | >75% Grass cover, Good, HSG B |
| 2,550 | 89 | Weighted Average |
| 637 | | 24.98% Pervious Area |
| 1,913 | | 75.02% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 7S: To CB 4

Hydrograph



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Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Subcatchment 8S: To CB 6

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 1,467 cf, Depth> 4.77"

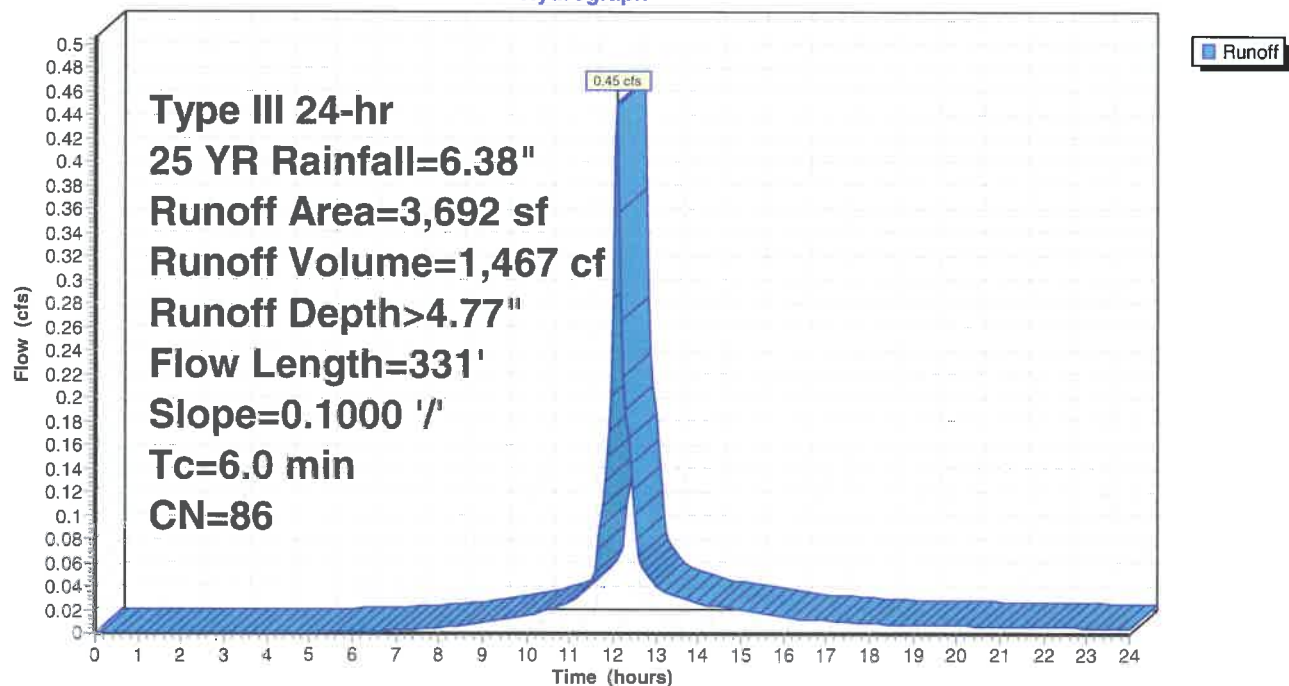
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,493 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,199 | 61 | >75% Grass cover, Good, HSG B |
| 3,692 | 86 | Weighted Average |
| 1,199 | | 32.48% Pervious Area |
| 2,493 | | 67.52% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 281 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 331 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 8S: To CB 6

Hydrograph



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Summary for Subcatchment 9S: To CB 5

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 1,618 cf, Depth> 5.55"

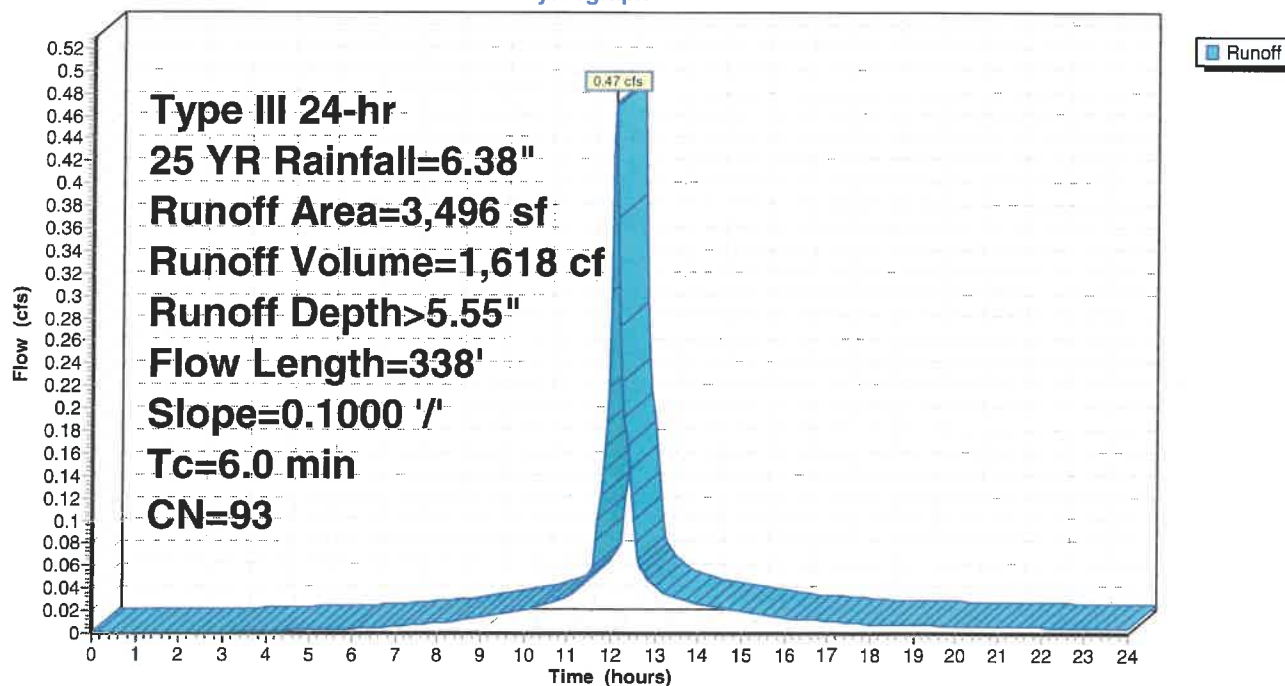
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 3,000 | 98 | Paved roads w/curbs & sewers, HSG B |
| 496 | 61 | >75% Grass cover, Good, HSG B |
| 3,496 | 93 | Weighted Average |
| 496 | | 14.19% Pervious Area |
| 3,000 | | 85.81% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 288 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 338 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 9S: To CB 5

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Summary for Subcatchment 10S: To CB 7

Runoff = 0.53 cfs @ 12.09 hrs, Volume= 1,729 cf, Depth> 4.77"

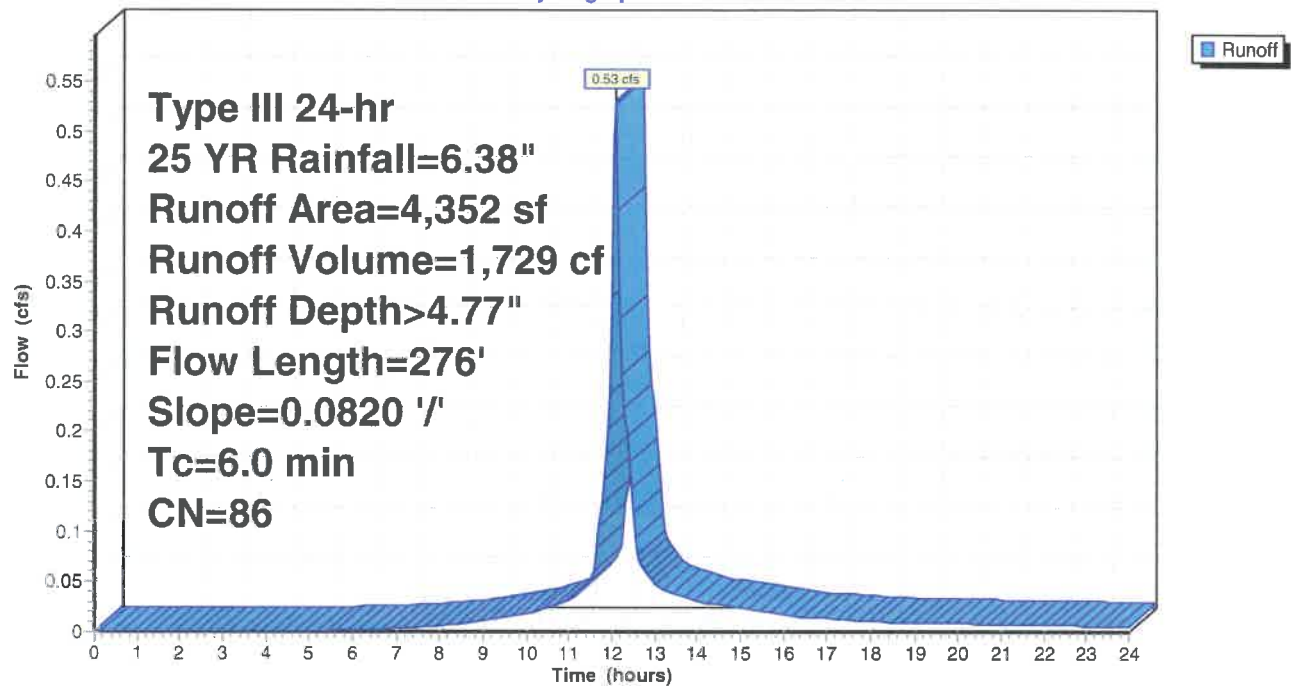
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,363 | 61 | >75% Grass cover, Good, HSG B |
| 4,352 | 86 | Weighted Average |
| 1,363 | | 31.32% Pervious Area |
| 2,989 | | 68.68% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.6 | 226 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.0 | 276 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 10S: To CB 7

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Summary for Subcatchment 11S: To CB 8

Runoff = 0.52 cfs @ 12.09 hrs, Volume= 1,695 cf, Depth> 4.99"

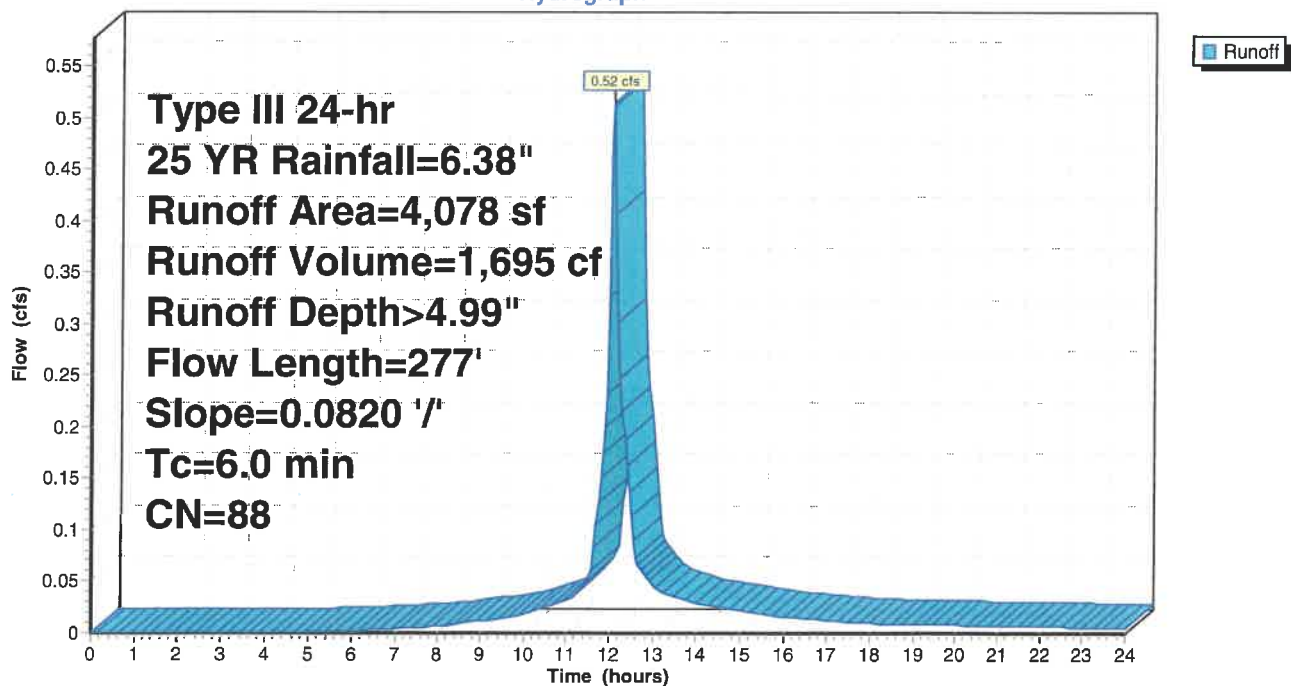
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,089 | 61 | >75% Grass cover, Good, HSG B |
| 4,078 | 88 | Weighted Average |
| 1,089 | | 26.70% Pervious Area |
| 2,989 | | 73.30% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 227 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 277 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 11S: To CB 8

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Summary for Subcatchment 12S: To CB 9

Runoff = 1.59 cfs @ 12.09 hrs, Volume= 5,138 cf, Depth> 4.55"

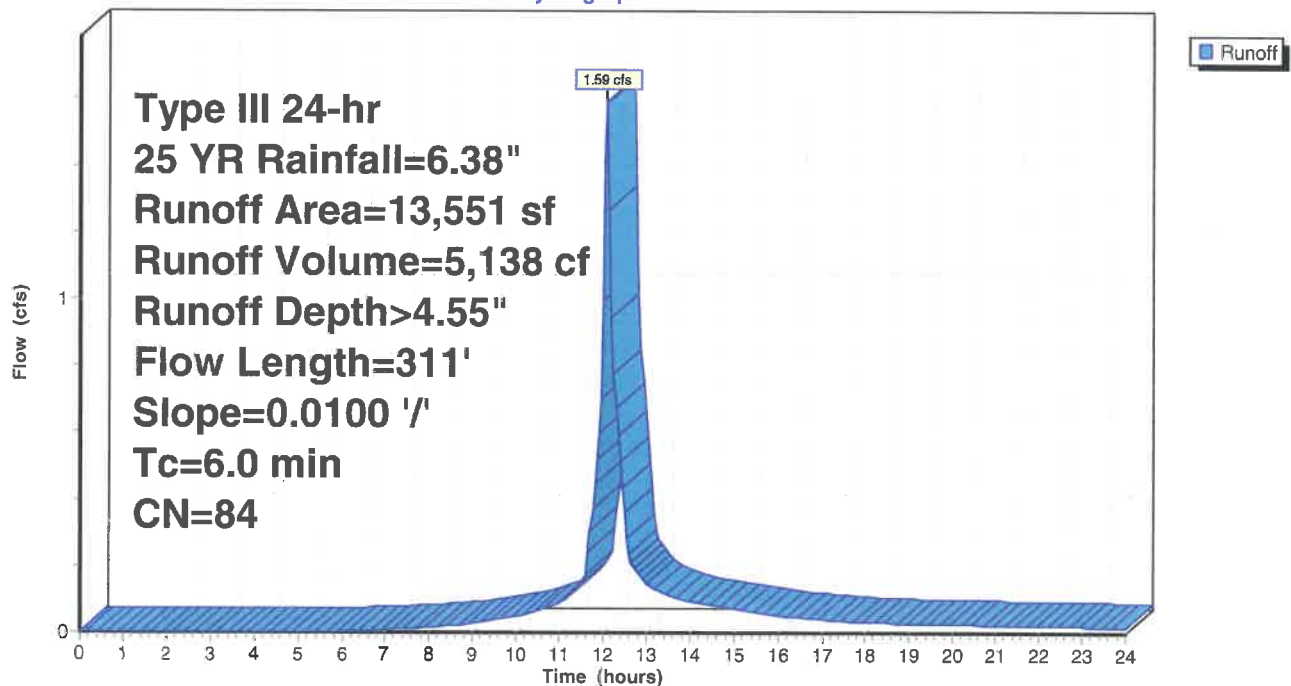
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| 5,218 | 61 | >75% Grass cover, Good, HSG B |
| 13,551 | 84 | Weighted Average |
| 5,218 | | 38.51% Pervious Area |
| 8,333 | | 61.49% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.1 | 261 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.0 | 311 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 12S: To CB 9

Hydrograph



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Summary for Subcatchment 13S: To CB 10

Runoff = 1.67 cfs @ 12.09 hrs, Volume= 5,344 cf, Depth> 4.33"

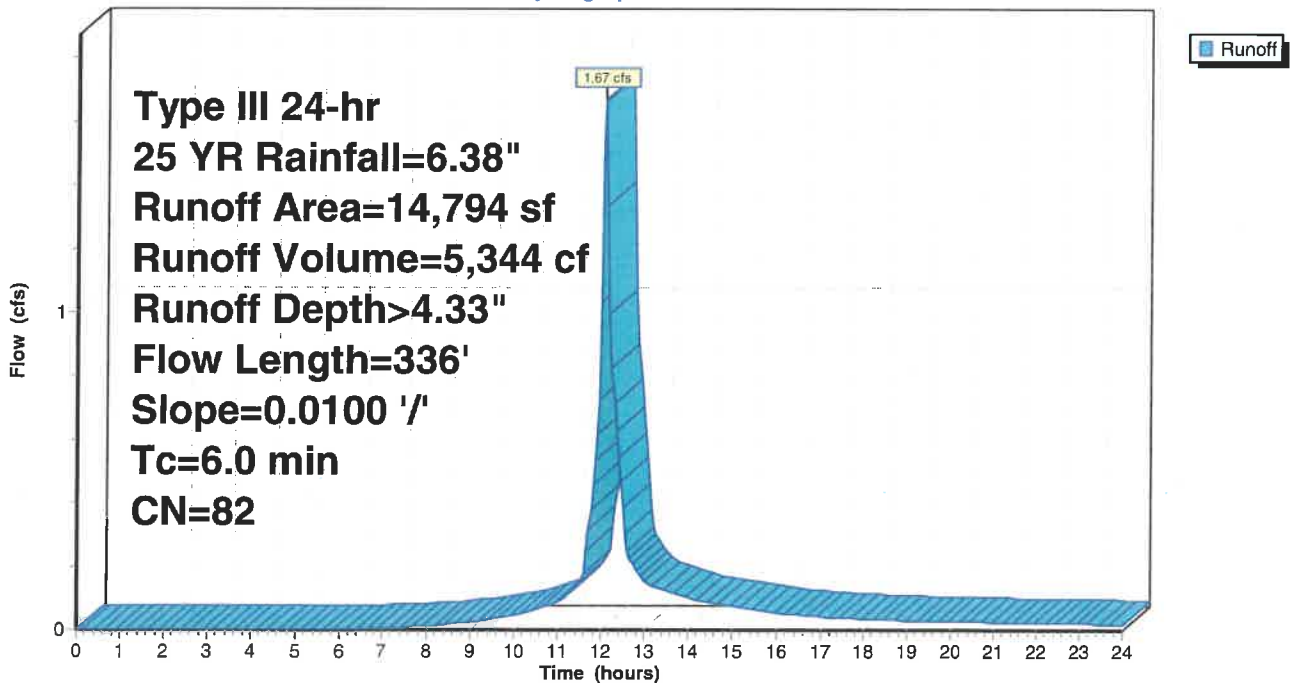
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| | Area (sf) | CN | Description |
|---|-----------|----|-------------------------------------|
| * | 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| | 6,461 | 61 | >75% Grass cover, Good, HSG B |
| | 14,794 | 82 | Weighted Average |
| | 6,461 | | 43.67% Pervious Area |
| | 8,333 | | 56.33% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|--|----------------------|-------------------|--|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.3 | 286 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.2 | 336 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 13S: To CB 10

Hydrograph



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Summary for Subcatchment 14S: To Wetland

Runoff = 26.13 cfs @ 12.36 hrs, Volume= 133,201 cf, Depth> 2.62"

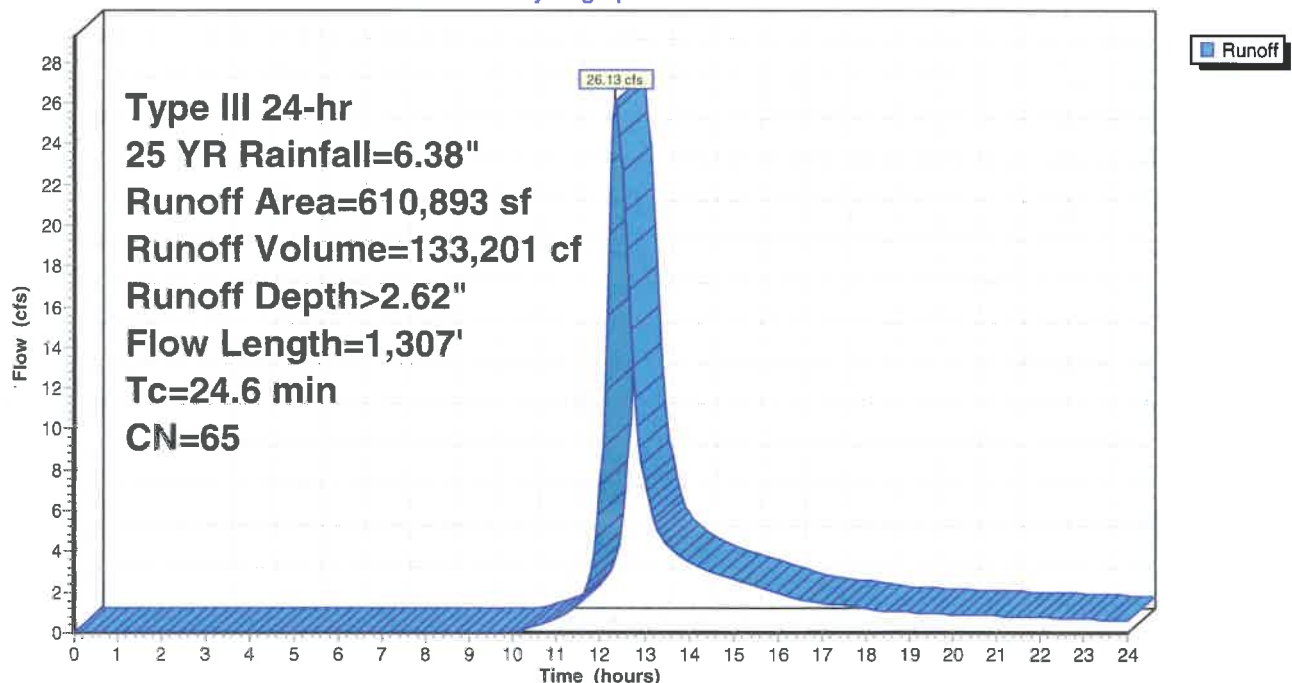
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 326,808 | 55 | Woods, Good, HSG B |
| 260,659 | 77 | Woods, Good, HSG D |
| 23,426 | 61 | >75% Grass cover, Good, HSG B |
| 610,893 | 65 | Weighted Average |
| 610,893 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 7.9 | 648 | 0.0750 | 1.37 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 6.9 | 609 | 0.0200 | 1.47 | 1.76 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 24.6 | 1,307 | Total | | | |

Subcatchment 14S: To Wetland

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Summary for Subcatchment 15S: TO BASIN

Runoff = 3.95 cfs @ 12.20 hrs, Volume= 16,894 cf, Depth> 1.82"

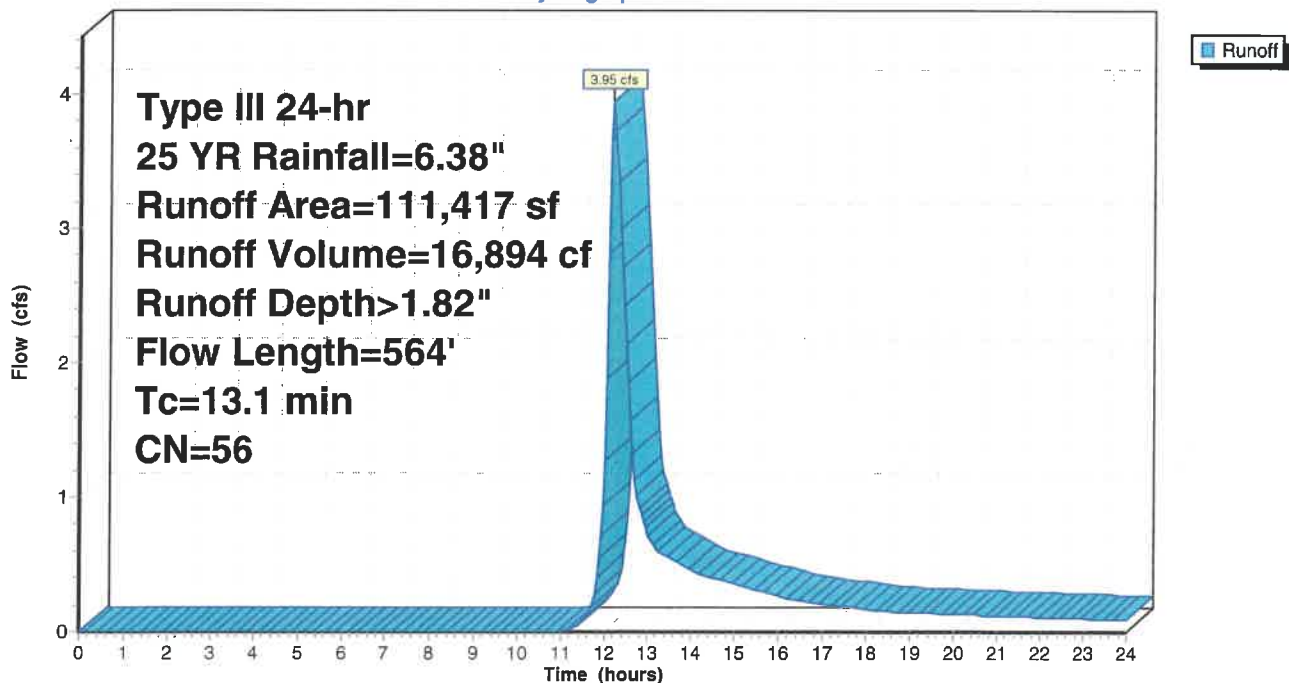
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 89,974 | 55 | Woods, Good, HSG B |
| 21,443 | 61 | >75% Grass cover, Good, HSG B |
| 111,417 | 56 | Weighted Average |
| 111,417 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|--|
| 7.8 | 50 | 0.0620 | 0.11 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 5.1 | 467 | 0.0931 | 1.53 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.2 | 47 | 0.3190 | 3.95 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 13.1 | 564 | Total | | | |

Subcatchment 15S: TO BASIN

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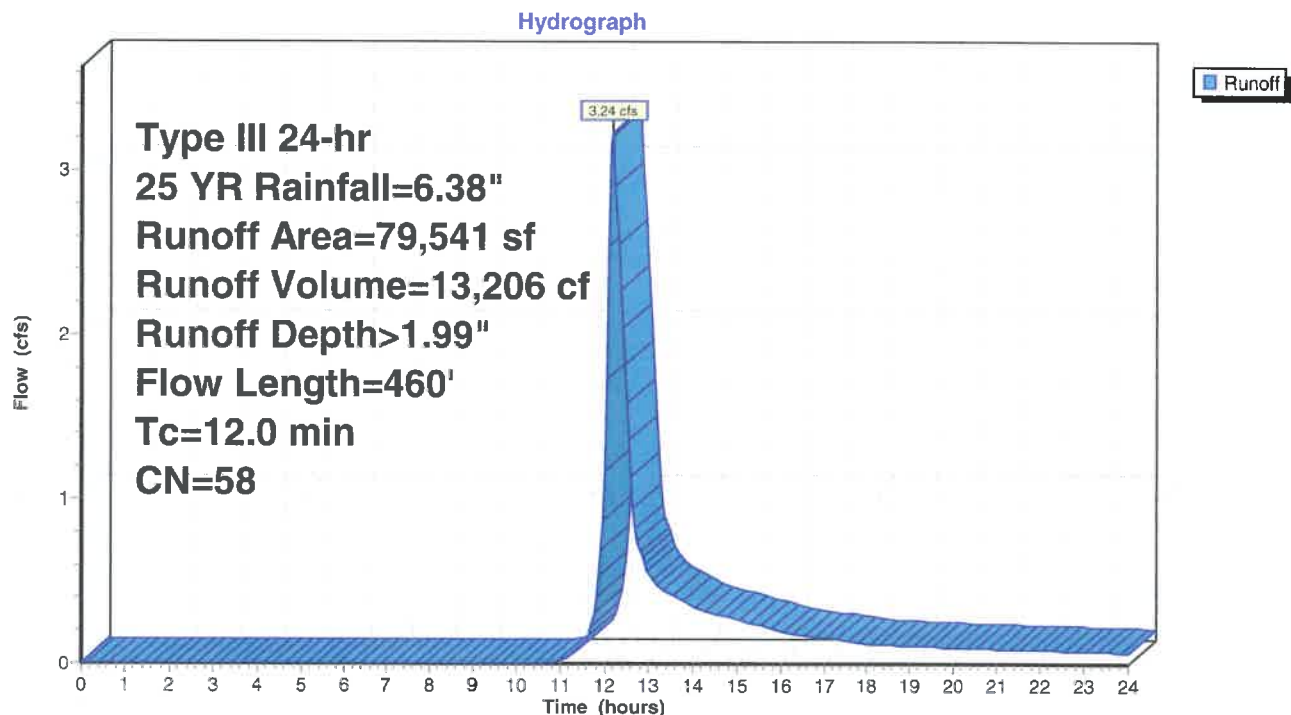
Summary for Subcatchment 16S: TO BASIN

Runoff = 3.24 cfs @ 12.18 hrs, Volume= 13,206 cf, Depth> 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 35,427 | 61 | >75% Grass cover, Good, HSG B |
| 44,114 | 55 | Woods, Good, HSG B |
| 79,541 | 58 | Weighted Average |
| 79,541 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 4.8 | 391 | 0.0735 | 1.36 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.1 | 19 | 0.3150 | 5.05 | | Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps |
| 12.0 | 460 | Total | | | |

Subcatchment 16S: TO BASIN

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Summary for Subcatchment 17S: To Off Site

Runoff = 6.37 cfs @ 12.17 hrs, Volume= 25,928 cf, Depth> 1.74"

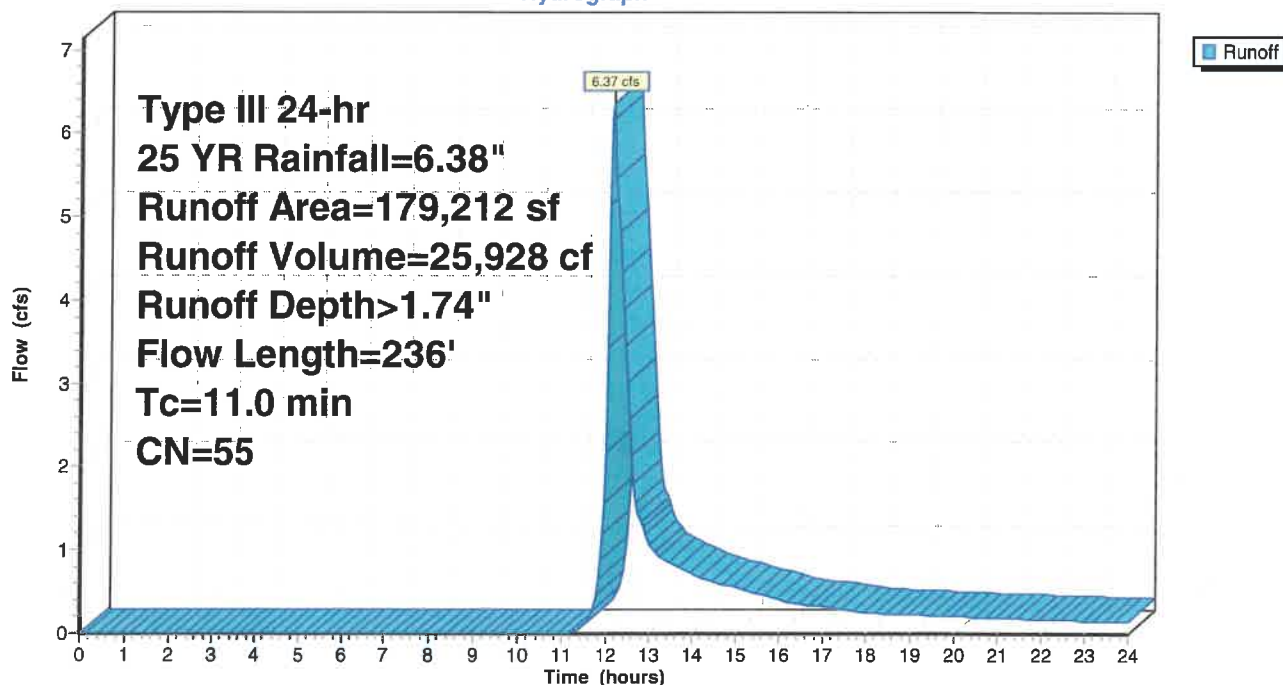
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 179,212 | 55 | Woods, Good, HSG B |
| 179,212 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 8.5 | 50 | 0.0500 | 0.10 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.5 | 186 | 0.0600 | 1.22 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 11.0 | 236 | Total | | | |

Subcatchment 17S: To Off Site

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Summary for Subcatchment 19S: Area To Basin3

Runoff = 1.81 cfs @ 12.10 hrs, Volume= 5,900 cf, Depth> 2.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

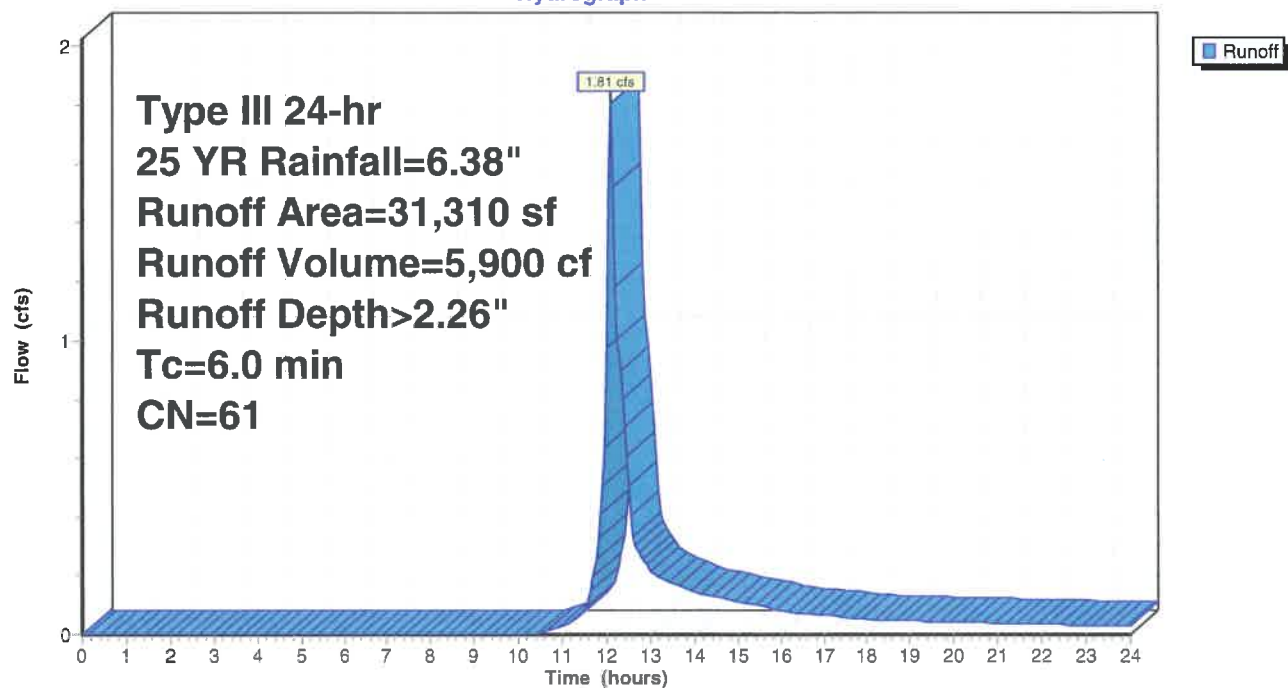
Type III 24-hr 25 YR Rainfall=6.38"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 31,310 | 61 | >75% Grass cover, Good, HSG B |
| 31,310 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|-------------------|
| 6.0 | | | | | Direct Entry, a-b |

Subcatchment 19S: Area To Basin3

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Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 2.25" for 25 YR event
Inflow = 16.88 cfs @ 12.41 hrs, Volume= 92,300 cf
Outflow = 16.87 cfs @ 12.42 hrs, Volume= 92,284 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.70 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 3.27 fps, Avg. Travel Time= 0.3 min

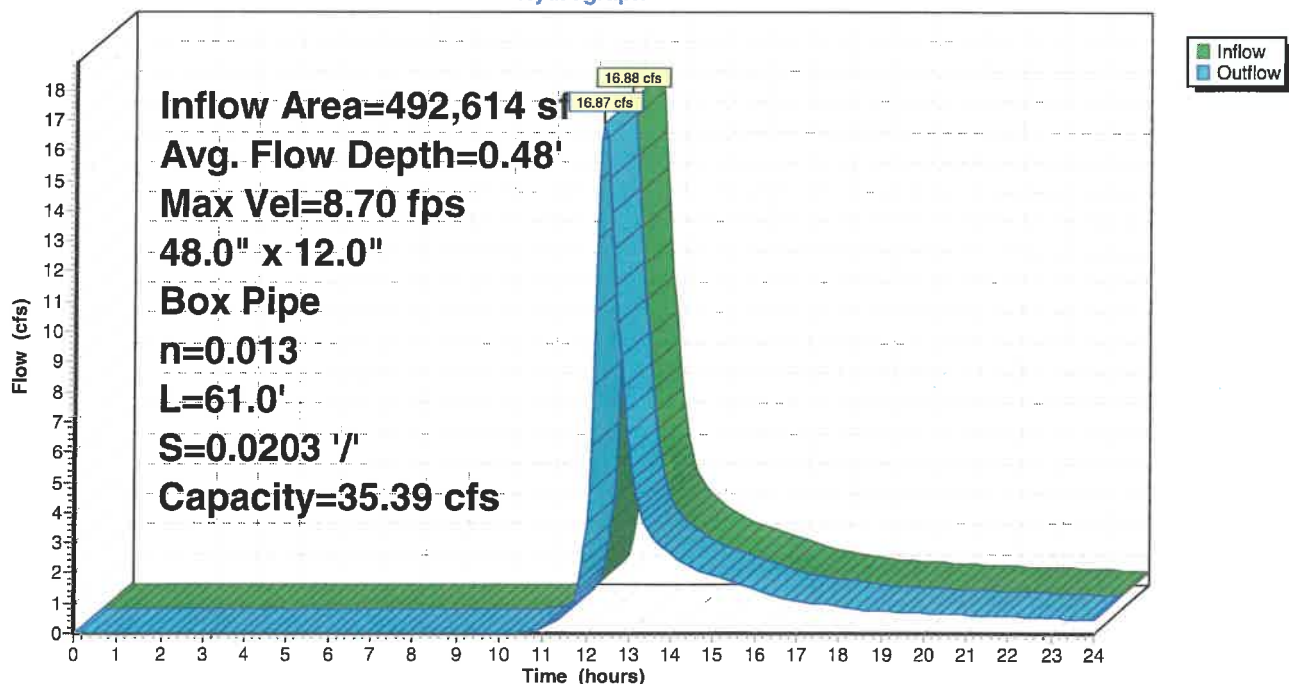
Peak Storage= 118 cf @ 12.41 hrs
Average Depth at Peak Storage= 0.48' , Surface Width= 4.00'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe
n= 0.013 Concrete, trowel finish
Length= 61.0' Slope= 0.0203 1/'
Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

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Summary for Reach 2R: New Box Cumvert

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 1.85" for 25 YR event
Inflow = 45.93 cfs @ 13.06 hrs, Volume= 425,751 cf
Outflow = 45.89 cfs @ 13.07 hrs, Volume= 425,722 cf, Atten= 0%, Lag= 0.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 12.64 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 6.35 fps, Avg. Travel Time= 0.1 min

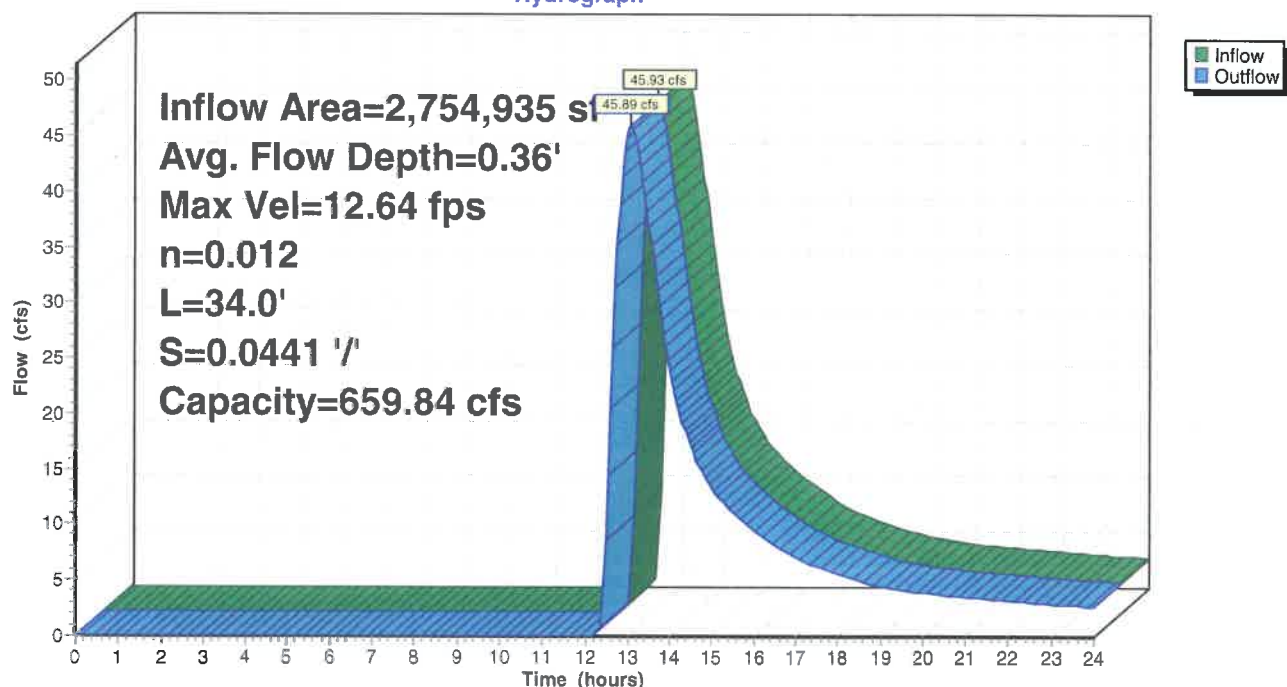
Peak Storage= 123 cf @ 13.07 hrs
Average Depth at Peak Storage= 0.36' , Surface Width= 10.00'
Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 659.84 cfs

10.00' x 2.00' deep channel, n= 0.012 Concrete pipe, finished
Length= 34.0' Slope= 0.0441 '/'
Inlet Invert= 294.00', Outlet Invert= 292.50'



Reach 2R: New Box Cumvert

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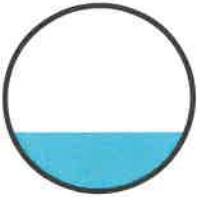
Summary for Reach 3R: CB1 to DMH 1

Inflow Area = 13,094 sf, 84.66% Impervious, Inflow Depth > 5.44" for 25 YR event
Inflow = 1.75 cfs @ 12.09 hrs, Volume= 5,936 cf
Outflow = 1.75 cfs @ 12.09 hrs, Volume= 5,936 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.17 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.71 fps, Avg. Travel Time= 0.0 min

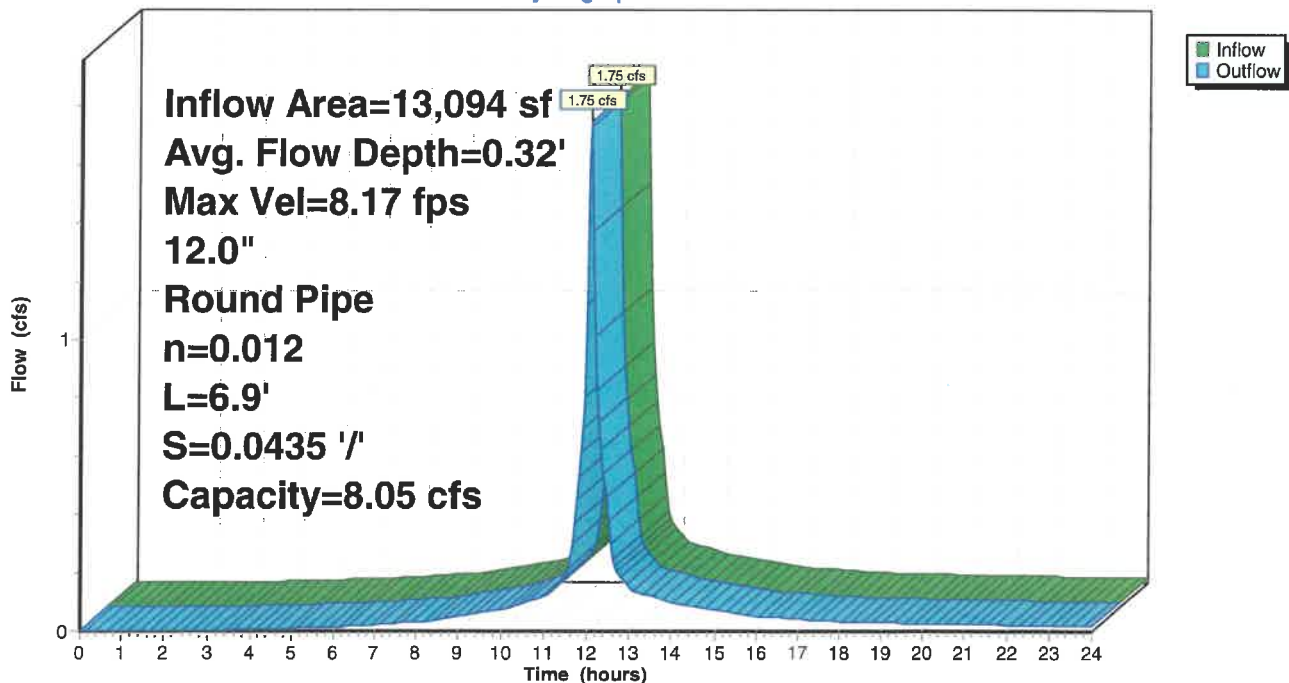
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.32', Surface Width= 0.93'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 3R: CB1 to DMH 1

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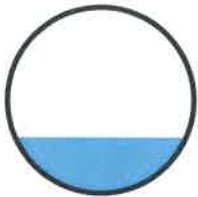
Summary for Reach 4R: CB2 to DMH 1

Inflow Area = 11,319 sf, 94.70% Impervious, Inflow Depth > 5.90" for 25 YR event
Inflow = 1.57 cfs @ 12.09 hrs, Volume= 5,567 cf
Outflow = 1.57 cfs @ 12.09 hrs, Volume= 5,567 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.92 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 2.64 fps, Avg. Travel Time= 0.0 min

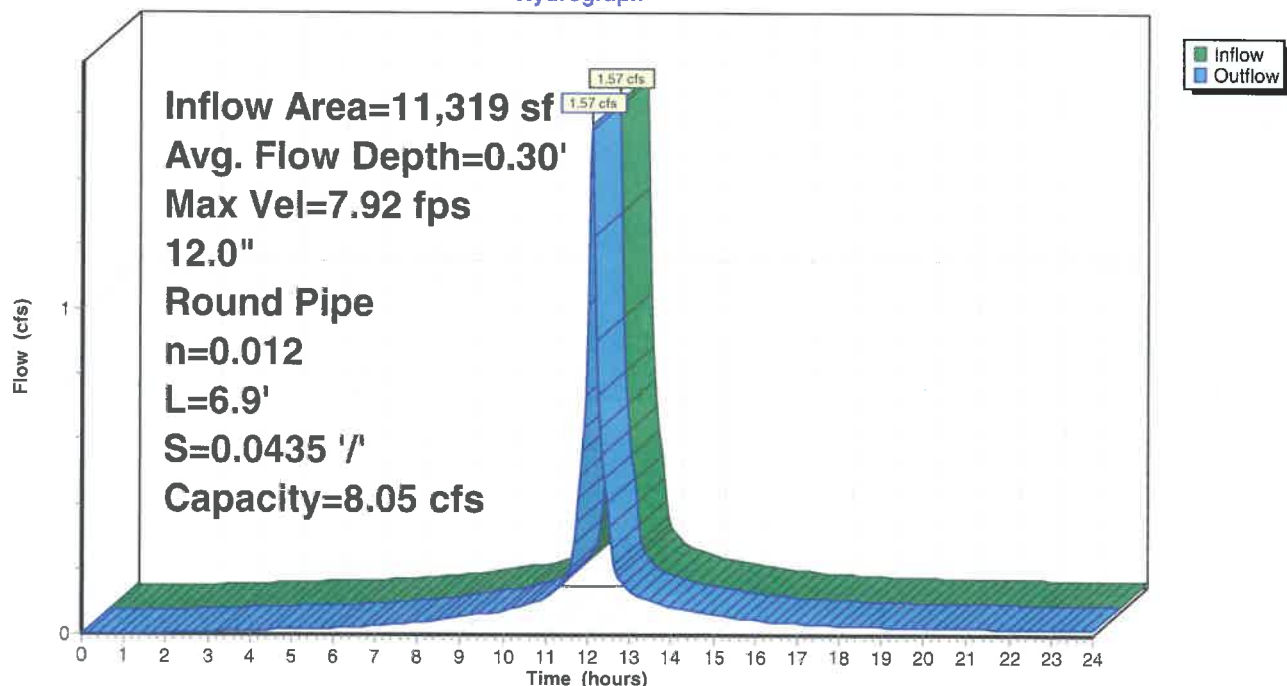
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.30', Surface Width= 0.92'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 4R: CB2 to DMH 1

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Summary for Reach 5R: DMH1 to DMH 2

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 5.65" for 25 YR event
Inflow = 3.32 cfs @ 12.09 hrs, Volume= 11,503 cf
Outflow = 3.30 cfs @ 12.09 hrs, Volume= 11,501 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.56 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 1.82 fps, Avg. Travel Time= 0.7 min

Peak Storage= 45 cf @ 12.09 hrs

Average Depth at Peak Storage= 0.56', Surface Width= 1.45'

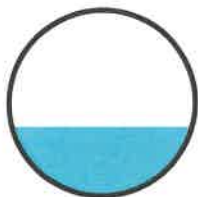
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 11.30 cfs

18.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

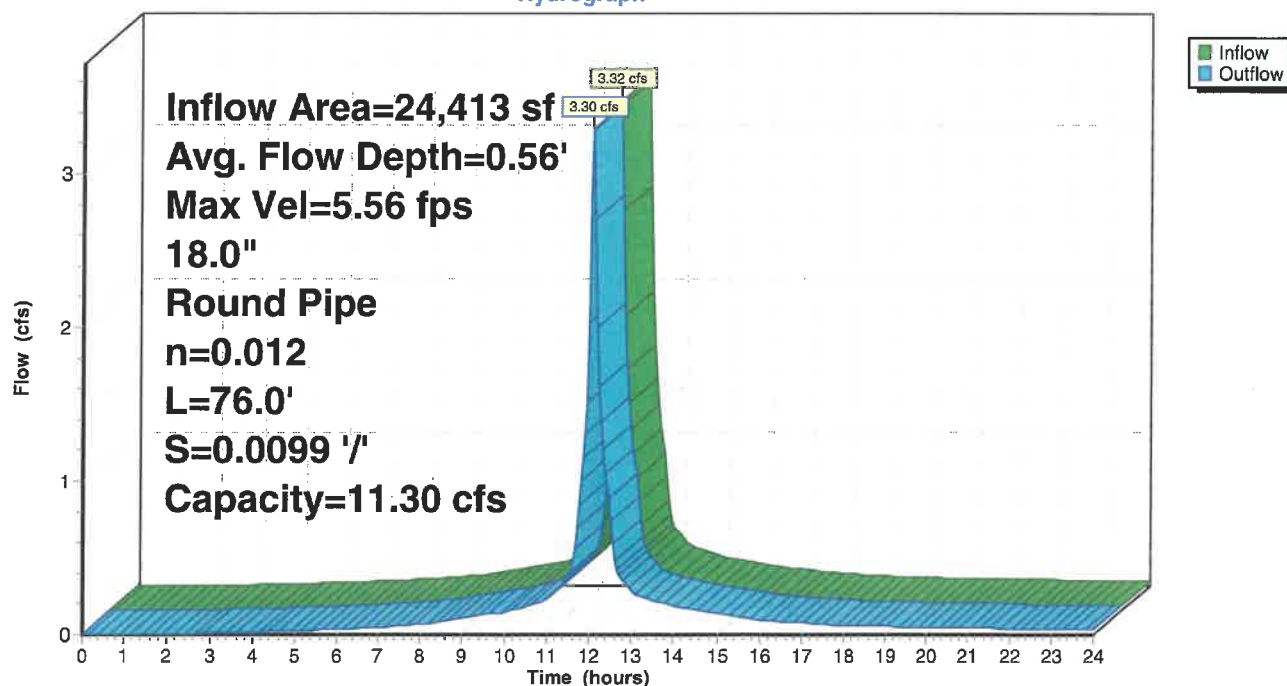
Length= 76.0' Slope= 0.0099 '/'

Inlet Invert= 291.75', Outlet Invert= 291.00'



Reach 5R: DMH1 to DMH 2

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Summary for Reach 6R: CB3 to DMH 2

Inflow Area = 3,124 sf, 75.00% Impervious, Inflow Depth > 5.10" for 25 YR event
Inflow = 0.40 cfs @ 12.09 hrs, Volume= 1,328 cf
Outflow = 0.40 cfs @ 12.09 hrs, Volume= 1,328 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.47 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.82 fps, Avg. Travel Time= 0.1 min

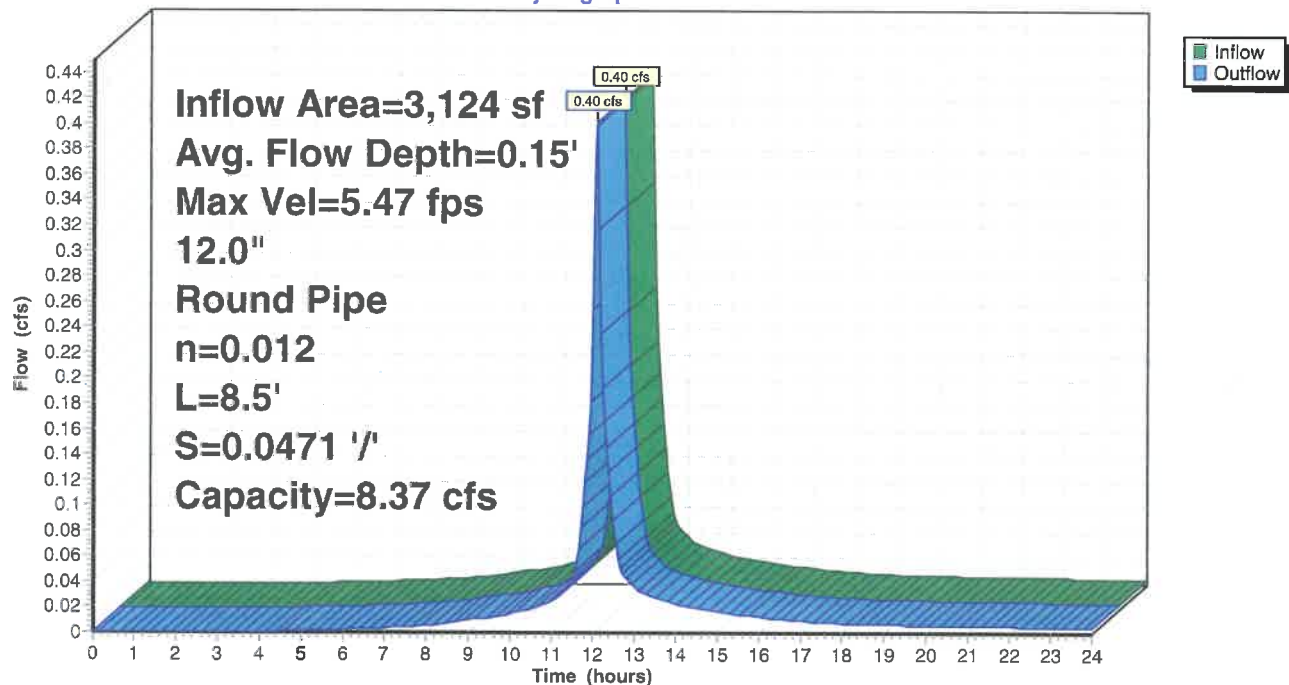
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.15' , Surface Width= 0.71'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 8.5' Slope= 0.0471 '/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 6R: CB3 to DMH 2

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Summary for Reach 7R: CB4 to DMH 2

Inflow Area = 2,550 sf, 75.02% Impervious, Inflow Depth > 5.10" for 25 YR event
Inflow = 0.33 cfs @ 12.09 hrs, Volume= 1,084 cf
Outflow = 0.33 cfs @ 12.09 hrs, Volume= 1,084 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.33 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.44 fps, Avg. Travel Time= 0.2 min

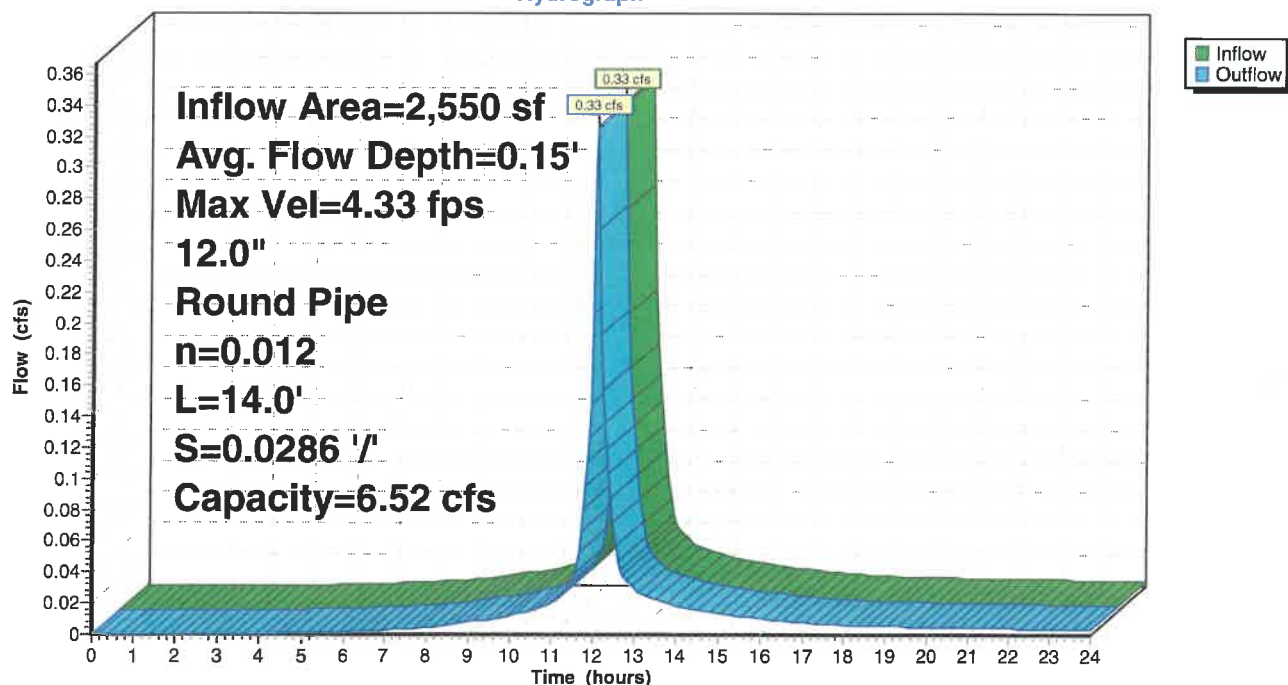
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.15', Surface Width= 0.72'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 6.52 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 14.0' Slope= 0.0286 '/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 7R: CB4 to DMH 2

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Summary for Reach 8R: DMH 2 TO DMH 7

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 5.13" for 25 YR event
Inflow = 1.63 cfs @ 12.10 hrs, Volume= 5,496 cf
Outflow = 1.58 cfs @ 12.11 hrs, Volume= 5,492 cf, Atten= 3%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.85 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 1.88 fps, Avg. Travel Time= 2.1 min

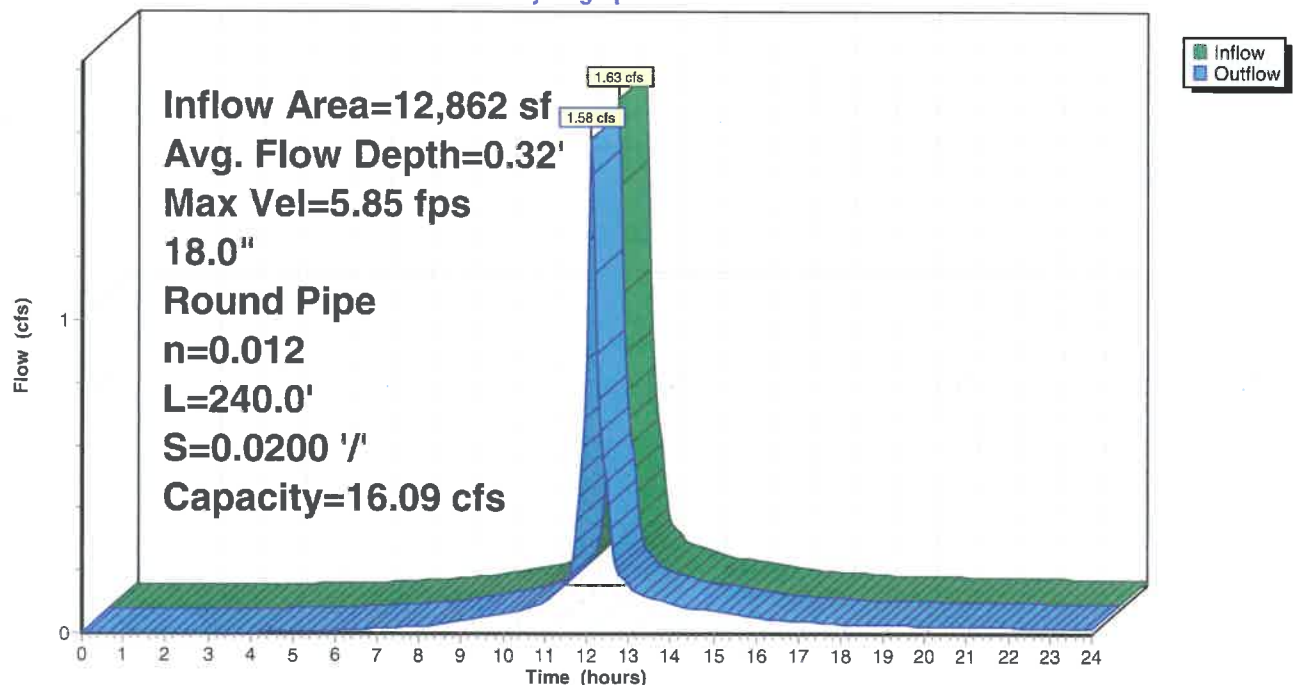
Peak Storage= 67 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.32', Surface Width= 1.23'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 16.09 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0200 '/'
Inlet Invert= 294.80', Outlet Invert= 290.00'



Reach 8R: DMH 2 TO DMH 7

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Summary for Reach 9R: DMH 3 to DMH 2

Inflow Area = 7,188 sf, 76.42% Impervious, Inflow Depth > 5.15" for 25 YR event
Inflow = 0.92 cfs @ 12.09 hrs, Volume= 3,085 cf
Outflow = 0.91 cfs @ 12.10 hrs, Volume= 3,084 cf, Atten= 2%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.47 fps, Min. Travel Time= 0.4 min
Avg. Velocity = 2.73 fps, Avg. Travel Time= 1.3 min

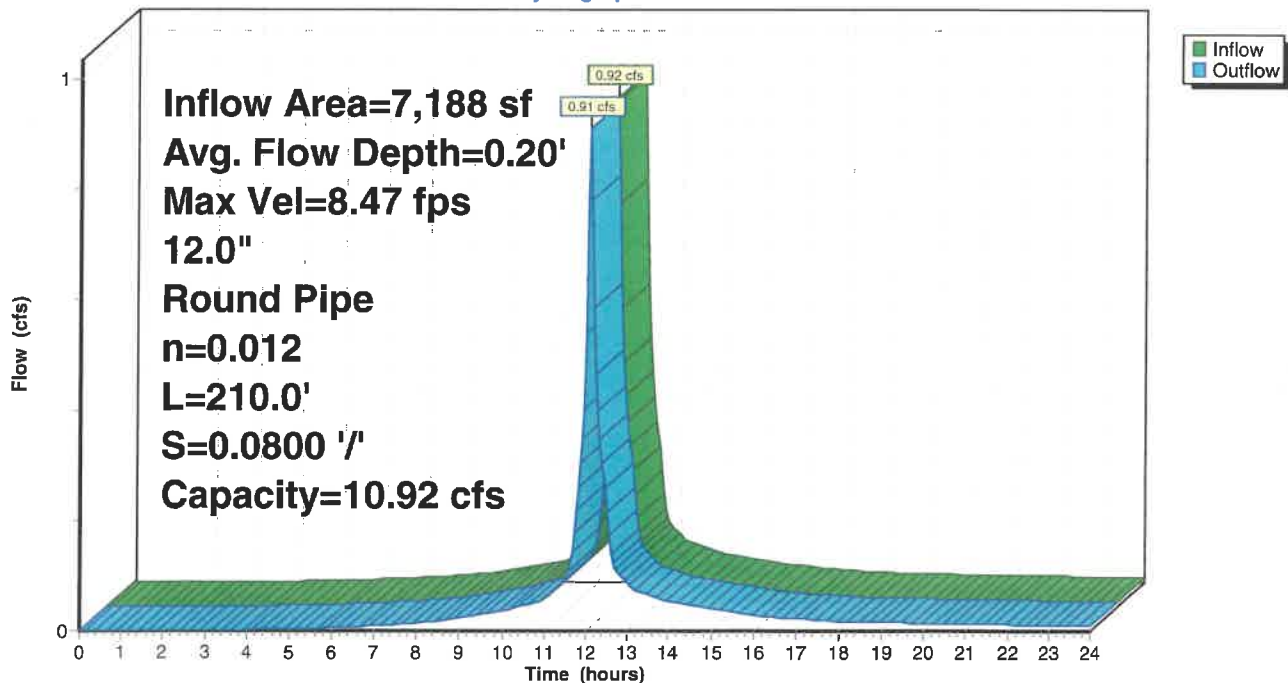
Peak Storage= 23 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.20' , Surface Width= 0.80'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 210.0' Slope= 0.0800 '/'
Inlet Invert= 315.40', Outlet Invert= 298.60'



Reach 9R: DMH 3 to DMH 2

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Summary for Reach 10R: CB5 to DMH 3

Inflow Area = 3,692 sf, 67.52% Impervious, Inflow Depth > 4.77" for 25 YR event
Inflow = 0.45 cfs @ 12.09 hrs, Volume= 1,467 cf
Outflow = 0.45 cfs @ 12.09 hrs, Volume= 1,467 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.54 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 1.86 fps, Avg. Travel Time= 0.1 min

Peak Storage= 1 cf @ 12.09 hrs

Average Depth at Peak Storage= 0.16' , Surface Width= 0.73'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.11 cfs

12.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

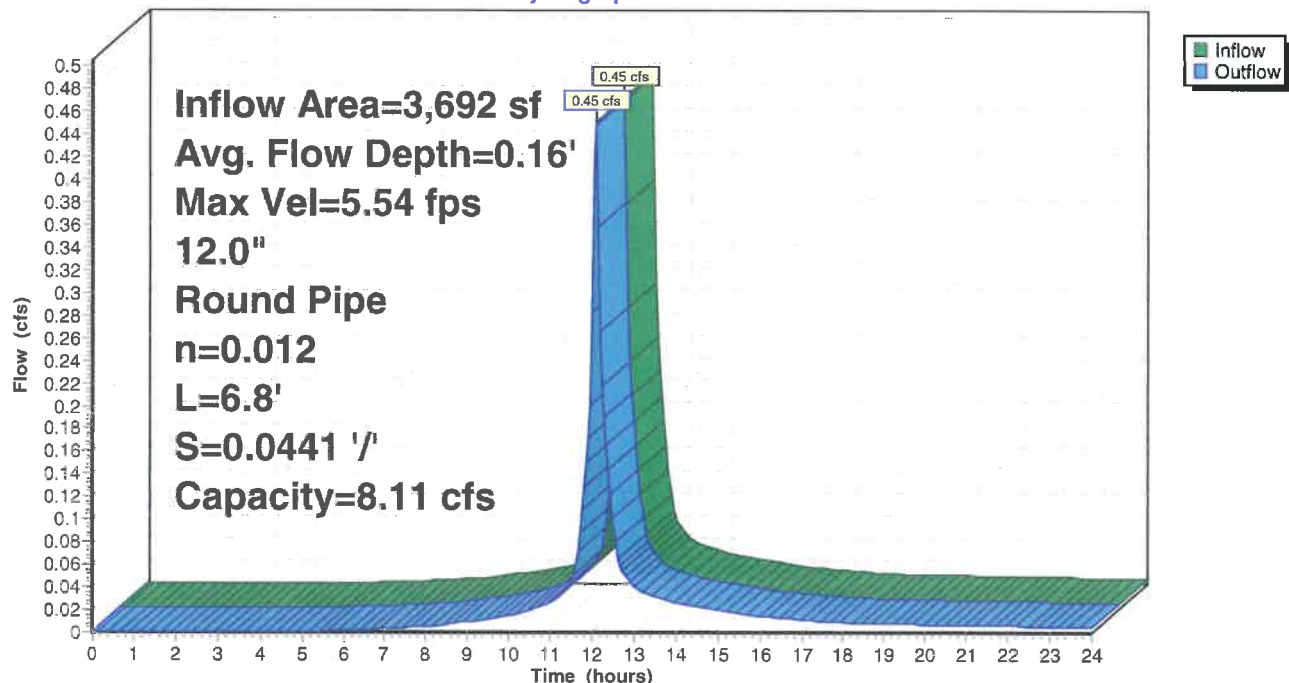
Length= 6.8' Slope= 0.0441 '/'

Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 10R: CB5 to DMH 3

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Summary for Reach 11R: CB6 to DMH 3

Inflow Area = 3,496 sf, 85.81% Impervious, Inflow Depth > 5.55" for 25 YR event
Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,618 cf
Outflow = 0.47 cfs @ 12.09 hrs, Volume= 1,618 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.42 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.46 fps, Avg. Travel Time= 0.2 min

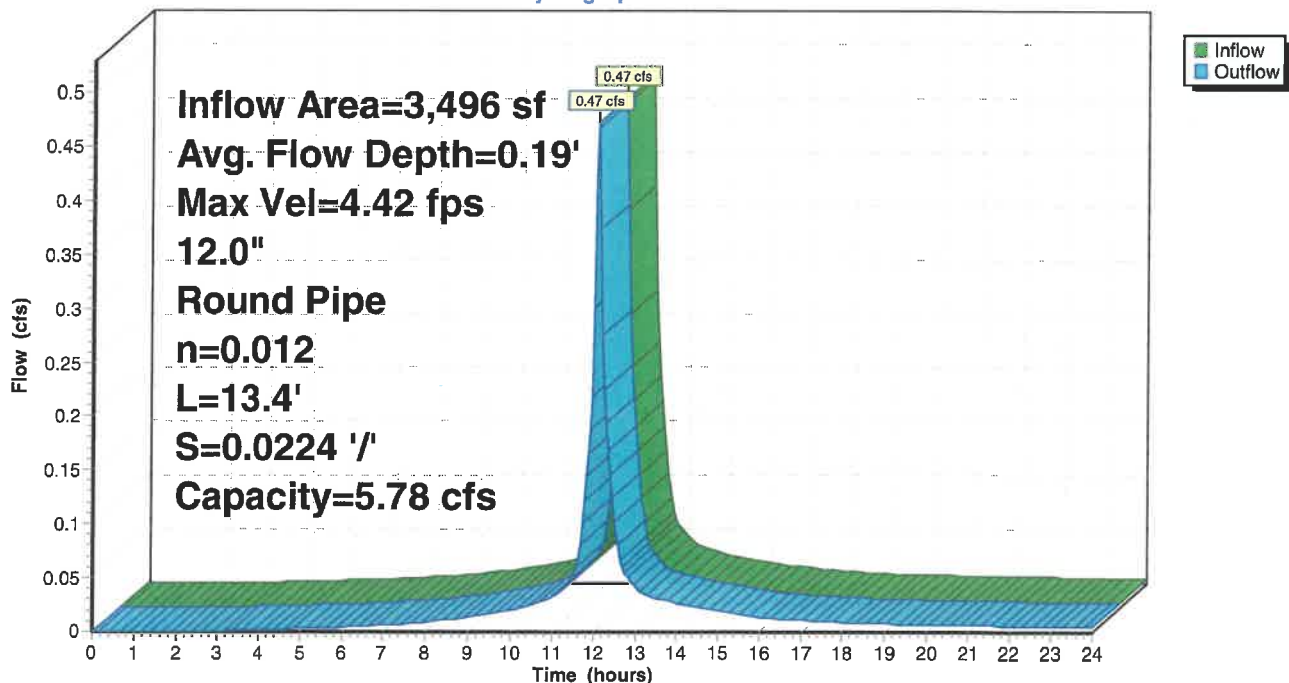
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.19', Surface Width= 0.79'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.78 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0224 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 11R: CB6 to DMH 3

Hydrograph



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Summary for Reach 12R: DMH 7 TO BASIN

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 5.12" for 25 YR event
Inflow = 1.58 cfs @ 12.11 hrs, Volume= 5,492 cf
Outflow = 1.53 cfs @ 12.13 hrs, Volume= 5,489 cf, Atten= 3%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.56 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 2.16 fps, Avg. Travel Time= 1.9 min

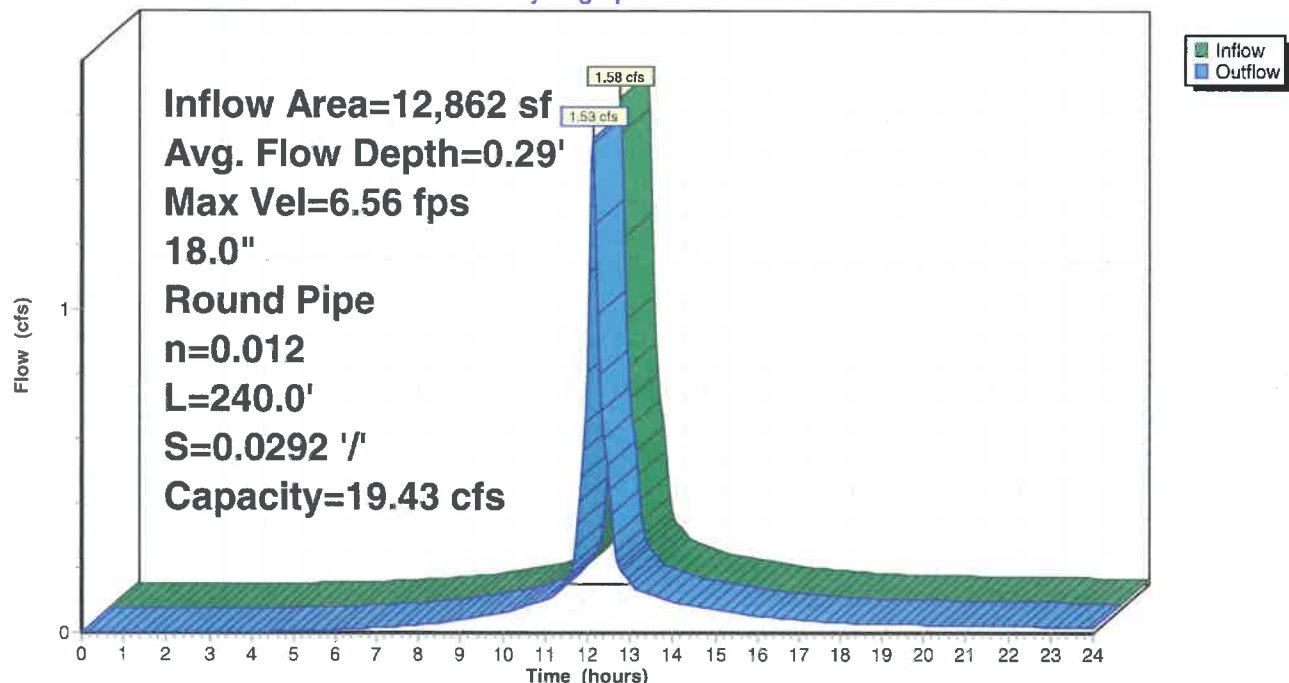
Peak Storage= 57 cf @ 12.12 hrs
Average Depth at Peak Storage= 0.29' , Surface Width= 1.18'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 19.43 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0292 '/'
Inlet Invert= 290.00', Outlet Invert= 283.00'



Reach 12R: DMH 7 TO BASIN

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Summary for Reach 13R: CB7 TO DMH 4

Inflow Area = 4,352 sf, 68.68% Impervious, Inflow Depth > 4.77" for 25 YR event
Inflow = 0.53 cfs @ 12.09 hrs, Volume= 1,729 cf
Outflow = 0.53 cfs @ 12.09 hrs, Volume= 1,729 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 5.38 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 1.80 fps, Avg. Travel Time= 0.1 min

Peak Storage= 1 cf @ 12.09 hrs

Average Depth at Peak Storage= 0.18', Surface Width= 0.77'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.24 cfs

12.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

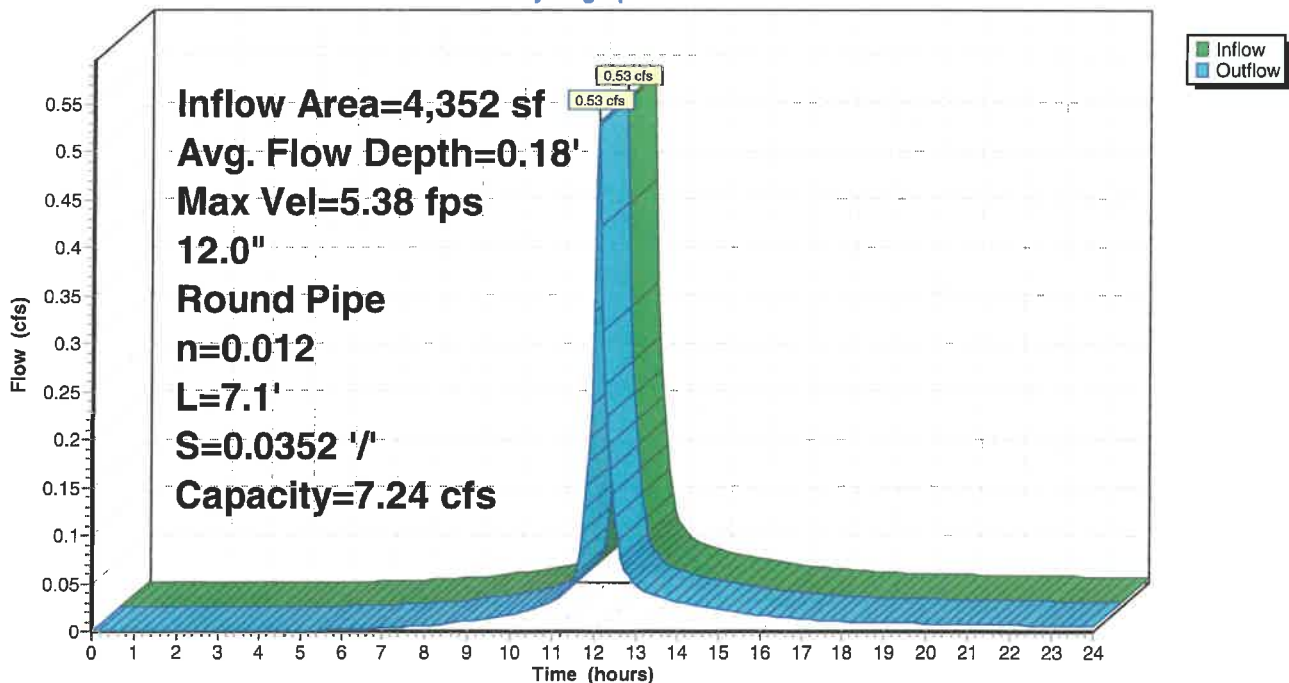
Length= 7.1' Slope= 0.0352 '/'

Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 13R: CB7 TO DMH 4

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Summary for Reach 14R: CB 8 TO DMH 4

Inflow Area = 4,078 sf, 73.30% Impervious, Inflow Depth > 4.99" for 25 YR event
Inflow = 0.52 cfs @ 12.09 hrs, Volume= 1,695 cf
Outflow = 0.51 cfs @ 12.09 hrs, Volume= 1,695 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.26 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.41 fps, Avg. Travel Time= 0.2 min

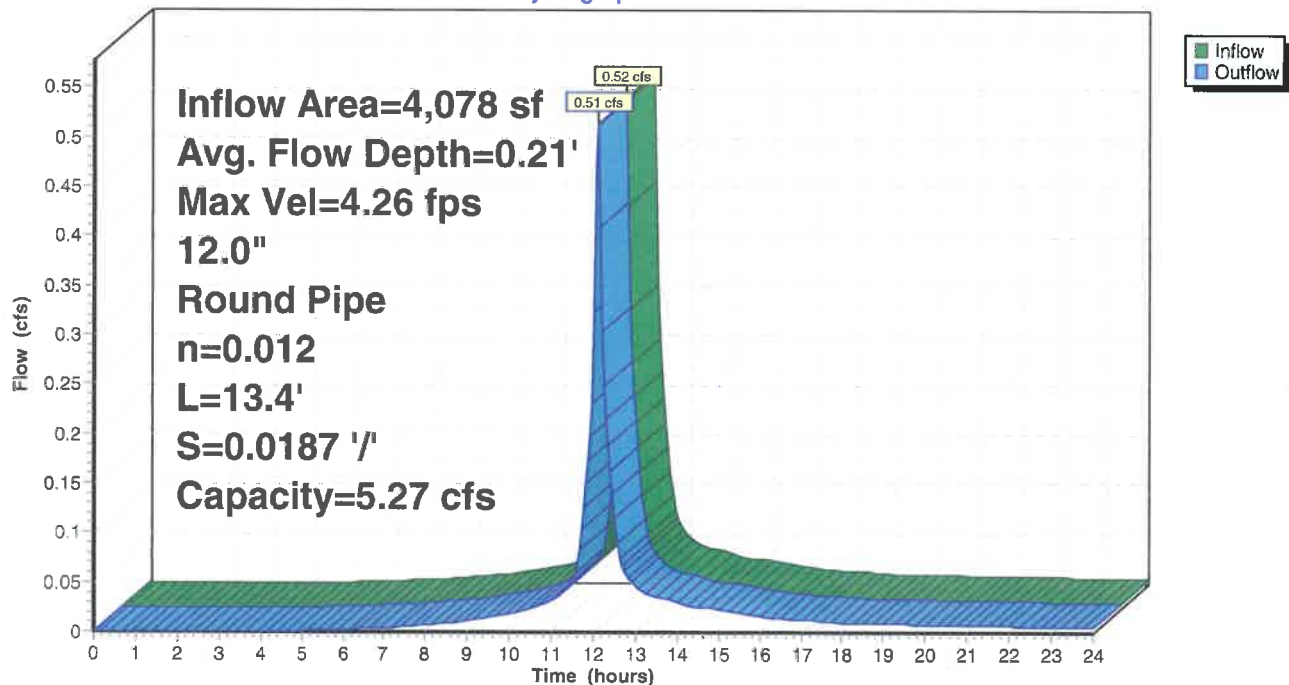
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.21', Surface Width= 0.82'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.27 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0187 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 14R: CB 8 TO DMH 4

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Summary for Reach 15R: DMH 4 TO DMH 5

Inflow Area = 8,430 sf, 70.91% Impervious, Inflow Depth > 4.87" for 25 YR event
Inflow = 1.05 cfs @ 12.09 hrs, Volume= 3,424 cf
Outflow = 1.04 cfs @ 12.09 hrs, Volume= 3,424 cf, Atten= 1%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.78 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.90 fps, Avg. Travel Time= 0.6 min

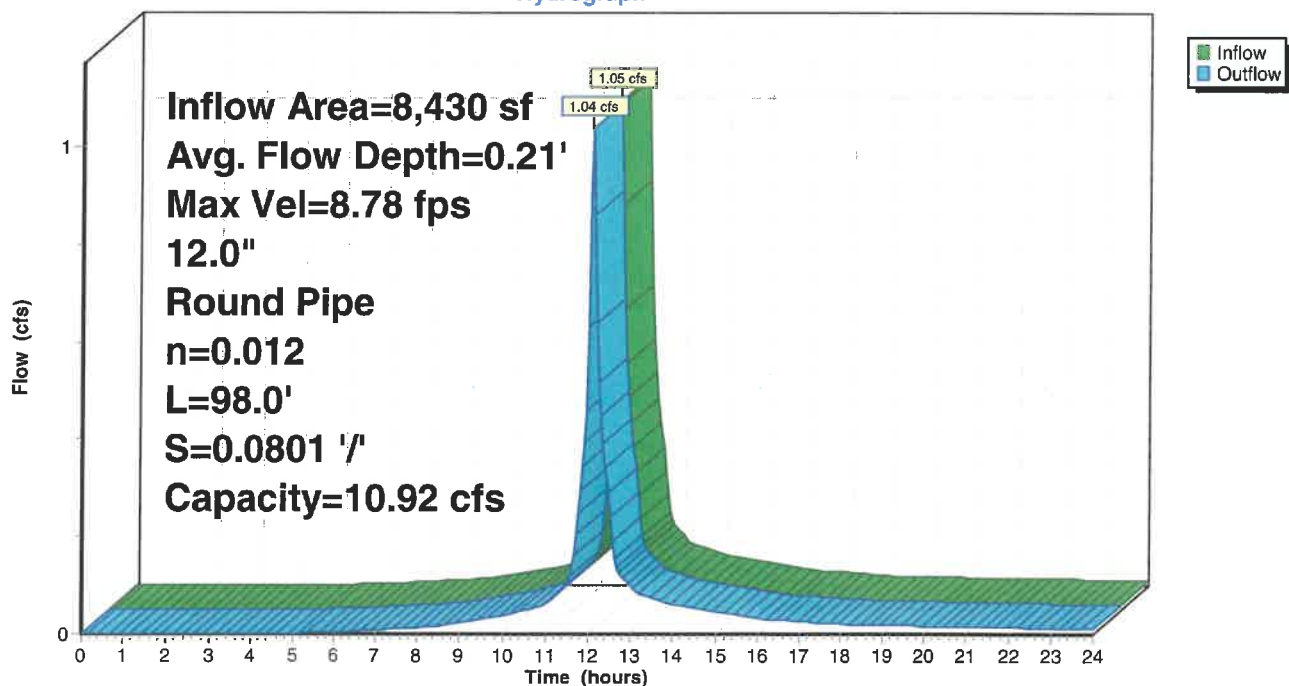
Peak Storage= 12 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.21' , Surface Width= 0.81'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 98.0' Slope= 0.0801 '/'
Inlet Invert= 316.20', Outlet Invert= 308.35'



Reach 15R: DMH 4 TO DMH 5

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Summary for Reach 16R: DMH 5 TO DMH 6

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 4.54" for 25 YR event
Inflow = 4.28 cfs @ 12.10 hrs, Volume= 13,903 cf
Outflow = 4.13 cfs @ 12.11 hrs, Volume= 13,895 cf, Atten= 3%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.01 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 2.03 fps, Avg. Travel Time= 2.0 min

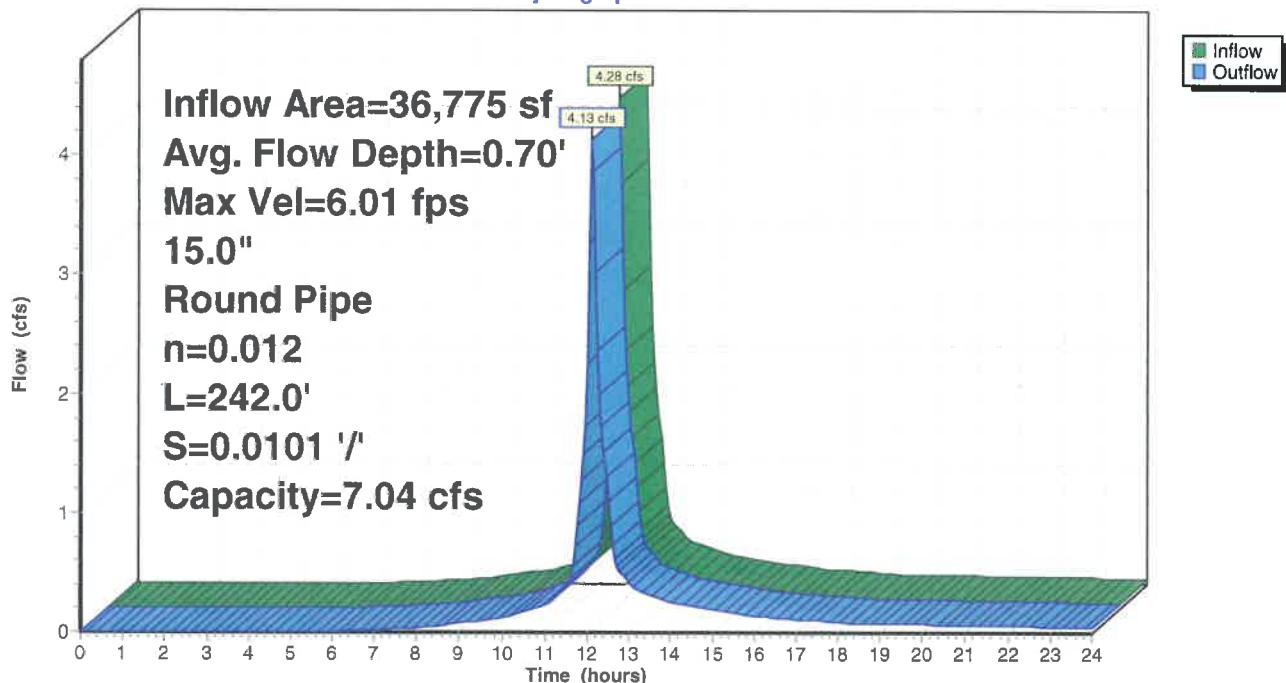
Peak Storage= 172 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.70' , Surface Width= 1.24'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.04 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 242.0' Slope= 0.0101 '/'
Inlet Invert= 308.10', Outlet Invert= 305.65'



Reach 16R: DMH 5 TO DMH 6

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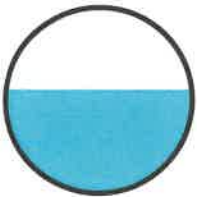
Summary for Reach 17R: DMH 6 TO DMH 5

Inflow Area = 28,345 sf, 58.80% Impervious, Inflow Depth > 4.44" for 25 YR event
Inflow = 3.26 cfs @ 12.09 hrs, Volume= 10,482 cf
Outflow = 3.24 cfs @ 12.10 hrs, Volume= 10,479 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.25 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.52 fps, Avg. Travel Time= 0.6 min

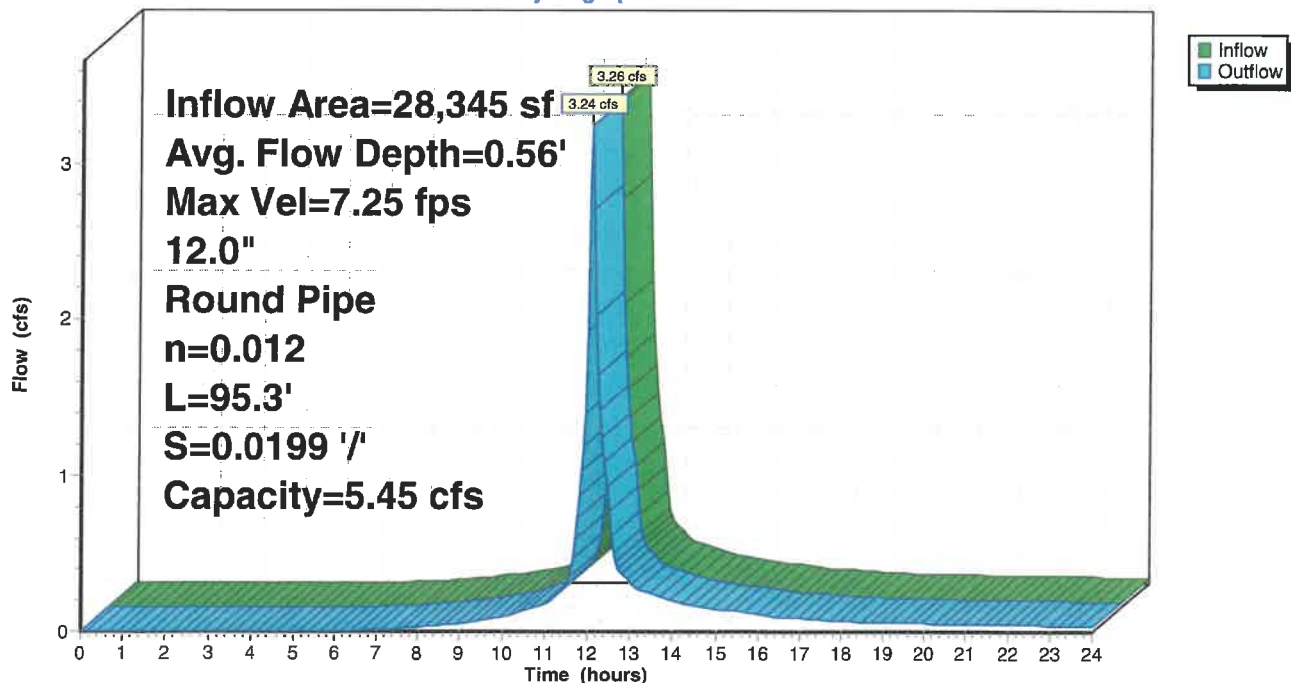
Peak Storage= 43 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.56', Surface Width= 0.99'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.45 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 95.3' Slope= 0.0199 '/'
Inlet Invert= 310.25', Outlet Invert= 308.35'



Reach 17R: DMH 6 TO DMH 5

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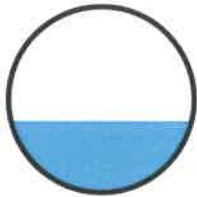
Summary for Reach 18R: CB 10 TO DMH 6

Inflow Area = 14,794 sf, 56.33% Impervious, Inflow Depth > 4.33" for 25 YR event
Inflow = 1.67 cfs @ 12.09 hrs, Volume= 5,344 cf
Outflow = 1.67 cfs @ 12.09 hrs, Volume= 5,344 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.03 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.08 fps, Avg. Travel Time= 0.1 min

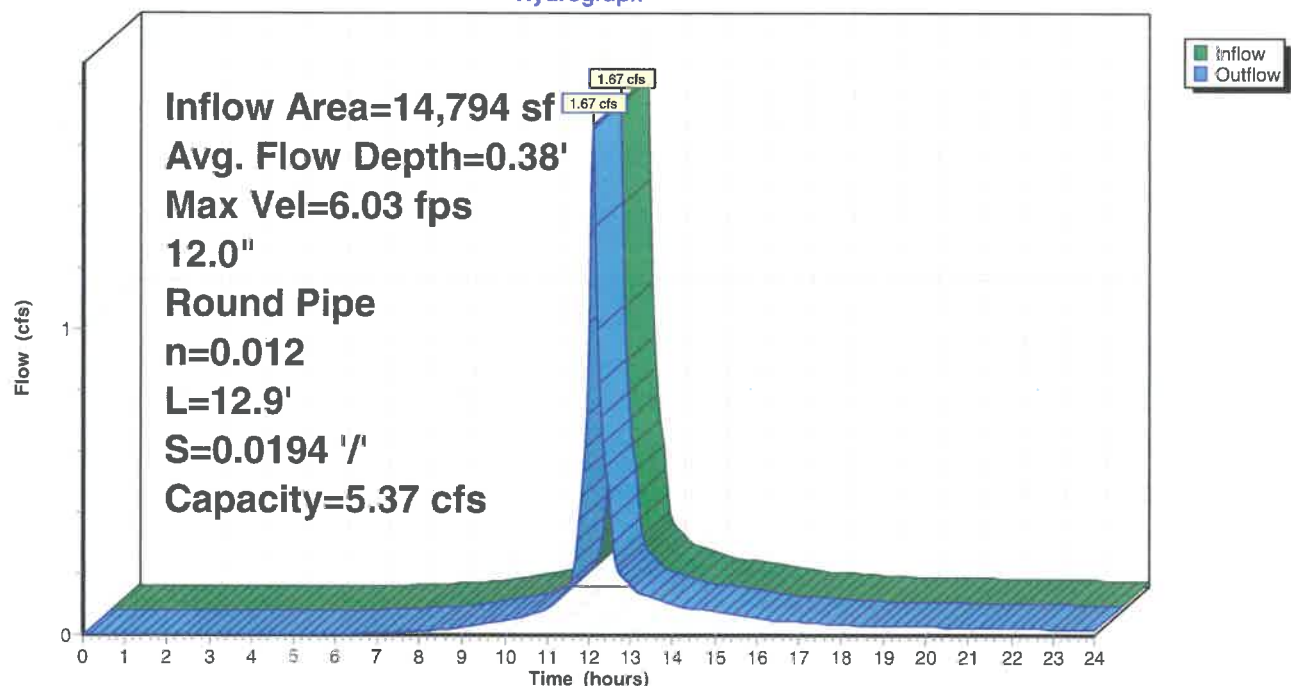
Peak Storage= 4 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.38', Surface Width= 0.97'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 12.9' Slope= 0.0194 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 18R: CB 10 TO DMH 6

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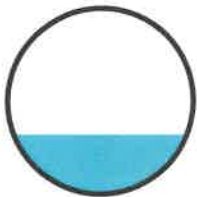
Summary for Reach 19R: CB 9 TO DMH 6

Inflow Area = 13,551 sf, 61.49% Impervious, Inflow Depth > 4.55" for 25 YR event
Inflow = 1.59 cfs @ 12.09 hrs, Volume= 5,138 cf
Outflow = 1.59 cfs @ 12.09 hrs, Volume= 5,138 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.42 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.52 fps, Avg. Travel Time= 0.0 min

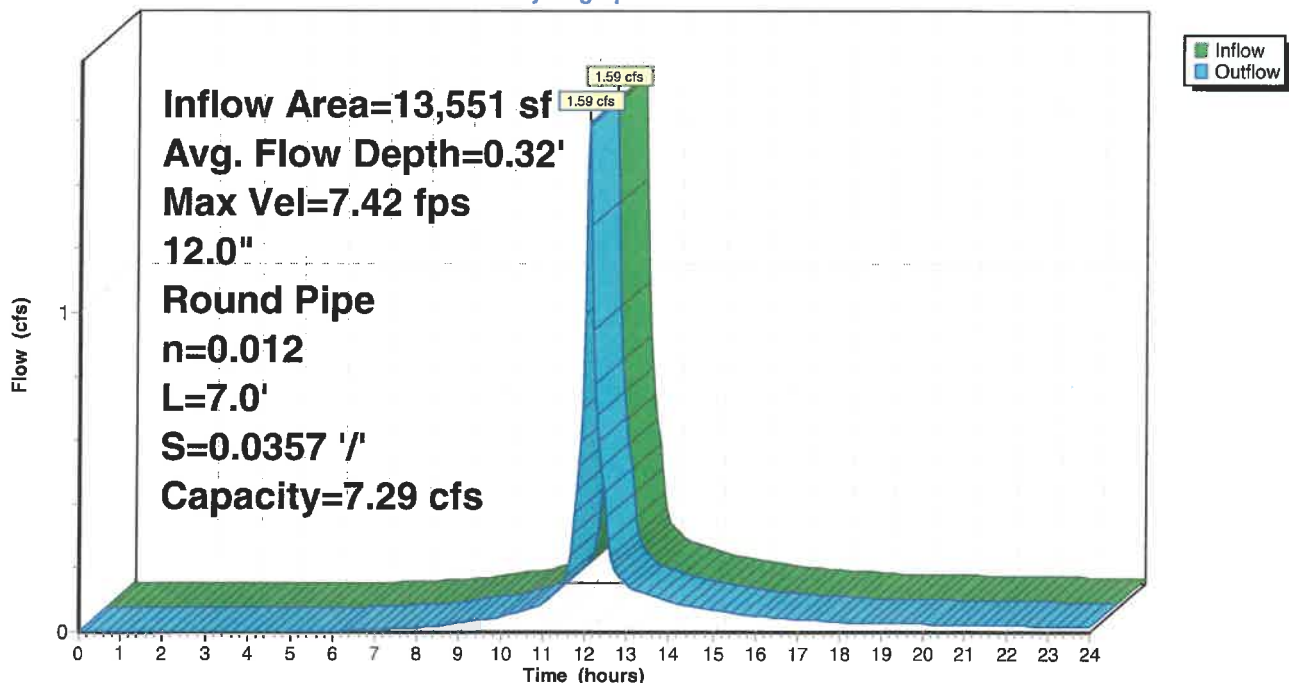
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.32', Surface Width= 0.93'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.29 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.0' Slope= 0.0357 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 19R: CB 9 TO DMH 6

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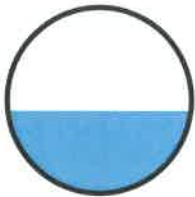
Summary for Reach 20R: DMH 6 to Basin

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 4.53" for 25 YR event
Inflow = 4.13 cfs @ 12.11 hrs, Volume= 13,895 cf
Outflow = 4.12 cfs @ 12.12 hrs, Volume= 13,894 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.88 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 2.64 fps, Avg. Travel Time= 0.2 min

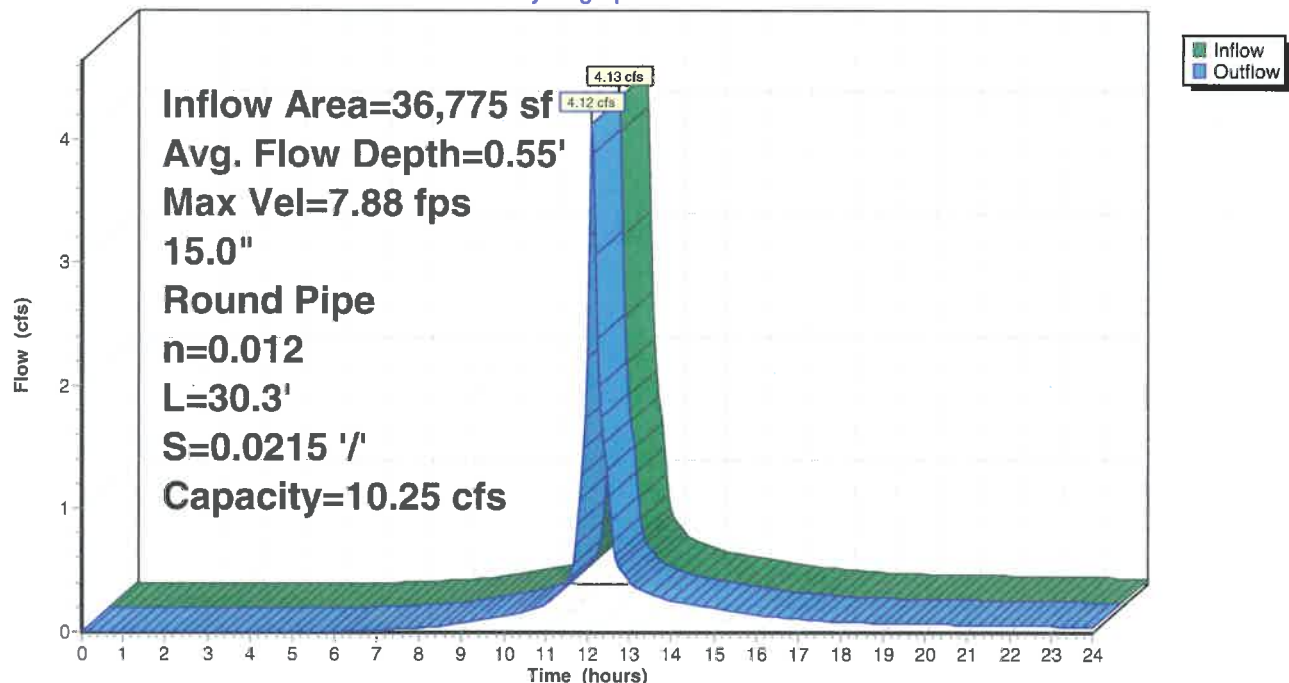
Peak Storage= 16 cf @ 12.12 hrs
Average Depth at Peak Storage= 0.55', Surface Width= 1.24'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 10.25 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 30.3' Slope= 0.0215 '/'
Inlet Invert= 305.65', Outlet Invert= 305.00'



Reach 20R: DMH 6 to Basin

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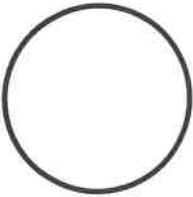
Summary for Reach 21R: 2-8" DI

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth = 0.00" for 25 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

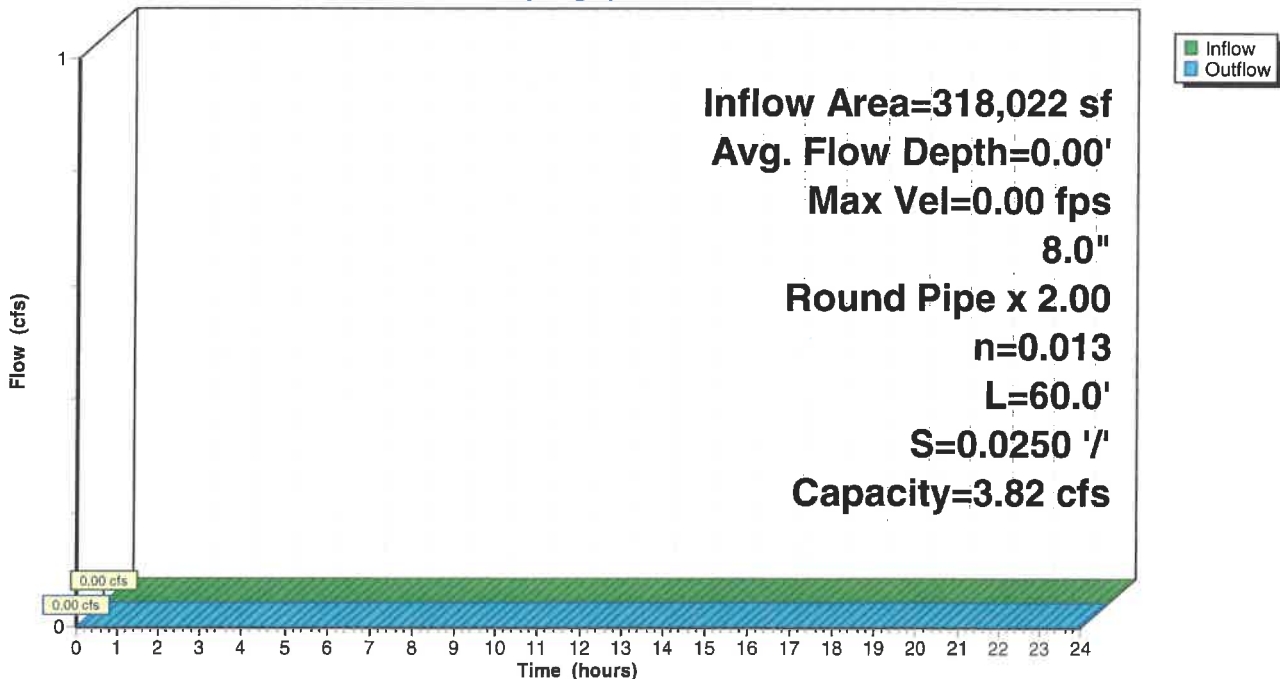
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.67' Flow Area= 0.7 sf, Capacity= 3.82 cfs

A factor of 2.00 has been applied to the storage and discharge capacity
8.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 60.0' Slope= 0.0250 '/'
Inlet Invert= 312.50', Outlet Invert= 311.00'



Reach 21R: 2-8" DI

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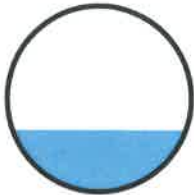
Summary for Reach 22R: Basin 2 Outfall

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 1.35" for 25 YR event
Inflow = 1.23 cfs @ 12.72 hrs, Volume= 13,101 cf
Outflow = 1.23 cfs @ 12.72 hrs, Volume= 13,100 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.42 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 3.14 fps, Avg. Travel Time= 0.3 min

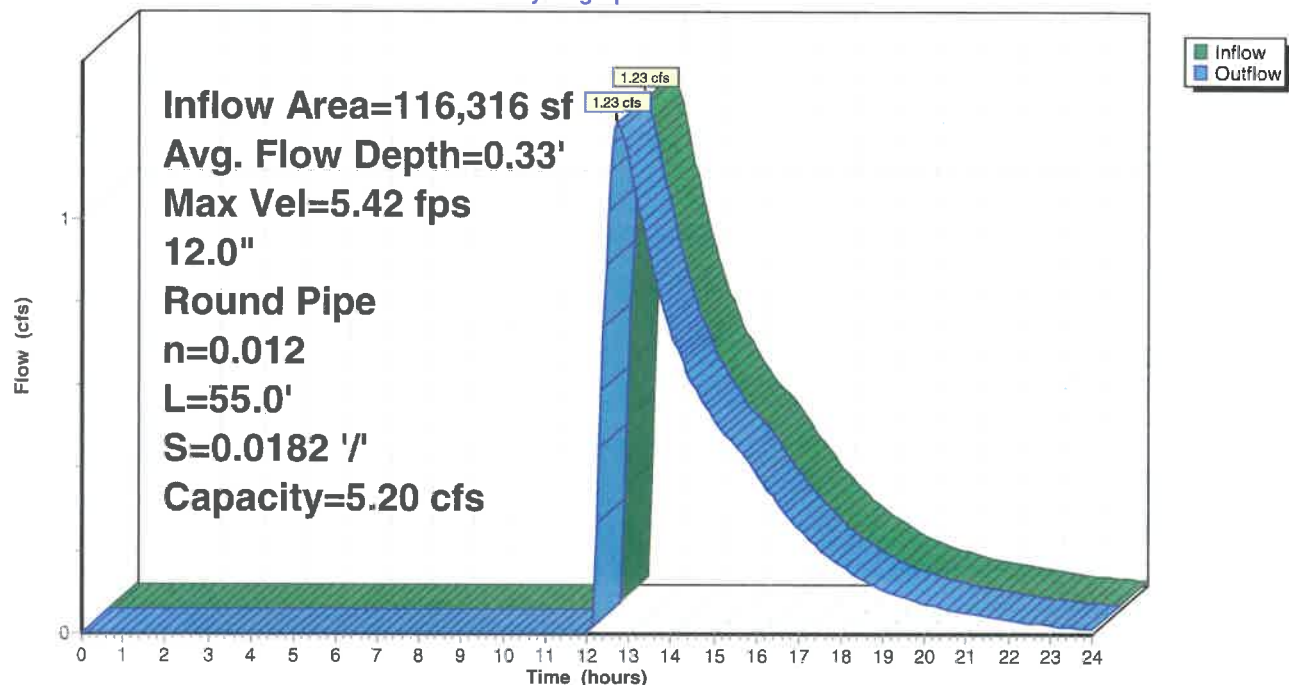
Peak Storage= 12 cf @ 12.72 hrs
Average Depth at Peak Storage= 0.33' , Surface Width= 0.94'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.20 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 55.0' Slope= 0.0182 '/'
Inlet Invert= 304.00', Outlet Invert= 303.00'



Reach 22R: Basin 2 Outfall

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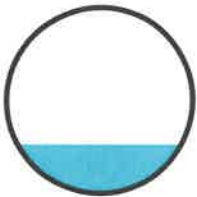
Summary for Reach 23R: Basin 1 Outfall

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 1.06" for 25 YR event
Inflow = 0.82 cfs @ 12.94 hrs, Volume= 10,997 cf
Outflow = 0.82 cfs @ 12.95 hrs, Volume= 10,996 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.01 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 3.27 fps, Avg. Travel Time= 0.3 min

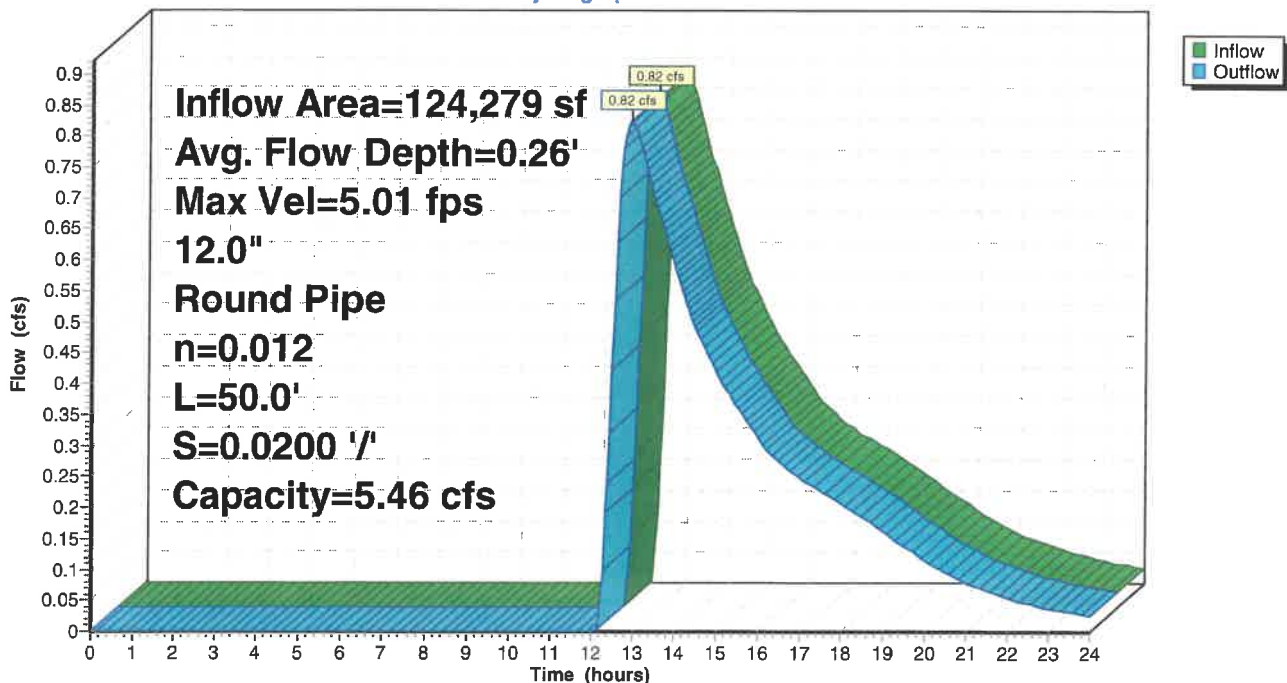
Peak Storage= 8 cf @ 12.95 hrs
Average Depth at Peak Storage= 0.26', Surface Width= 0.88'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.46 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 50.0' Slope= 0.0200 '/'
Inlet Invert= 281.00', Outlet Invert= 280.00'



Reach 23R: Basin 1 Outfall

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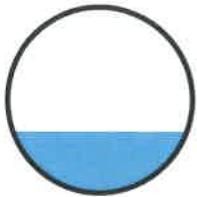
Summary for Reach 24R: Basin 3 Outfall

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 2.20" for 25 YR event
Inflow = 1.32 cfs @ 12.49 hrs, Volume= 10,195 cf
Outflow = 1.32 cfs @ 12.50 hrs, Volume= 10,194 cf, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.90 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 3.12 fps, Avg. Travel Time= 0.2 min

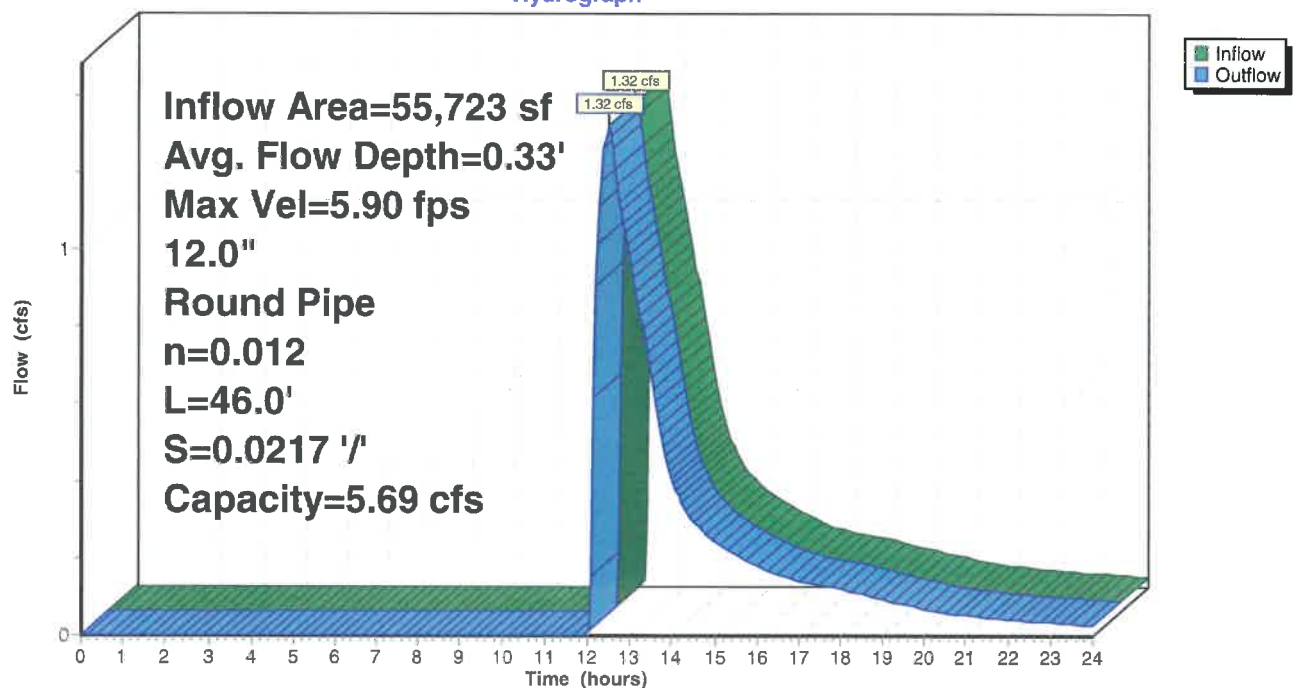
Peak Storage= 10 cf @ 12.50 hrs
Average Depth at Peak Storage= 0.33' , Surface Width= 0.94'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.69 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 46.0' Slope= 0.0217 '/'
Inlet Invert= 289.00', Outlet Invert= 288.00'



Reach 24R: Basin 3 Outfall

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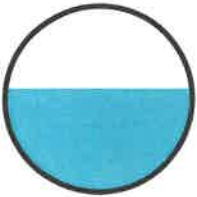
Summary for Reach 25R: 24" Bypass Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 2.25" for 25 YR event
Inflow = 16.87 cfs @ 12.42 hrs, Volume= 92,284 cf
Outflow = 16.86 cfs @ 12.42 hrs, Volume= 92,271 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 9.40 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 4.41 fps, Avg. Travel Time= 0.3 min

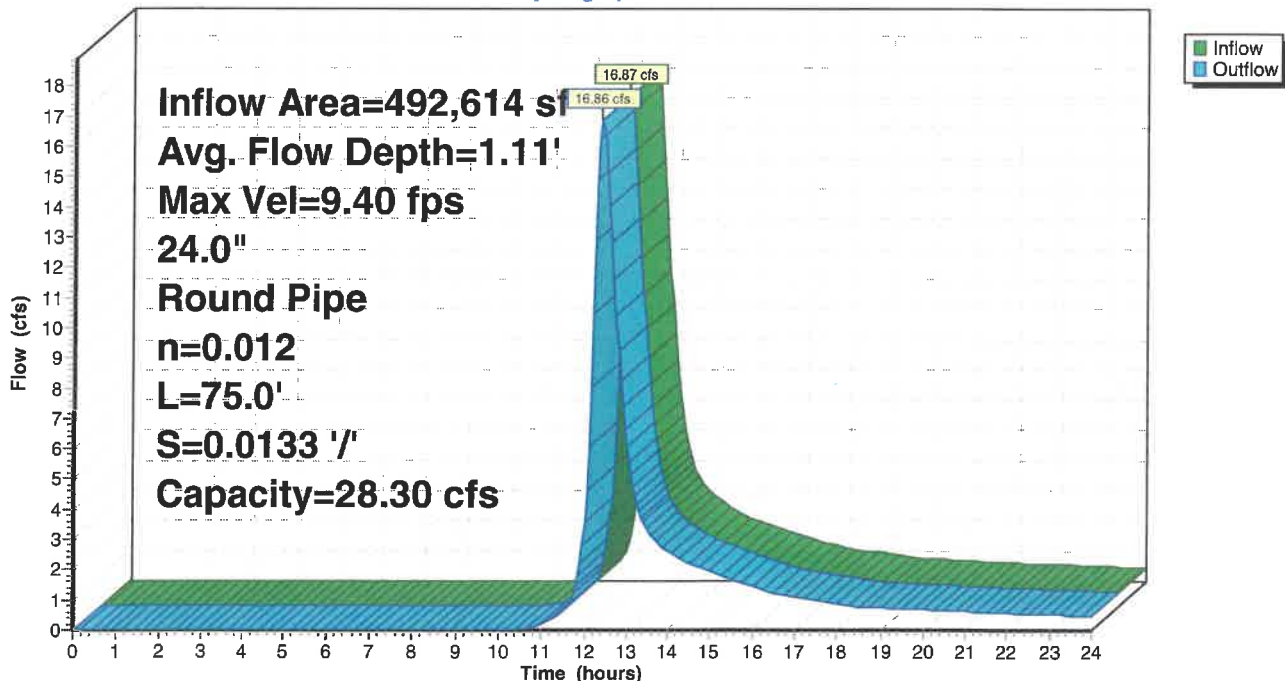
Peak Storage= 135 cf @ 12.42 hrs
Average Depth at Peak Storage= 1.11', Surface Width= 1.99'
Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 28.30 cfs

24.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 75.0' Slope= 0.0133 '/'
Inlet Invert= 292.50', Outlet Invert= 291.50'



Reach 25R: 24" Bypass Culvert

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Summary for Reach EV1: Road Berm

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 1.85" for 25 YR event
Inflow = 45.97 cfs @ 13.05 hrs, Volume= 425,771 cf
Outflow = 45.93 cfs @ 13.06 hrs, Volume= 425,751 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.17 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.16 fps, Avg. Travel Time= 0.1 min

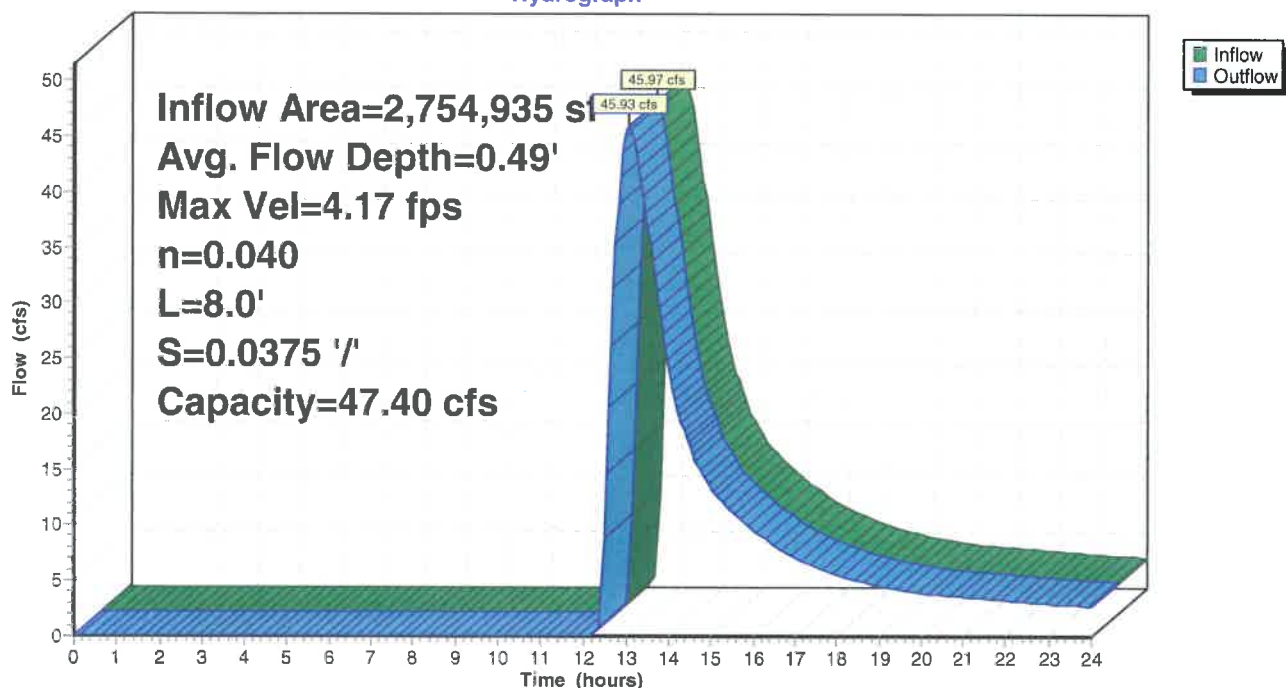
Peak Storage= 88 cf @ 13.06 hrs
Average Depth at Peak Storage= 0.49' , Surface Width= 24.91'
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides
Side Slope Z-value= 5.0 ' / ' Top Width= 25.00'
Length= 8.0' Slope= 0.0375 ' / '
Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: Road Berm

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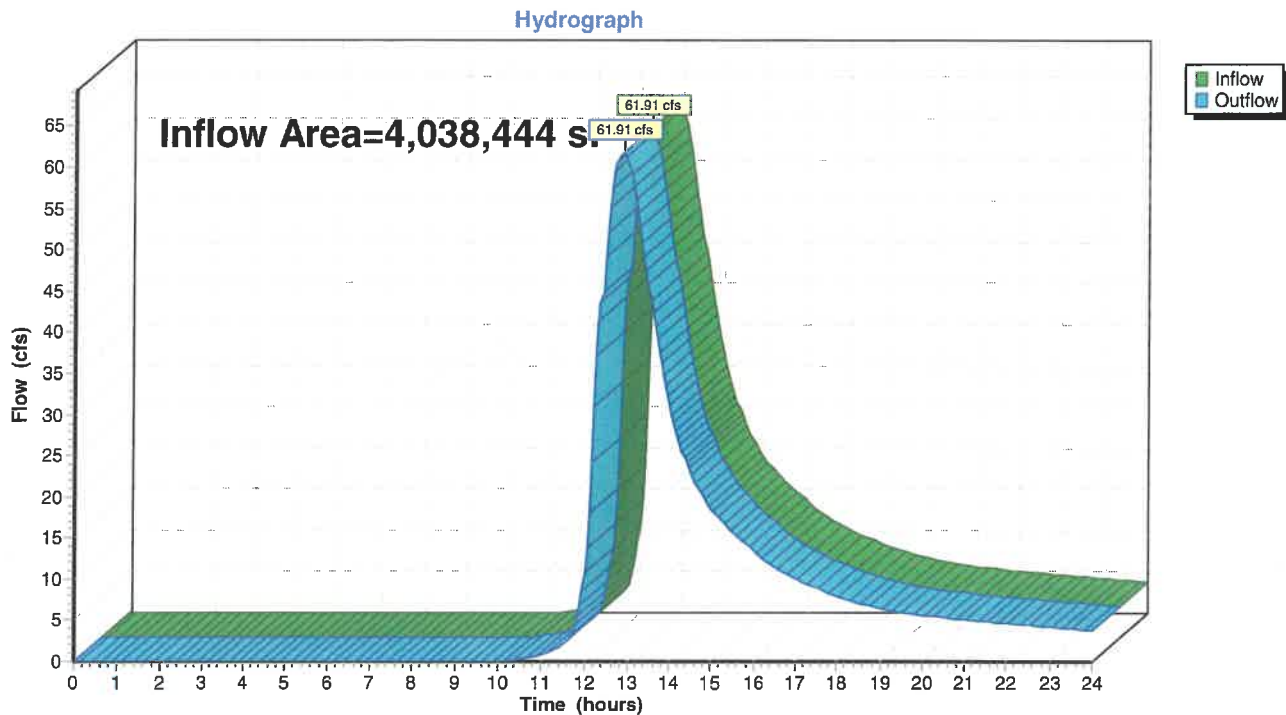
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Summary for Reach EV2: To Wetland

Inflow Area = 4,038,444 sf, 4.14% Impervious, Inflow Depth > 2.00" for 25 YR event
Inflow = 61.91 cfs @ 12.89 hrs, Volume= 672,383 cf
Outflow = 61.91 cfs @ 12.89 hrs, Volume= 672,383 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV2: To Wetland



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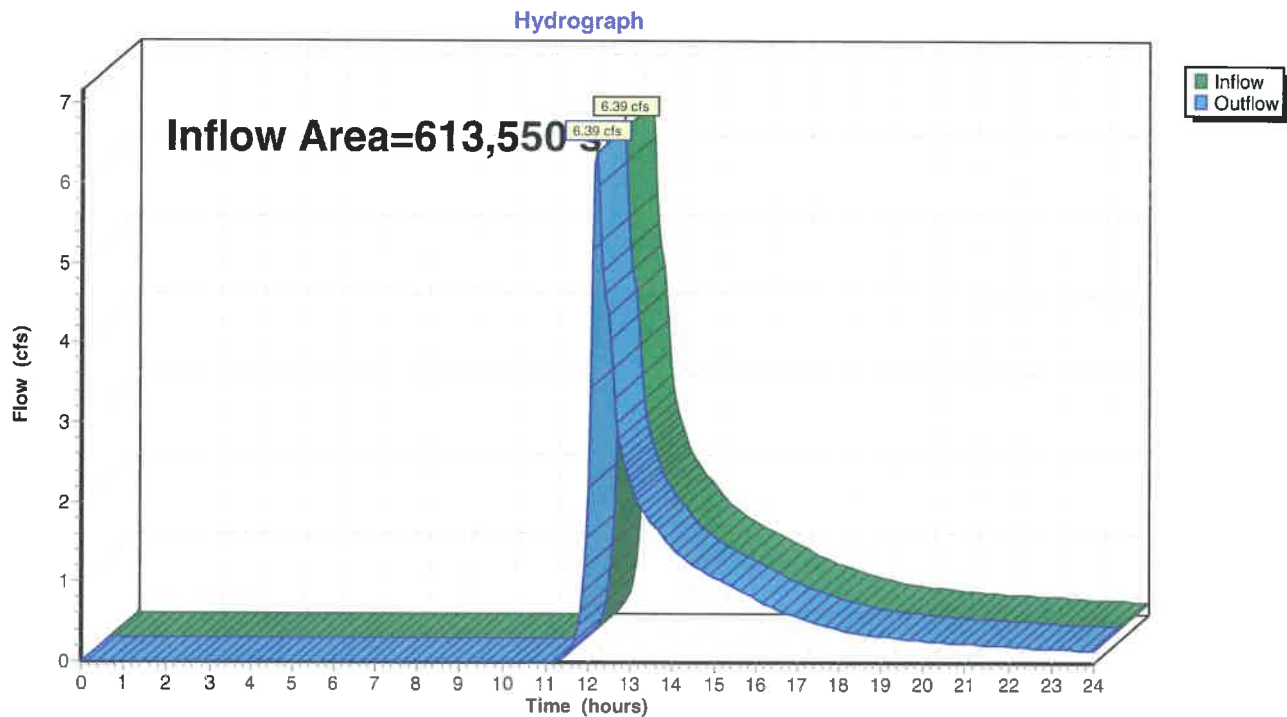
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Summary for Reach EV3: To Offsite

Inflow Area = 613,550 sf, 3.69% Impervious, Inflow Depth > 0.76" for 25 YR event
Inflow = 6.39 cfs @ 12.17 hrs, Volume= 39,028 cf
Outflow = 6.39 cfs @ 12.17 hrs, Volume= 39,028 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV3: To Offsite



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Summary for Pond 1P: Forebay

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 5.12" for 25 YR event
 Inflow = 1.53 cfs @ 12.13 hrs, Volume= 5,489 cf
 Outflow = 1.50 cfs @ 12.15 hrs, Volume= 3,999 cf, Atten= 2%, Lag= 1.0 min
 Primary = 1.50 cfs @ 12.15 hrs, Volume= 3,999 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 282.60' @ 12.15 hrs Surf.Area= 1,296 sf Storage= 1,605 cf

Plug-Flow detention time= 147.1 min calculated for 3,991 cf (73% of inflow)
 Center-of-Mass det. time= 61.0 min (848.7 - 787.7)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 281.00' | 3,838 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 281.00 | 737 | 101.0 | 0 | 0 | 737 |
| 282.00 | 1,071 | 120.0 | 899 | 899 | 1,089 |
| 284.00 | 1,908 | 158.0 | 2,939 | 3,838 | 1,974 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 282.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=1.49 cfs @ 12.15 hrs HW=282.60' (Free Discharge)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 1.49 cfs @ 0.76 fps)

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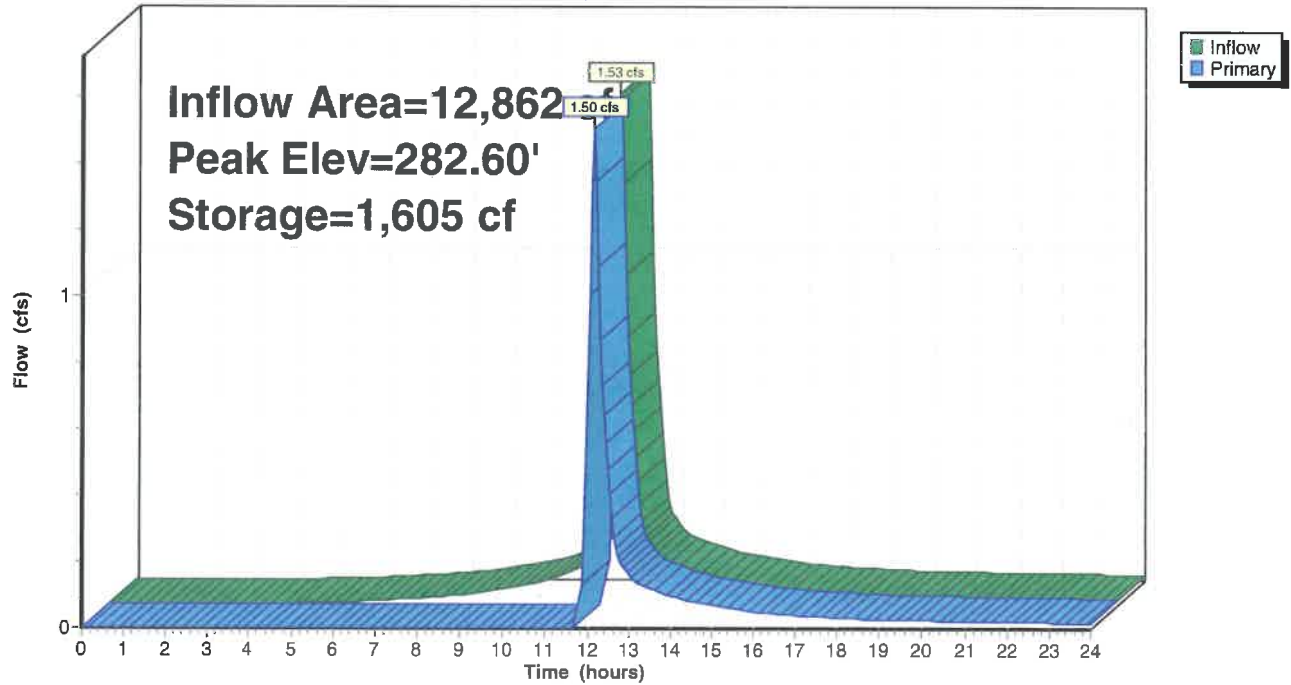
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Pond 1P: Forebay

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Summary for Pond 2P: Basin 1

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 2.02" for 25 YR event
 Inflow = 5.32 cfs @ 12.19 hrs, Volume= 20,894 cf
 Outflow = 0.96 cfs @ 12.94 hrs, Volume= 16,402 cf, Atten= 82%, Lag= 45.6 min
 Discarded = 0.13 cfs @ 12.94 hrs, Volume= 5,405 cf
 Primary = 0.82 cfs @ 12.94 hrs, Volume= 10,997 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 282.90' @ 12.94 hrs Surf.Area= 5,127 sf Storage= 8,345 cf

Plug-Flow detention time= 191.2 min calculated for 16,402 cf (79% of inflow)
 Center-of-Mass det. time= 106.8 min (976.3 - 869.5)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 281.00' | 51,257 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 281.00 | 3,708 | 232.0 | 0 | 0 | 3,708 |
| 282.00 | 4,433 | 251.0 | 4,065 | 4,065 | 4,477 |
| 284.00 | 6,052 | 288.0 | 10,443 | 14,508 | 6,153 |
| 286.00 | 7,890 | 326.0 | 13,901 | 28,410 | 8,109 |
| 288.00 | 9,968 | 364.0 | 17,818 | 46,227 | 10,306 |
| 288.50 | 10,153 | 374.0 | 5,030 | 51,257 | 10,922 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 287.50' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 281.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 281.00' | 12.0" Round Culvert L= 44.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 281.00' / 280.00' S= 0.0227 ' S= 0.0227 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 282.00' | 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 282.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 283.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 286.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.13 cfs @ 12.94 hrs HW=282.90' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.13 cfs)

Primary OutFlow Max=0.82 cfs @ 12.94 hrs HW=282.90' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 0.82 cfs of 3.94 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.36 cfs @ 4.11 fps)

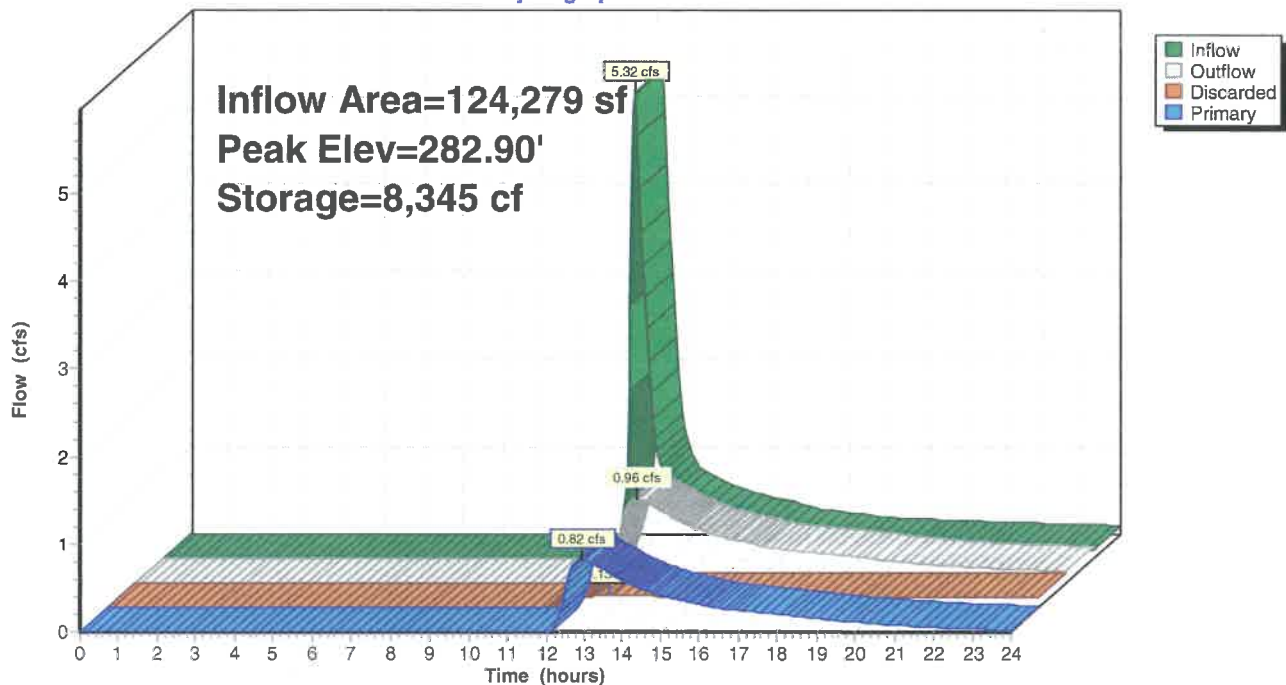
↳ **5=Orifice/Grate** (Orifice Controls 0.46 cfs @ 2.14 fps)

↳ **6=Orifice/Grate** (Controls 0.00 cfs)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 2P: Basin 1

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Summary for Pond 3P: Forebay

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 4.53" for 25 YR event
 Inflow = 4.12 cfs @ 12.12 hrs, Volume= 13,894 cf
 Outflow = 4.04 cfs @ 12.14 hrs, Volume= 11,680 cf, Atten= 2%, Lag= 1.3 min
 Primary = 4.04 cfs @ 12.14 hrs, Volume= 11,680 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.69' @ 12.14 hrs Surf.Area= 1,894 sf Storage= 2,555 cf

Plug-Flow detention time= 102.1 min calculated for 11,680 cf (84% of inflow)
 Center-of-Mass det. time= 36.5 min (839.5 - 802.9)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 10,058 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 1,160 | 129.0 | 0 | 0 | 1,160 |
| 306.00 | 2,048 | 166.0 | 3,166 | 3,166 | 2,077 |
| 308.00 | 3,164 | 204.0 | 5,172 | 8,338 | 3,256 |
| 308.50 | 3,725 | 227.0 | 1,720 | 10,058 | 4,052 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 305.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=3.93 cfs @ 12.14 hrs HW=305.69' (Free Discharge)

1=Broad-Crested Rectangular Weir (Weir Controls 3.93 cfs @ 1.05 fps)

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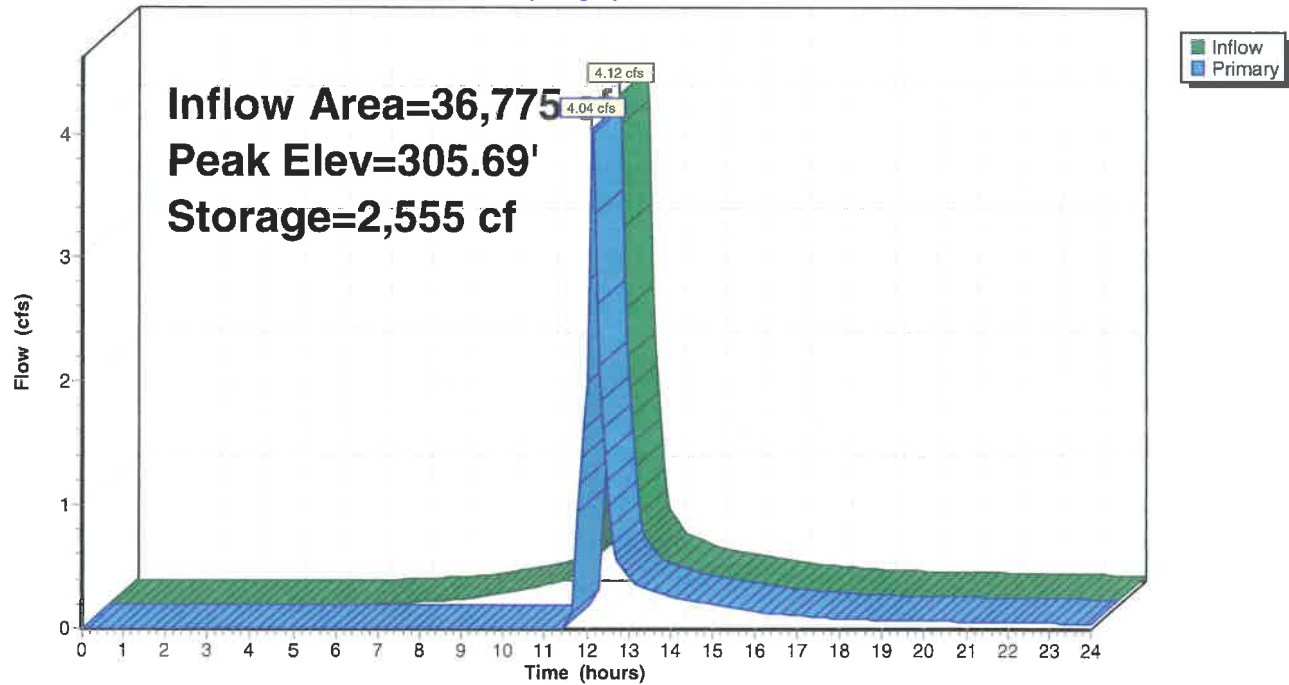
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Pond 3P: Forebay

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Summary for Pond 4P: Basin 2

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 2.57" for 25 YR event
 Inflow = 7.18 cfs @ 12.15 hrs, Volume= 24,886 cf
 Outflow = 1.39 cfs @ 12.72 hrs, Volume= 19,625 cf, Atten= 81%, Lag= 33.7 min
 Discarded = 0.16 cfs @ 12.72 hrs, Volume= 6,524 cf
 Primary = 1.23 cfs @ 12.72 hrs, Volume= 13,101 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.90' @ 12.72 hrs Surf.Area= 6,328 sf Storage= 10,237 cf

Plug-Flow detention time= 174.7 min calculated for 19,625 cf (79% of inflow)
 Center-of-Mass det. time= 93.2 min (947.8 - 854.7)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 30,313 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 4,504 | 302.0 | 0 | 0 | 4,504 |
| 306.00 | 6,434 | 340.0 | 10,881 | 10,881 | 6,549 |
| 308.00 | 8,590 | 378.0 | 14,972 | 25,853 | 8,836 |
| 308.50 | 9,256 | 397.0 | 4,460 | 30,313 | 10,023 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 307.70' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 304.00' | 1.100 in/hr Exfiltration over Surface area |
| #3 | Primary | 304.00' | 12.0" Round Culvert L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 304.00' / 303.00' S= 0.0286 ' S= 0.0286 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 305.00' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 305.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 306.00' | 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

Discarded OutFlow Max=0.16 cfs @ 12.72 hrs HW=305.90' (Free Discharge)

2=Exfiltration (Exfiltration Controls 0.16 cfs)

Primary OutFlow Max=1.23 cfs @ 12.72 hrs HW=305.90' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
 3=Culvert (Passes 1.23 cfs of 3.95 cfs potential flow)
 4=Orifice/Grate (Orifice Controls 0.76 cfs @ 3.88 fps)
 5=Orifice/Grate (Orifice Controls 0.47 cfs @ 2.15 fps)
 6=Orifice/Grate (Controls 0.00 cfs)

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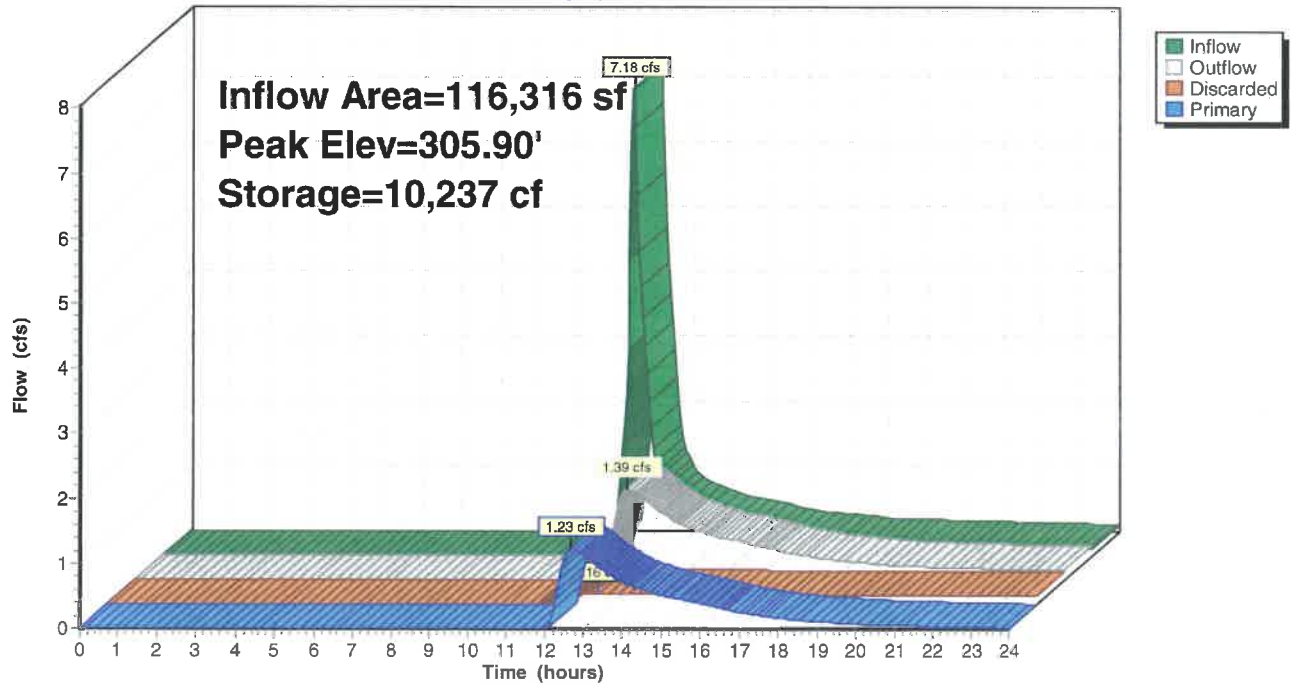
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Pond 4P: Basin 2

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Summary for Pond 5P: Forebay

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 5.65" for 25 YR event
 Inflow = 3.30 cfs @ 12.09 hrs, Volume= 11,501 cf
 Outflow = 3.14 cfs @ 12.11 hrs, Volume= 7,641 cf, Atten= 5%, Lag= 1.3 min
 Primary = 3.14 cfs @ 12.11 hrs, Volume= 7,641 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 291.36' @ 12.12 hrs Surf.Area= 2,335 sf Storage= 4,216 cf

Plug-Flow detention time= 178.4 min calculated for 7,625 cf (66% of inflow)
 Center-of-Mass det. time= 82.7 min (849.3 - 766.6)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 289.00' | 5,821 cf | Forebay (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 289.00 | 1,270 | 132.0 | 0 | 0 | 1,270 |
| 290.00 | 1,710 | 153.0 | 1,485 | 1,485 | 1,767 |
| 291.00 | 2,145 | 172.0 | 1,923 | 3,408 | 2,285 |
| 292.00 | 2,692 | 191.0 | 2,413 | 5,821 | 2,863 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 291.20' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=3.04 cfs @ 12.11 hrs HW=291.36' (Free Discharge)

1=Broad-Crested Rectangular Weir (Weir Controls 3.04 cfs @ 0.96 fps)

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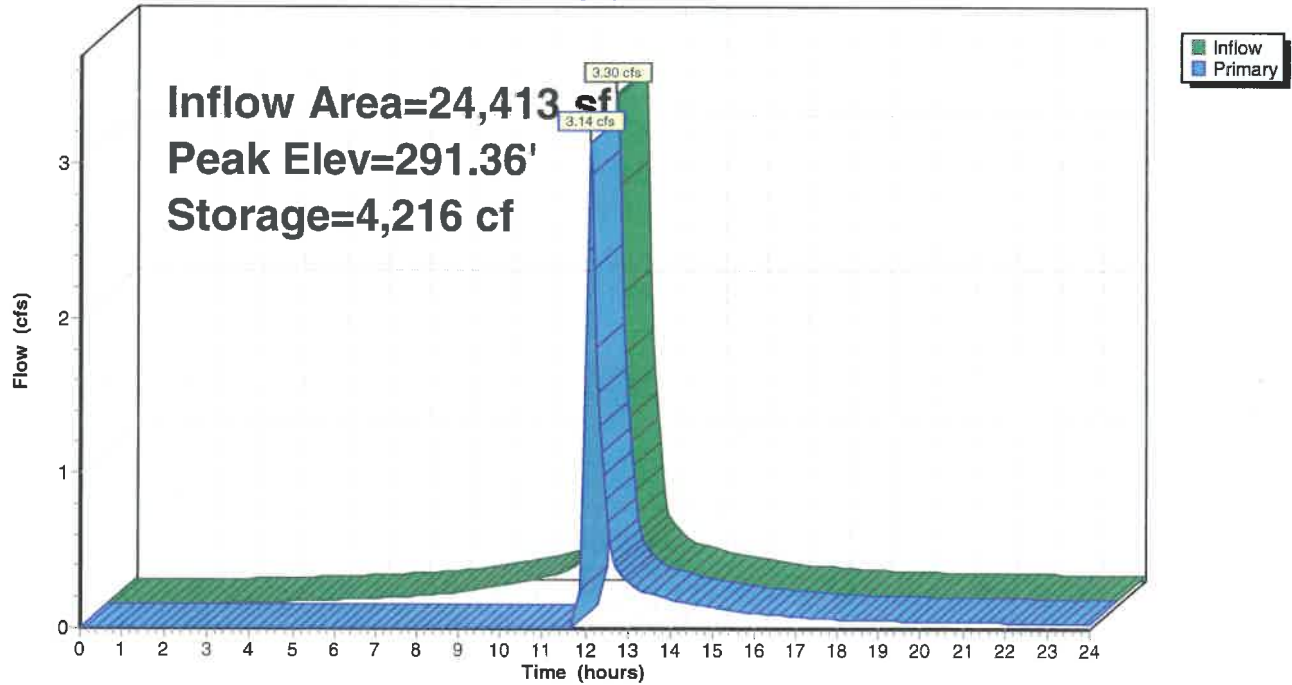
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Pond 5P: Forebay

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Summary for Pond 6P: Basin 3

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 2.92" for 25 YR event
 Inflow = 4.93 cfs @ 12.11 hrs, Volume= 13,541 cf
 Outflow = 1.38 cfs @ 12.49 hrs, Volume= 12,252 cf, Atten= 72%, Lag= 23.1 min
 Discarded = 0.07 cfs @ 12.49 hrs, Volume= 2,058 cf
 Primary = 1.32 cfs @ 12.49 hrs, Volume= 10,195 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 291.68' @ 12.49 hrs Surf.Area= 2,528 sf Storage= 4,411 cf

Plug-Flow detention time= 98.0 min calculated for 12,227 cf (90% of inflow)
 Center-of-Mass det. time= 52.5 min (904.6 - 852.1)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 289.00' | 12,306 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 289.00 | 840 | 185.0 | 0 | 0 | 840 |
| 290.00 | 1,423 | 204.0 | 1,119 | 1,119 | 1,459 |
| 291.00 | 2,065 | 223.0 | 1,734 | 2,853 | 2,139 |
| 292.00 | 2,763 | 242.0 | 2,406 | 5,258 | 2,880 |
| 293.00 | 3,517 | 260.0 | 3,132 | 8,391 | 3,642 |
| 294.00 | 4,328 | 279.0 | 3,915 | 12,306 | 4,500 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 293.25' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 289.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 289.00' | 12.0" Round Culvert L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 289.00' / 288.00' S= 0.0222 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 290.00' | 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 290.50' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 291.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 292.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.07 cfs @ 12.49 hrs HW=291.68' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=1.31 cfs @ 12.49 hrs HW=291.68' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 1.31 cfs of 4.93 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.29 cfs @ 6.00 fps)

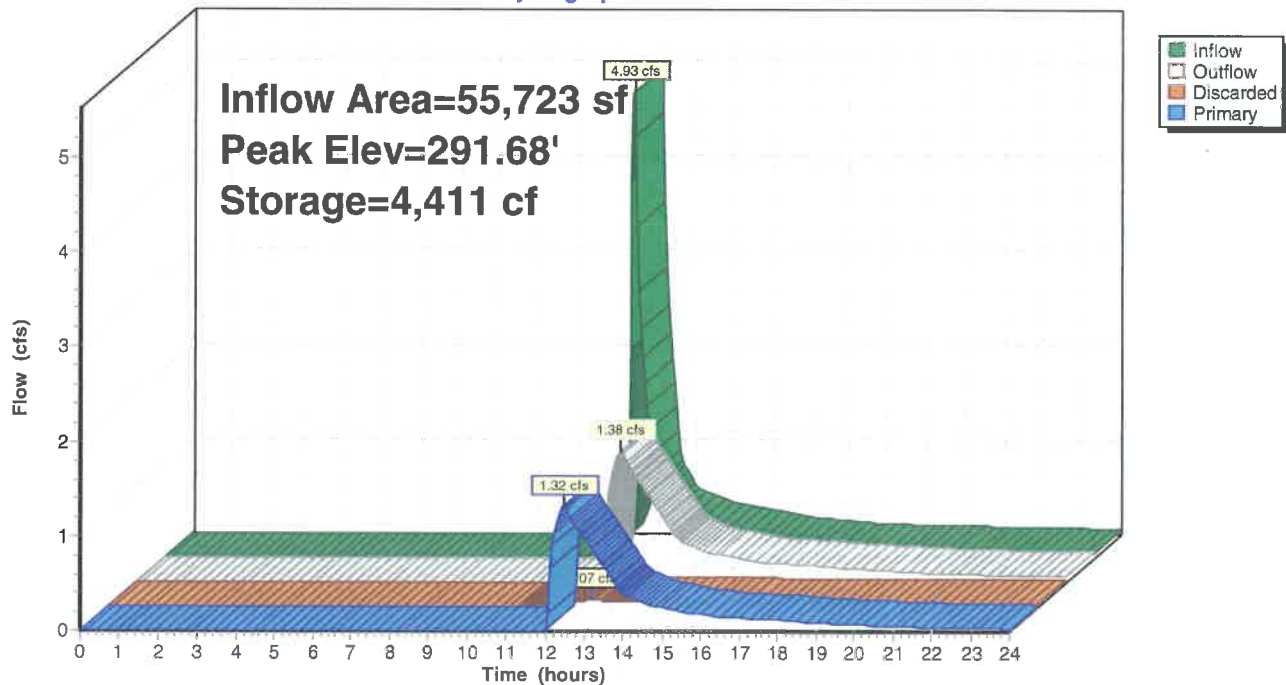
↳ **5=Orifice/Grate** (Orifice Controls 0.91 cfs @ 4.64 fps)

↳ **6=Orifice/Grate** (Orifice Controls 0.11 cfs @ 1.44 fps)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 6P: Basin 3

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Summary for Pond 7P: Vernal Pool and Wetland

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 2.06" for 25 YR event
 Inflow = 66.94 cfs @ 12.68 hrs, Volume= 472,904 cf
 Outflow = 45.97 cfs @ 13.05 hrs, Volume= 425,771 cf, Atten= 31%, Lag= 22.6 min
 Primary = 45.97 cfs @ 13.05 hrs, Volume= 425,771 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 295.48' @ 13.05 hrs Surf.Area= 58,432 sf Storage= 110,347 cf

Plug-Flow detention time= 82.7 min calculated for 425,771 cf (90% of inflow)
 Center-of-Mass det. time= 36.9 min (928.3 - 891.4)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 291.30' | 143,915 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=45.95 cfs @ 13.05 hrs HW=295.48' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 45.95 cfs @ 3.42 fps)

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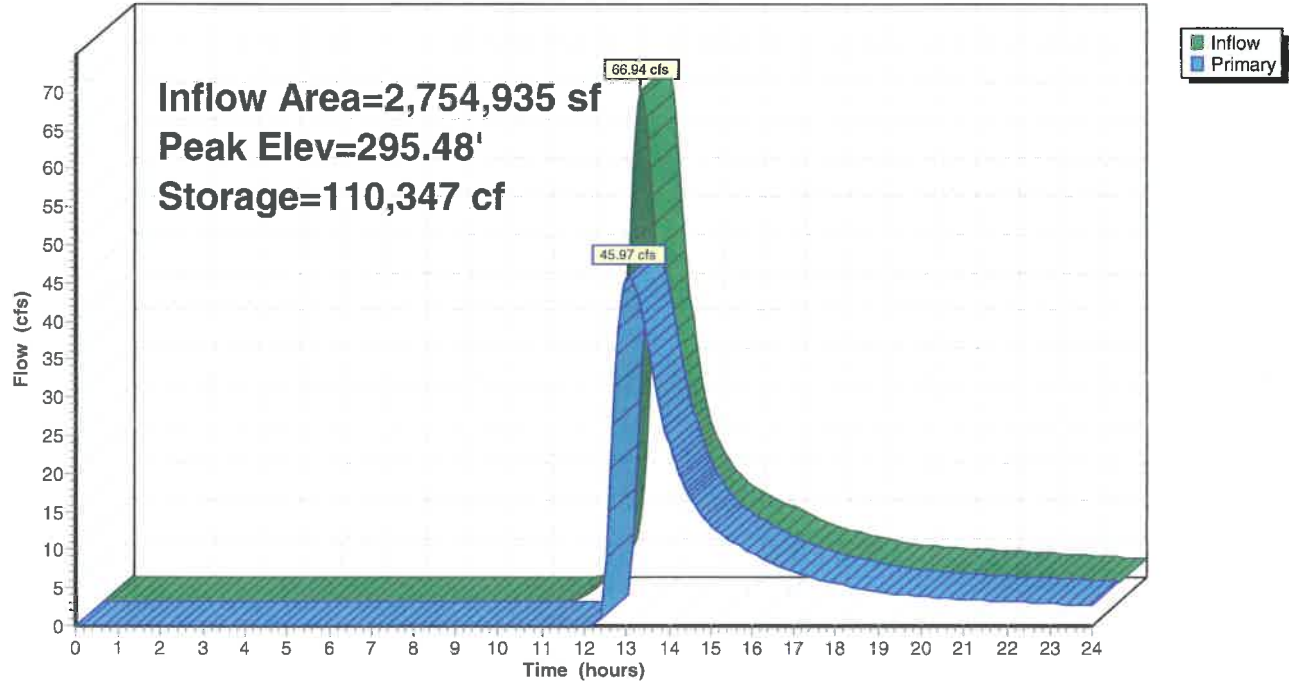
00454 - PROPOSED Conditions
Type III 24-hr 25 YR Rainfall=6.38"

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Pond 7P: Vernal Pool and Wetland

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Summary for Pond 8P: Road Berm

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth > 1.73" for 25 YR event
 Inflow = 8.70 cfs @ 12.34 hrs, Volume= 45,859 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 311.47' @ 24.00 hrs Surf.Area= 51,426 sf Storage= 45,832 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)

Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 310.00' | 256,496 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 312.50' | 8.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)

1=Orifice/Grate (Controls 0.00 cfs)

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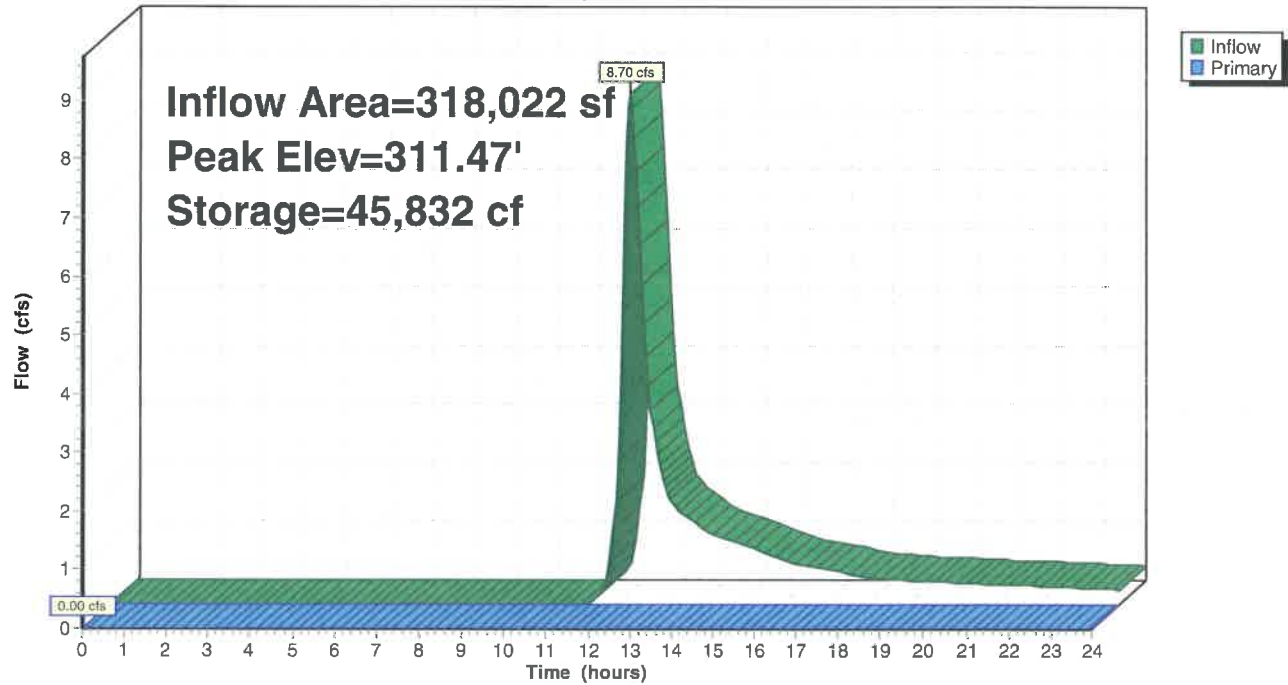
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Pond 8P: Road Berm

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Summary for Subcatchment 1S: To Culvert

Runoff = 30.99 cfs @ 12.40 hrs, Volume= 164,278 cf, Depth> 4.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 34,861 | 98 | Paved roads w/curbs & sewers, HSG B |
| 14,250 | 98 | Roofs, HSG B |
| 288,899 | 55 | Woods, Good, HSG B |
| 154,604 | 61 | >75% Grass cover, Good, HSG B |
| 492,614 | 61 | Weighted Average |
| 443,503 | | 90.03% Pervious Area |
| 49,111 | | 9.97% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|--|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.4 | 126 | 0.0317 | 0.89 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 10.1 | 880 | 0.0432 | 1.45 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 5.3 | 568 | 0.0140 | 1.77 | | Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps |
| 27.6 | 1,624 | Total | | | |

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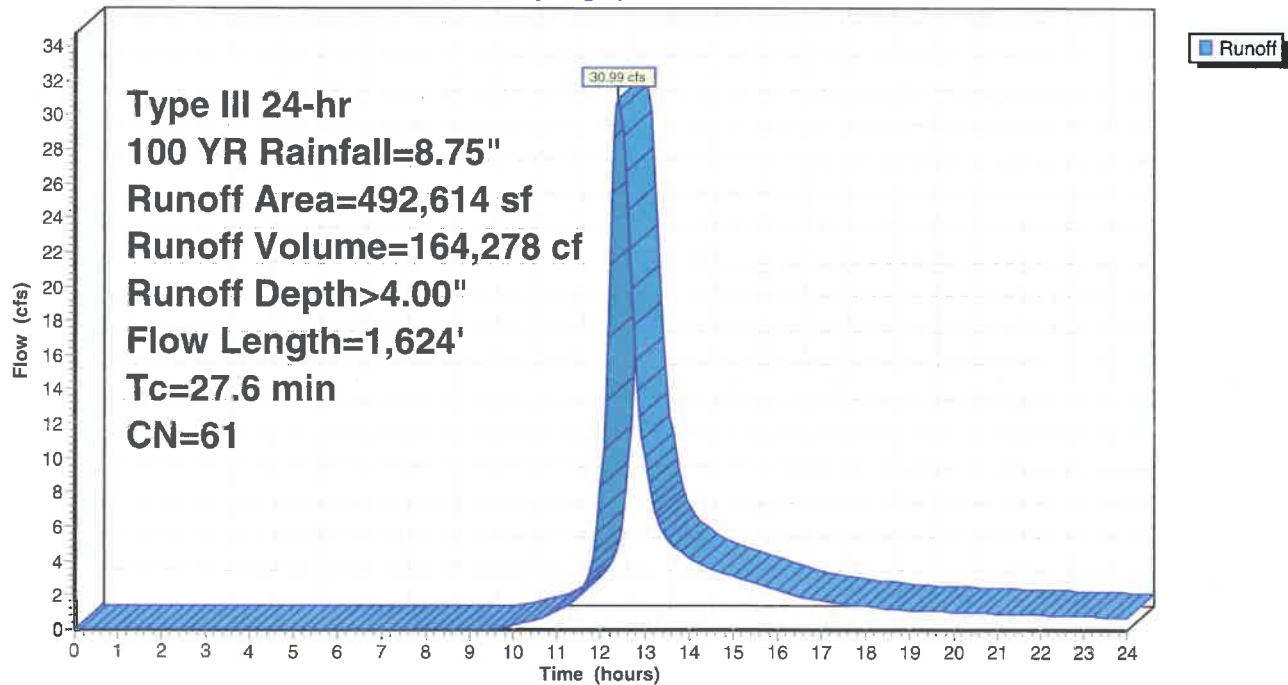
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Subcatchment 1S: To Culvert

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Summary for Subcatchment 2S: To Proposed Culvert

Runoff = 126.66 cfs @ 12.65 hrs, Volume= 859,736 cf, Depth> 3.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 57,874 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,163,489 | 55 | Woods, Good, HSG B |
| 236,877 | 77 | Woods, Good, HSG D |
| 25,616 | 98 | Roofs, HSG B |
| 3,202 | 98 | Roofs, HSG D |
| 20,077 | 80 | >75% Grass cover, Good, HSG D |
| 239,497 | 61 | >75% Grass cover, Good, HSG B |
| 882 | 96 | Gravel surface, HSG B |
| 7,421 | 96 | Gravel surface, HSG D |
| 2,754,935 | 59 | Weighted Average |
| 2,668,243 | | 96.85% Pervious Area |
| 86,692 | | 3.15% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 3.5 | 366 | 0.1202 | 1.73 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 32.5 | 2,172 | 0.0115 | 1.12 | 1.34 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 45.8 | 2,588 | Total | | | |

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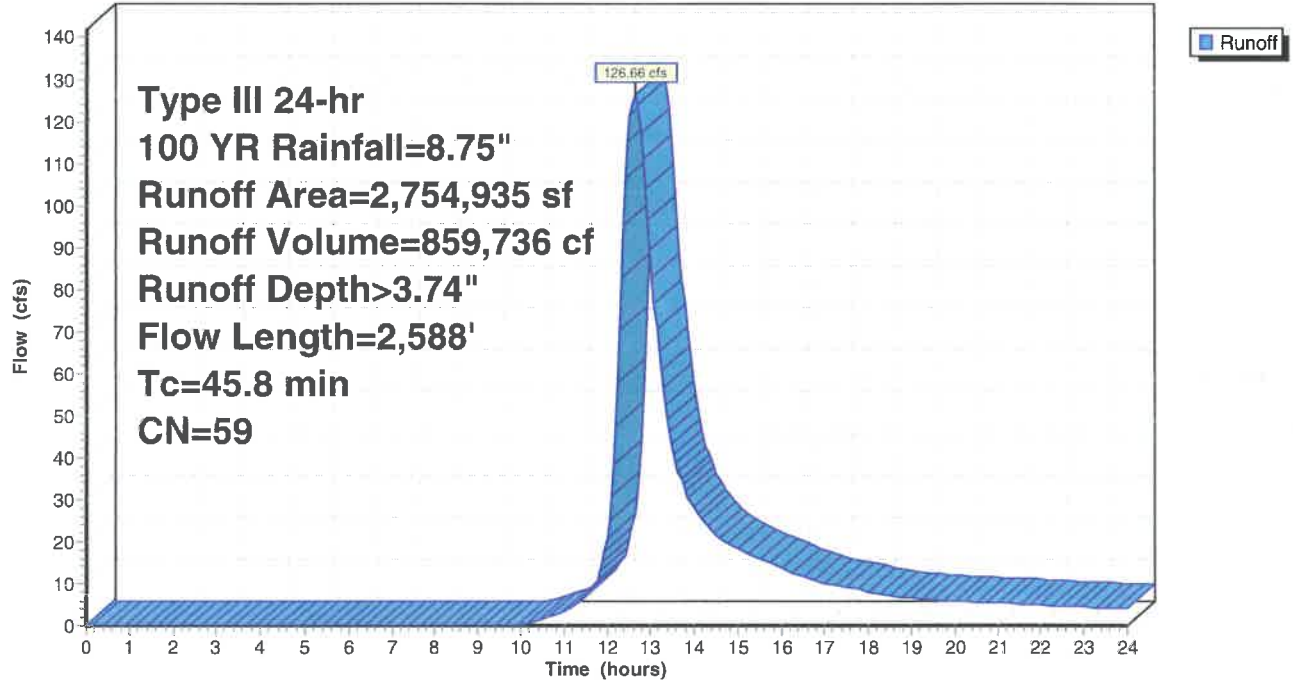
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Type III 24-hr 100 YR Rainfall=8.75"

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Subcatchment 2S: To Proposed Culvert

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Summary for Subcatchment 3S: To 8" pipes

Runoff = 17.73 cfs @ 12.32 hrs, Volume= 87,225 cf, Depth> 3.29"

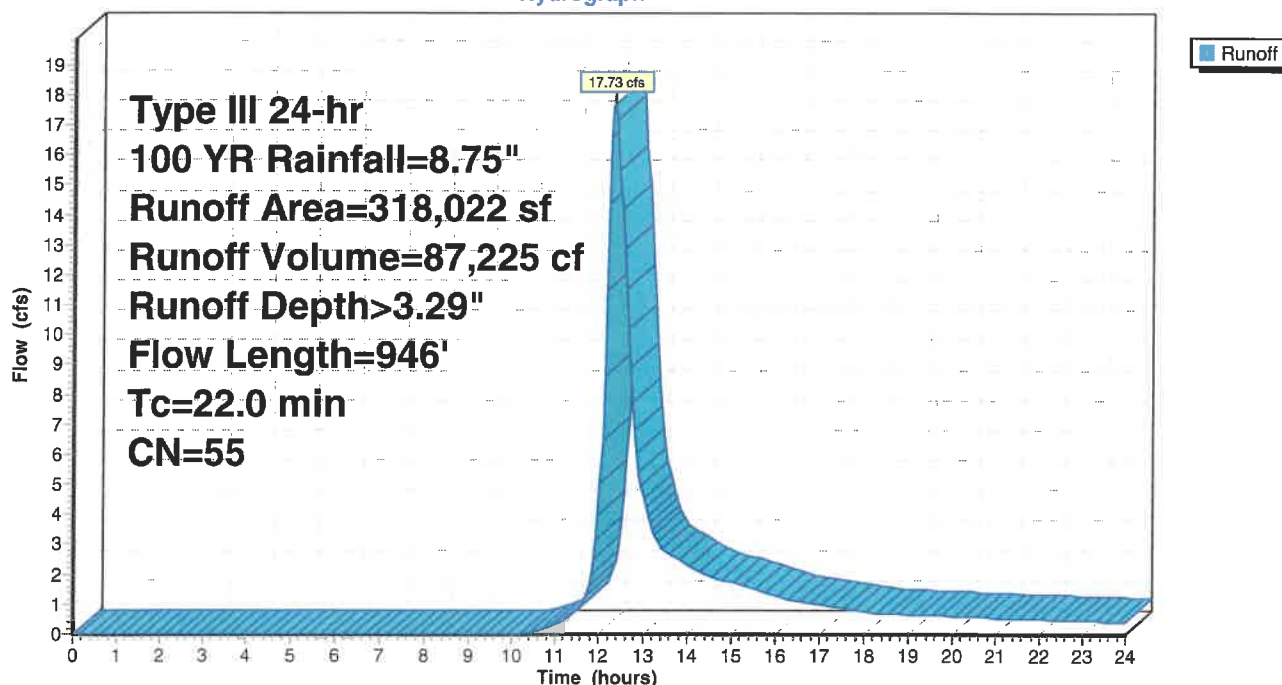
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 318,022 | 55 | Woods, Good, HSG B |
| 318,022 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|-------------|------------------|------------------|----------------------|-------------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.22" |
| 14.9 | 896 | 0.0401 | 1.00 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 22.0 | 946 | Total | | | |

Subcatchment 3S: To 8" pipes

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Summary for Subcatchment 4S: To CB 1

Runoff = 2.45 cfs @ 12.09 hrs, Volume= 8,491 cf, Depth> 7.78"

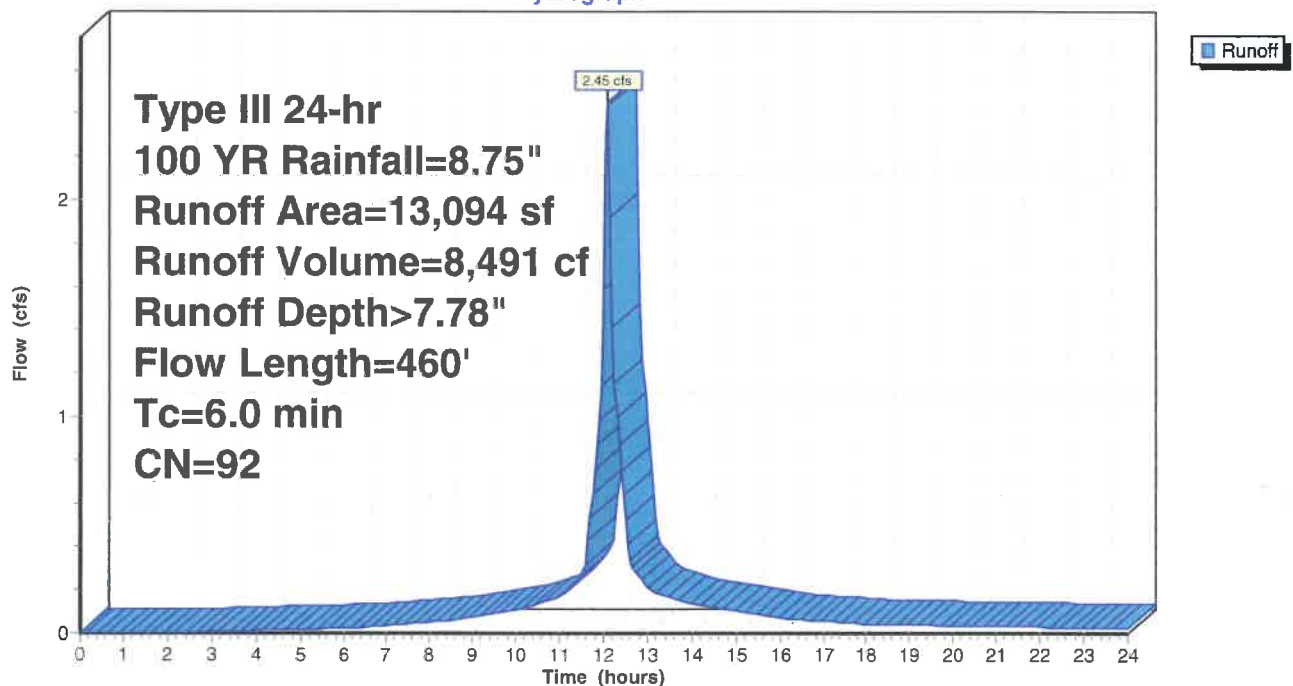
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 11,086 | 98 | Paved roads w/curbs & sewers, HSG B |
| 2,008 | 61 | >75% Grass cover, Good, HSG B |
| 13,094 | 92 | Weighted Average |
| 2,008 | | 15.34% Pervious Area |
| 11,086 | | 84.66% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 4S: To CB 1

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Summary for Subcatchment 5S: To CB 2

Runoff = 2.17 cfs @ 12.09 hrs, Volume= 7,795 cf, Depth> 8.26"

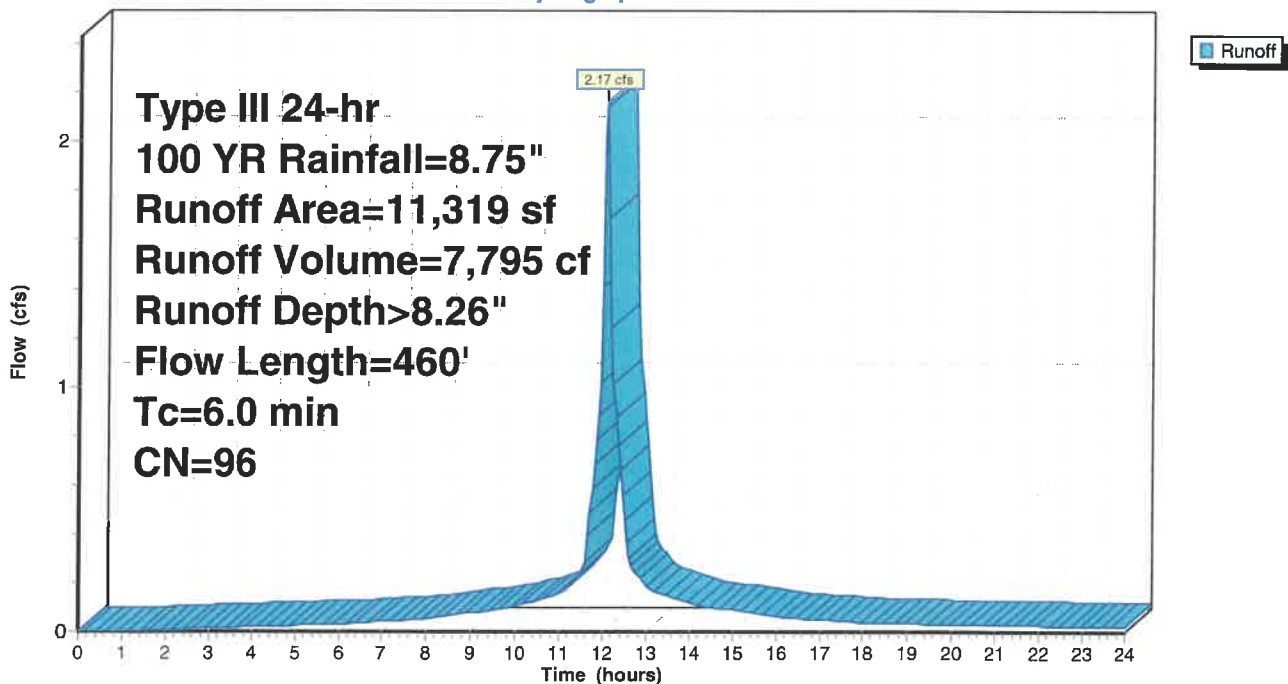
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 10,719 | 98 | Paved roads w/curbs & sewers, HSG B |
| 600 | 61 | >75% Grass cover, Good, HSG B |
| 11,319 | 96 | Weighted Average |
| 600 | | 5.30% Pervious Area |
| 10,719 | | 94.70% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.6 | 50 | 0.0240 | 1.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 3.3 | 410 | 0.0102 | 2.05 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.9 | 460 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 5S: To CB 2

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Summary for Subcatchment 6S: To CB 3

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 1,931 cf, Depth> 7.42"

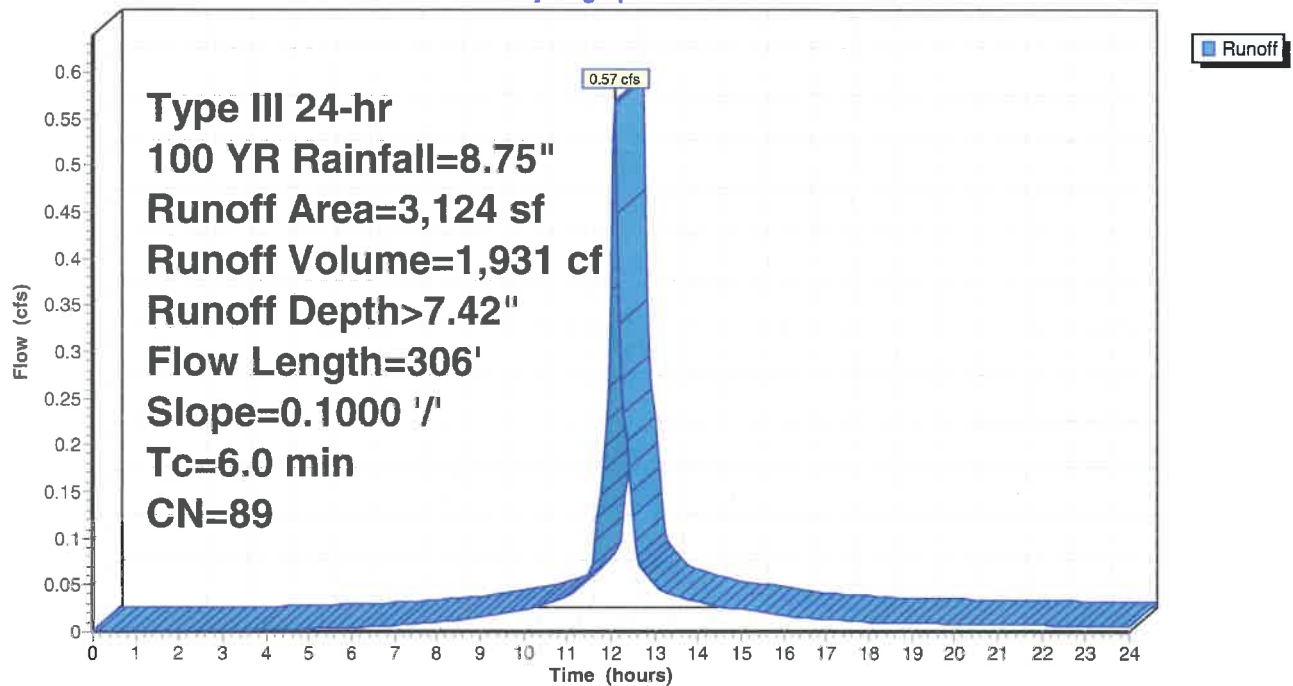
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,343 | 98 | Paved roads w/curbs & sewers, HSG B |
| 781 | 61 | >75% Grass cover, Good, HSG B |
| 3,124 | 89 | Weighted Average |
| 781 | | 25.00% Pervious Area |
| 2,343 | | 75.00% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 6S: To CB 3

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Summary for Subcatchment 7S: To CB 4

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 1,576 cf, Depth> 7.42"

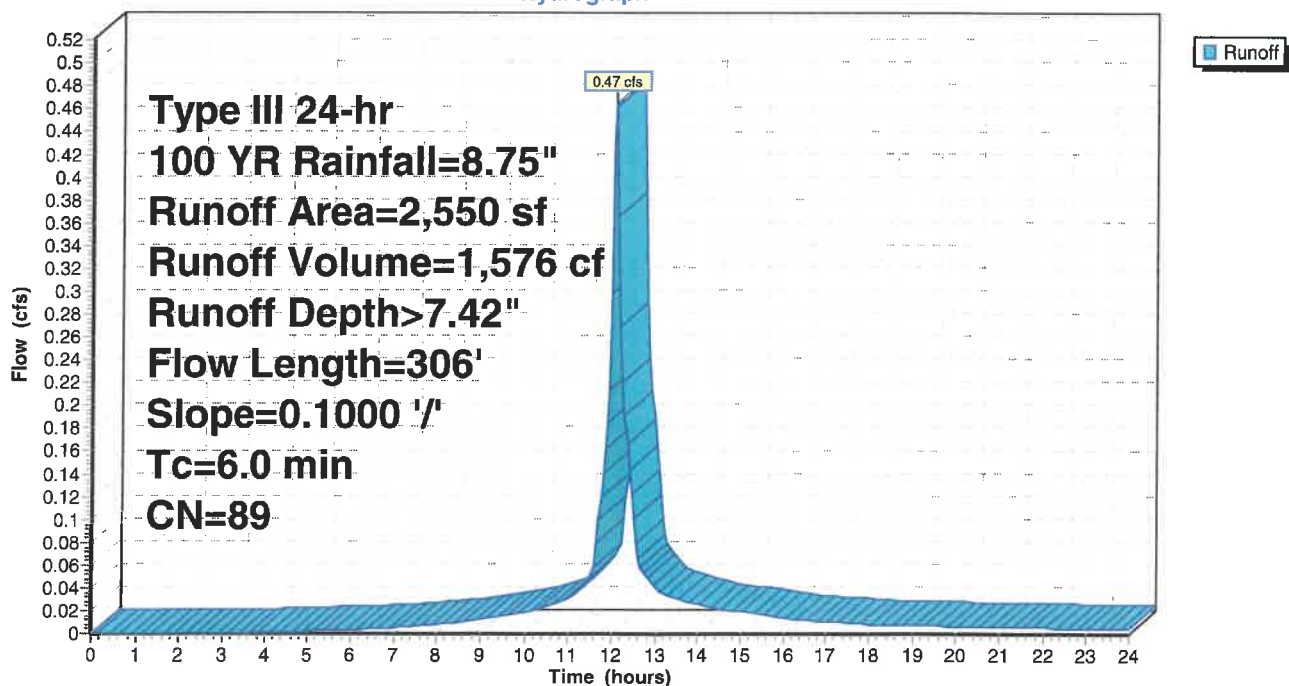
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 1,913 | 98 | Paved roads w/curbs & sewers, HSG B |
| 637 | 61 | >75% Grass cover, Good, HSG B |
| 2,550 | 89 | Weighted Average |
| 637 | | 24.98% Pervious Area |
| 1,913 | | 75.02% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 256 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 306 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 7S: To CB 4

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Summary for Subcatchment 8S: To CB 6

Runoff = 0.65 cfs @ 12.09 hrs, Volume= 2,171 cf, Depth> 7.06"

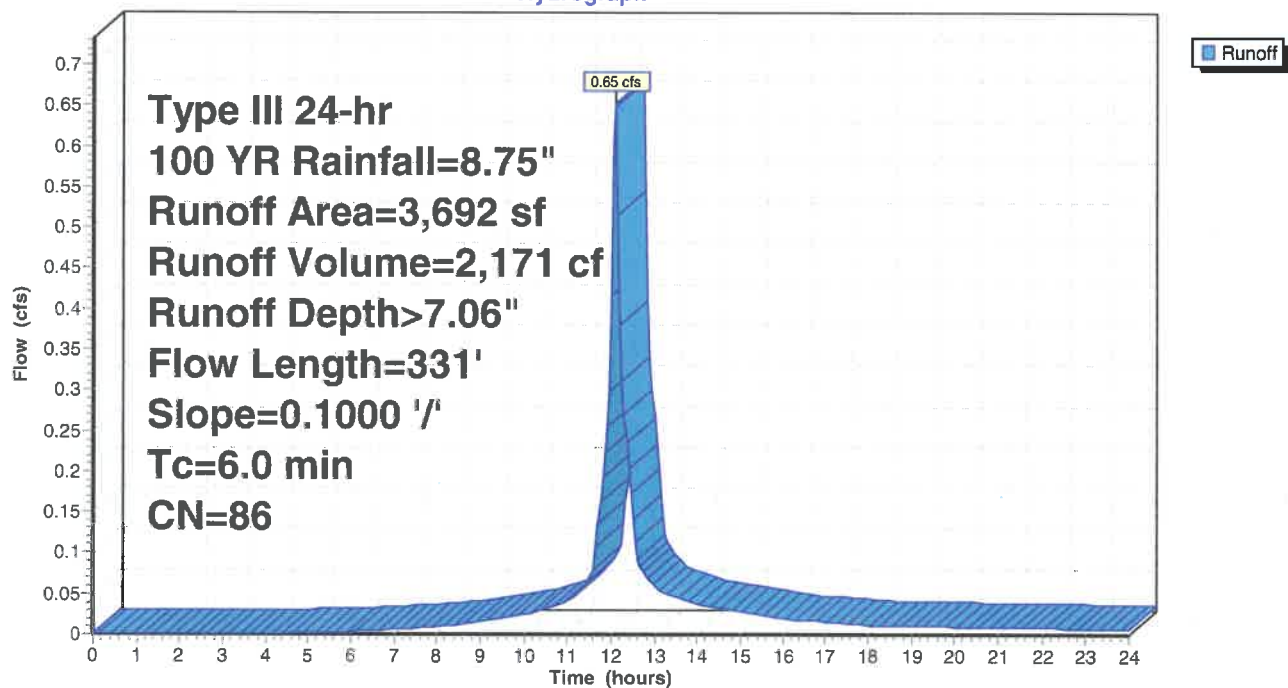
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,493 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,199 | 61 | >75% Grass cover, Good, HSG B |
| 3,692 | 86 | Weighted Average |
| 1,199 | | 32.48% Pervious Area |
| 2,493 | | 67.52% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 281 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 331 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 8S: To CB 6

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Summary for Subcatchment 9S: To CB 5

Runoff = 0.66 cfs @ 12.09 hrs, Volume= 2,302 cf, Depth> 7.90"

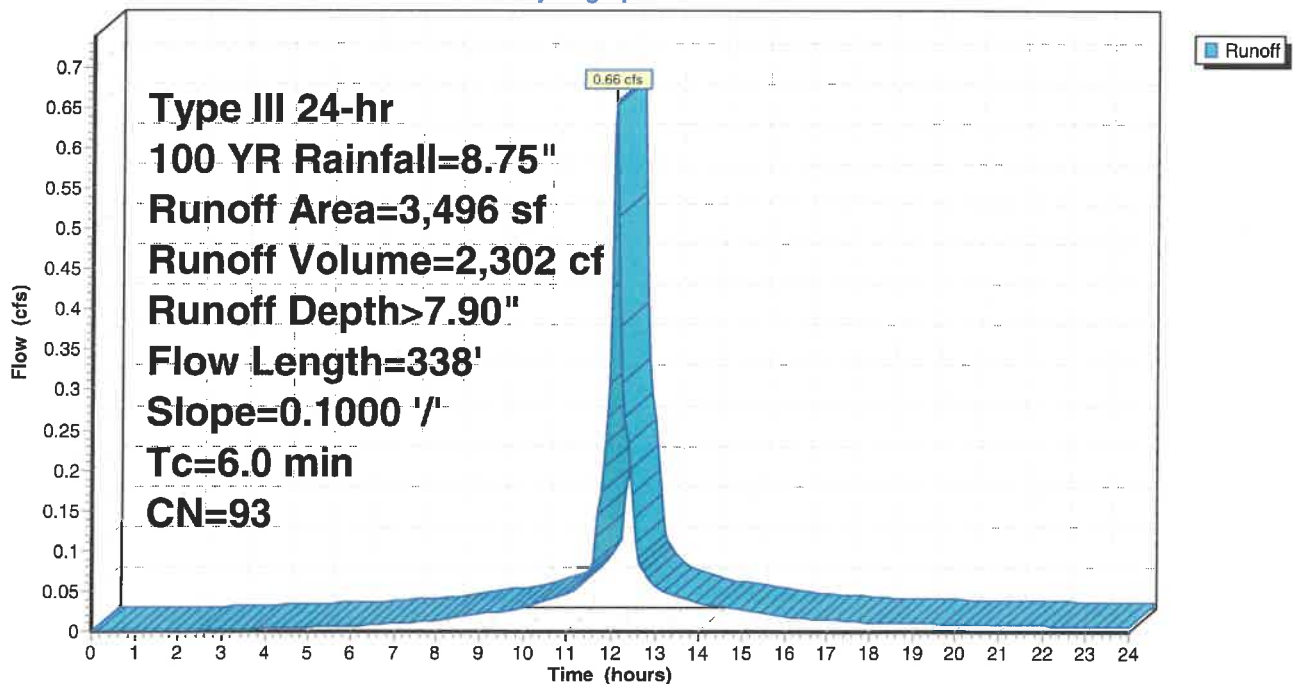
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 3,000 | 98 | Paved roads w/curbs & sewers, HSG B |
| 496 | 61 | >75% Grass cover, Good, HSG B |
| 3,496 | 93 | Weighted Average |
| 496 | | 14.19% Pervious Area |
| 3,000 | | 85.81% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.1000 | 2.29 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 288 | 0.1000 | 6.42 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 338 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 9S: To CB 5

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Summary for Subcatchment 10S: To CB 7

Runoff = 0.77 cfs @ 12.09 hrs, Volume= 2,559 cf, Depth> 7.06"

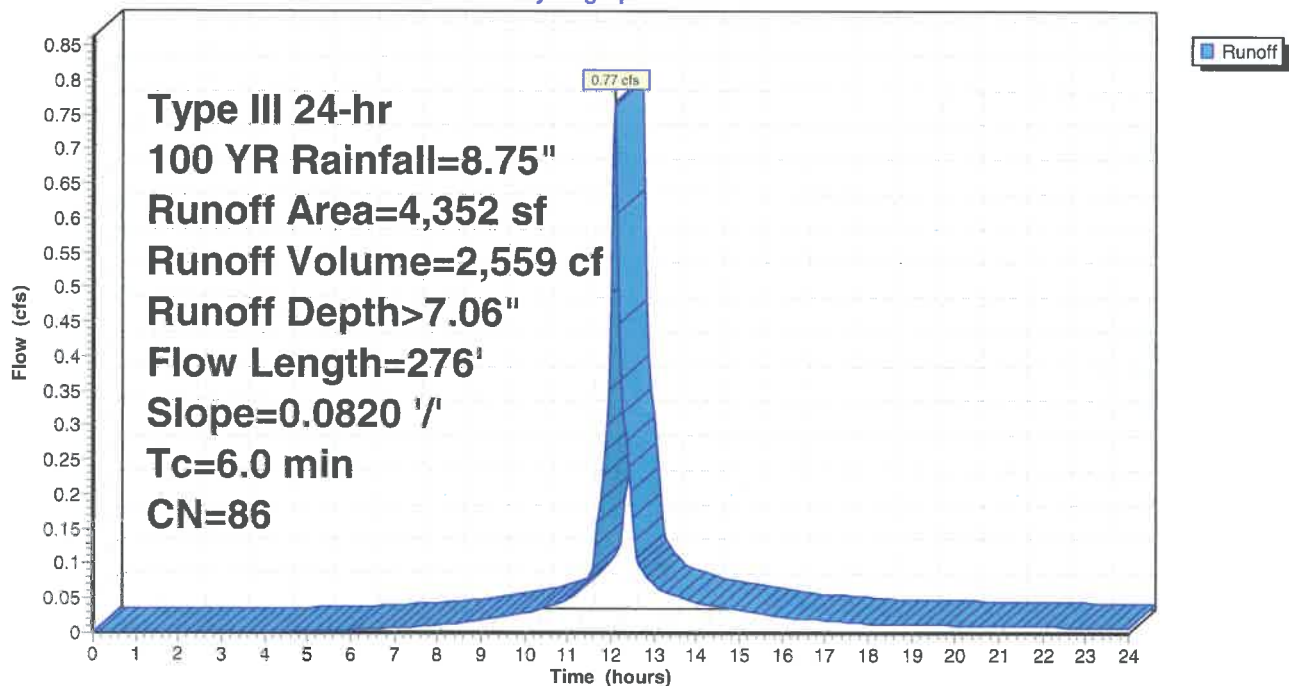
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,363 | 61 | >75% Grass cover, Good, HSG B |
| 4,352 | 86 | Weighted Average |
| 1,363 | | 31.32% Pervious Area |
| 2,989 | | 68.68% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.6 | 226 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.0 | 276 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 10S: To CB 7

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Summary for Subcatchment 11S: To CB 8

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 2,480 cf, Depth> 7.30"

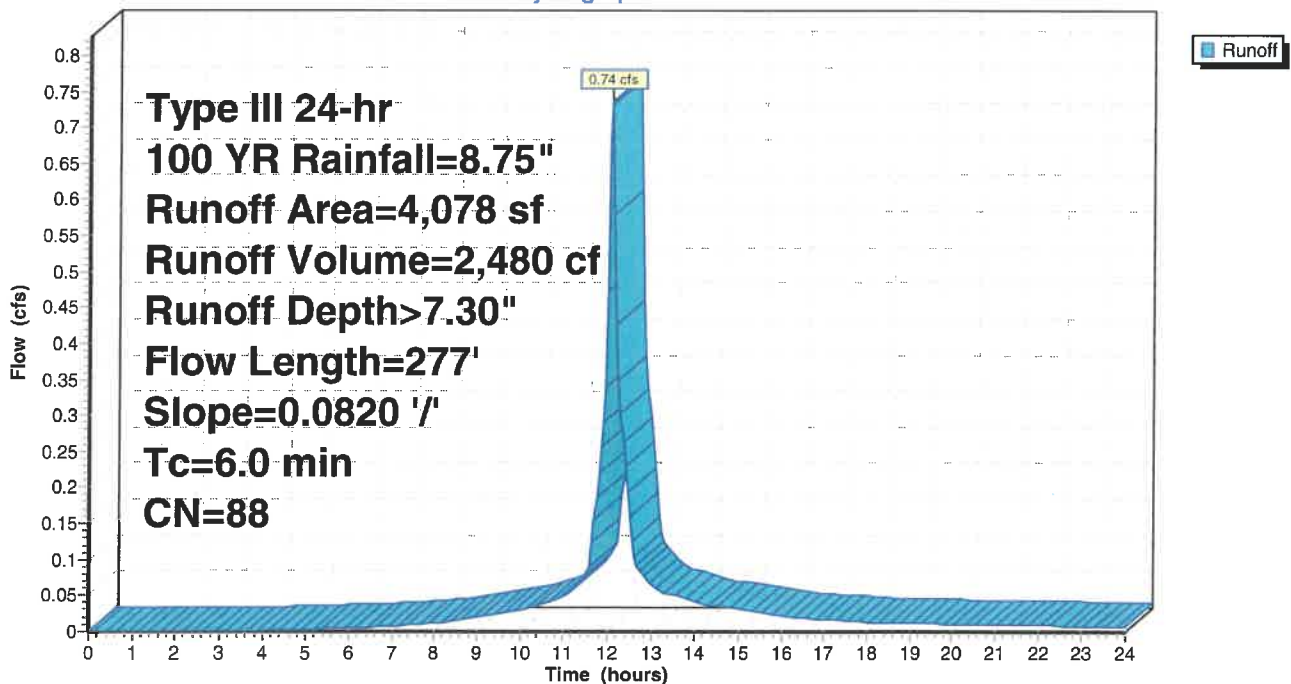
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 2,989 | 98 | Paved roads w/curbs & sewers, HSG B |
| 1,089 | 61 | >75% Grass cover, Good, HSG B |
| 4,078 | 88 | Weighted Average |
| 1,089 | | 26.70% Pervious Area |
| 2,989 | | 73.30% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.4 | 50 | 0.0820 | 2.11 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 0.7 | 227 | 0.0820 | 5.81 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 1.1 | 277 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 11S: To CB 8

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Summary for Subcatchment 12S: To CB 9

Runoff = 2.34 cfs @ 12.09 hrs, Volume= 7,693 cf, Depth> 6.81"

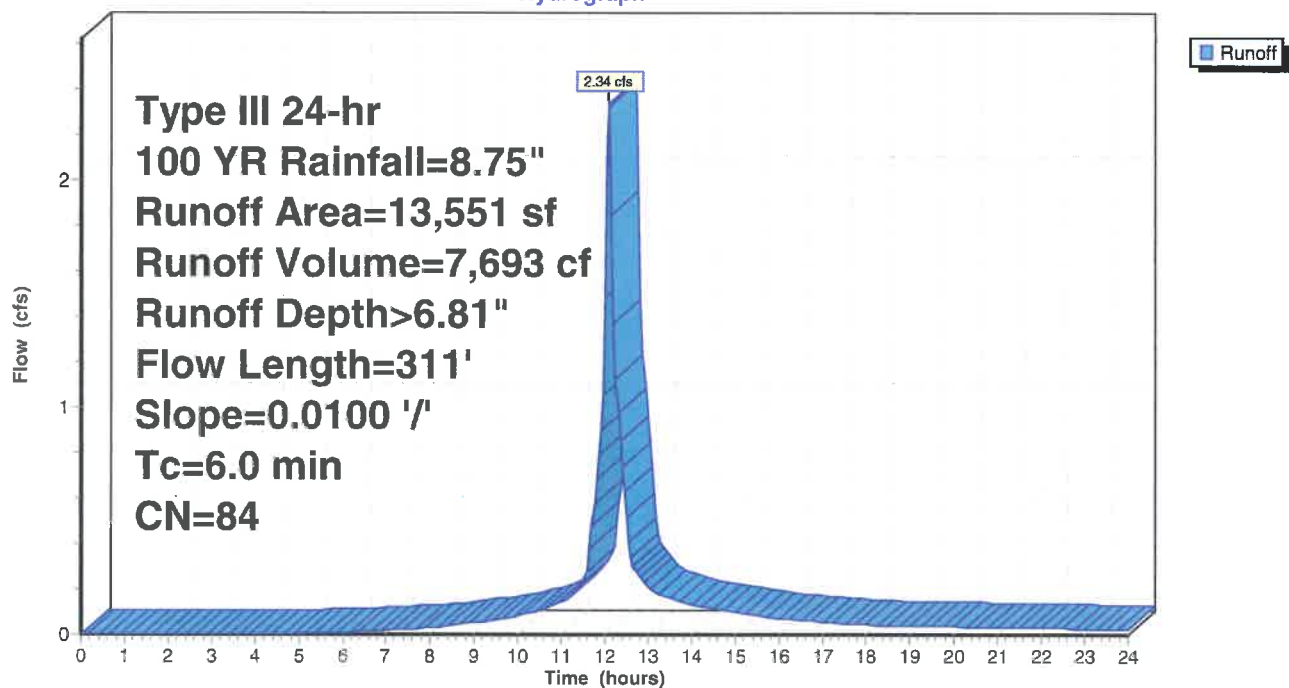
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------------|
| 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| 5,218 | 61 | >75% Grass cover, Good, HSG B |
| 13,551 | 84 | Weighted Average |
| 5,218 | | 38.51% Pervious Area |
| 8,333 | | 61.49% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|--|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.1 | 261 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.0 | 311 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 12S: To CB 9

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Summary for Subcatchment 13S: To CB 10

Runoff = 2.49 cfs @ 12.09 hrs, Volume= 8,099 cf, Depth> 6.57"

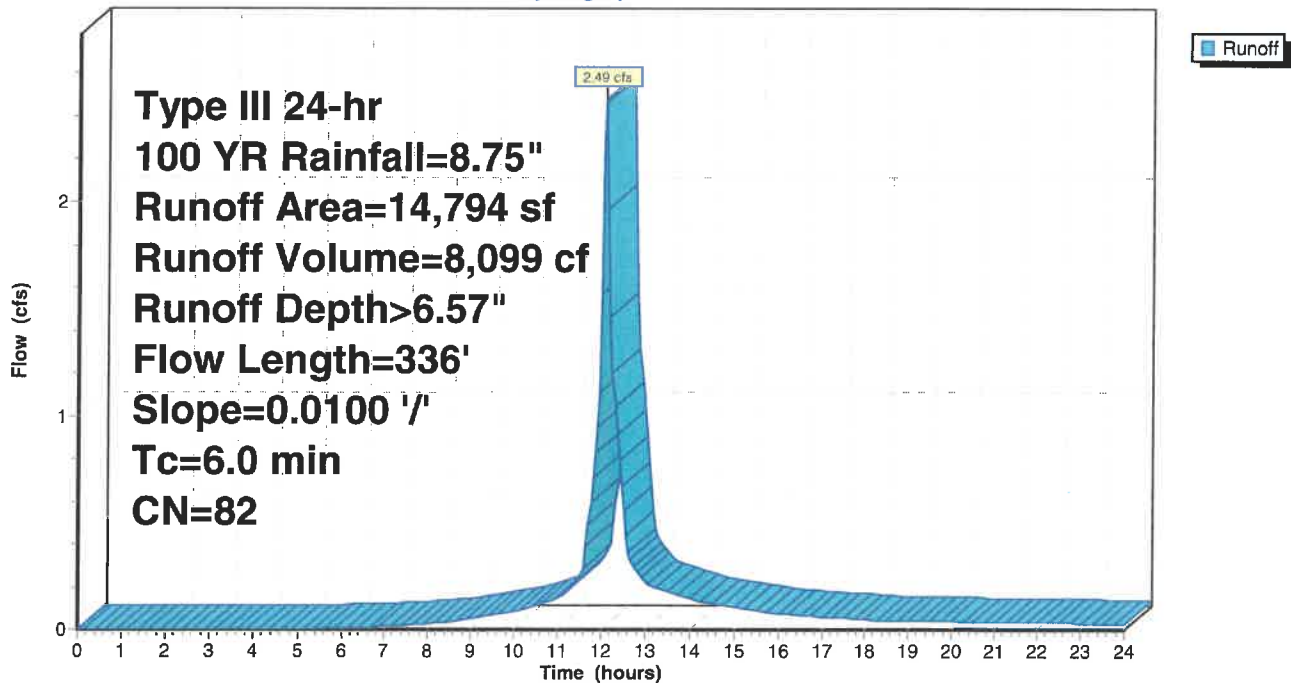
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| | Area (sf) | CN | Description |
|---|-----------|----|-------------------------------------|
| * | 8,333 | 98 | Paved roads w/curbs & sewers, HSG B |
| | 6,461 | 61 | >75% Grass cover, Good, HSG B |
| | 14,794 | 82 | Weighted Average |
| | 6,461 | | 43.67% Pervious Area |
| | 8,333 | | 56.33% Impervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|--|-------------------|----------------|---|
| 0.9 | 50 | 0.0100 | 0.91 | | Sheet Flow, Smooth surfaces n= 0.011 P2= 3.22" |
| 2.3 | 286 | 0.0100 | 2.03 | | Shallow Concentrated Flow, Paved Kv= 20.3 fps |
| 3.2 | 336 | Total, Increased to minimum Tc = 6.0 min | | | |

Subcatchment 13S: To CB 10

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Summary for Subcatchment 14S: To Wetland

Runoff = 45.59 cfs @ 12.35 hrs, Volume= 228,390 cf, Depth> 4.49"

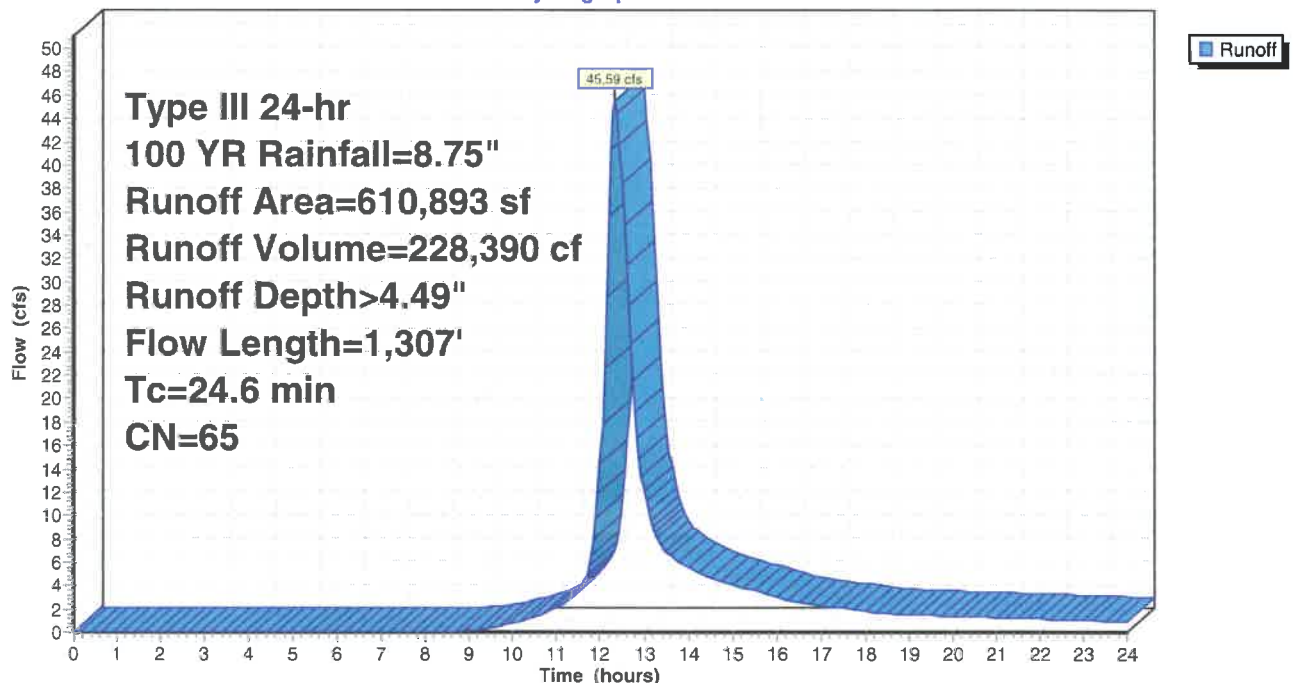
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 326,808 | 55 | Woods, Good, HSG B |
| 260,659 | 77 | Woods, Good, HSG D |
| 23,426 | 61 | >75% Grass cover, Good, HSG B |
| 610,893 | 65 | Weighted Average |
| 610,893 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 9.8 | 50 | 0.0350 | 0.08 | | Sheet Flow, Sheet Woods: Light underbrush n= 0.400 P2= 3.22" |
| 7.9 | 648 | 0.0750 | 1.37 | | Shallow Concentrated Flow, Shallow Woodland Kv= 5.0 fps |
| 6.9 | 609 | 0.0200 | 1.47 | 1.76 | Channel Flow, Stream Area= 1.2 sf Perim= 3.5' r= 0.34' n= 0.070 Medium-dense brush, winter |
| 24.6 | 1,307 | Total | | | |

Subcatchment 14S: To Wetland

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Summary for Subcatchment 15S: TO BASIN

Runoff = 7.89 cfs @ 12.19 hrs, Volume= 31,731 cf, Depth> 3.42"

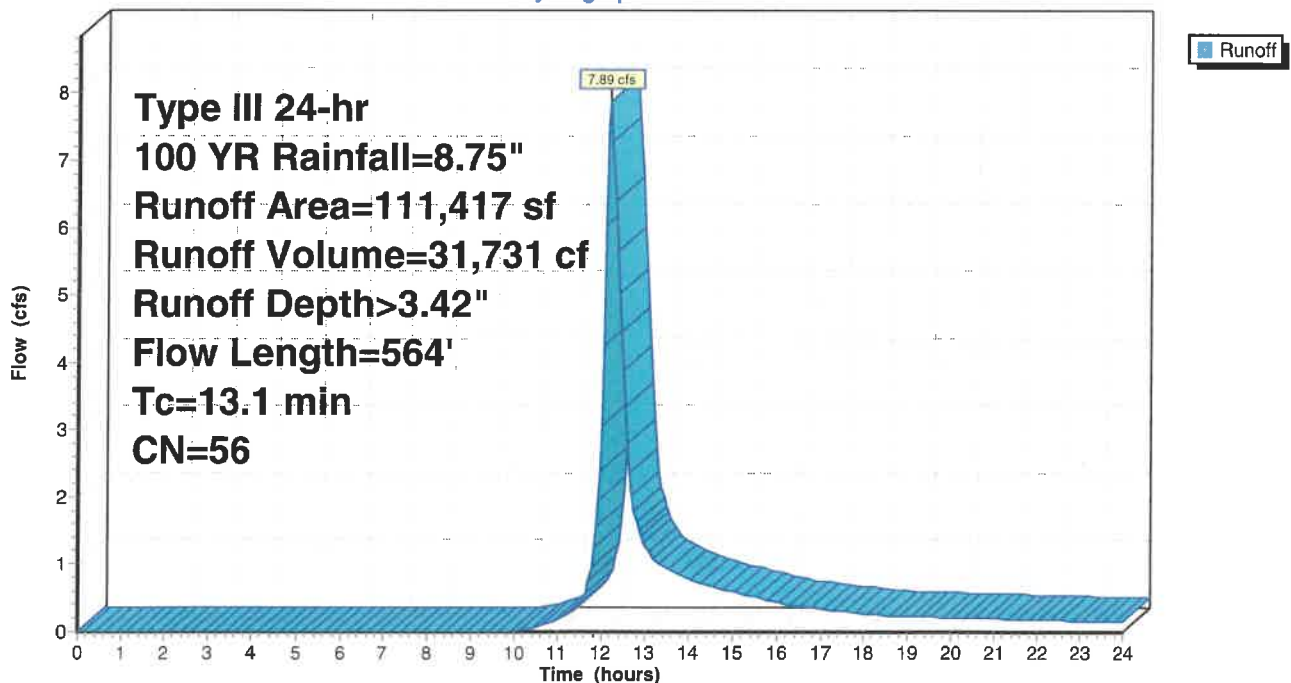
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 89,974 | 55 | Woods, Good, HSG B |
| 21,443 | 61 | >75% Grass cover, Good, HSG B |
| 111,417 | 56 | Weighted Average |
| 111,417 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 7.8 | 50 | 0.0620 | 0.11 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 5.1 | 467 | 0.0931 | 1.53 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.2 | 47 | 0.3190 | 3.95 | | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 13.1 | 564 | Total | | | |

Subcatchment 15S: TO BASIN

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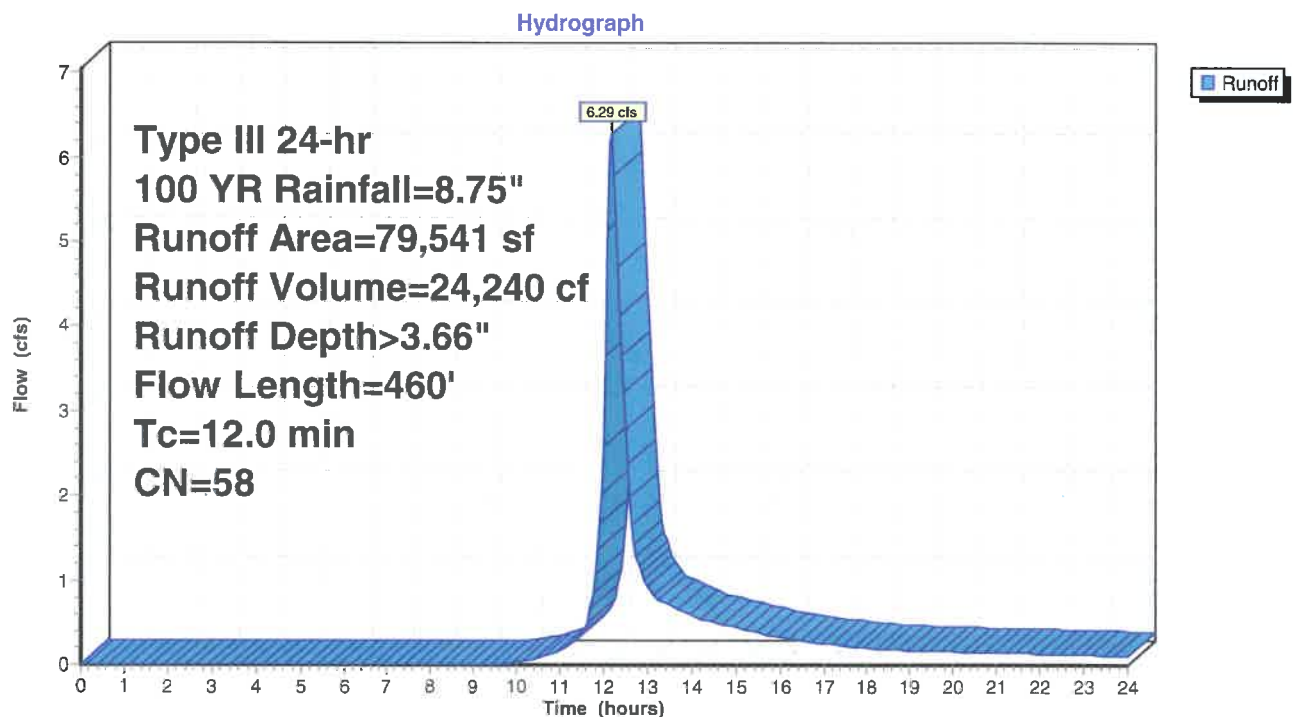
Summary for Subcatchment 16S: TO BASIN

Runoff = 6.29 cfs @ 12.17 hrs, Volume= 24,240 cf, Depth> 3.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 35,427 | 61 | >75% Grass cover, Good, HSG B |
| 44,114 | 55 | Woods, Good, HSG B |
| 79,541 | 58 | Weighted Average |
| 79,541 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|---|
| 7.1 | 50 | 0.0800 | 0.12 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 4.8 | 391 | 0.0735 | 1.36 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 0.1 | 19 | 0.3150 | 5.05 | | Shallow Concentrated Flow, Cultivated Straight Rows Kv= 9.0 fps |
| 12.0 | 460 | Total | | | |

Subcatchment 16S: TO BASIN

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Summary for Subcatchment 17S: To Off Site

Runoff = 13.01 cfs @ 12.16 hrs, Volume= 49,293 cf, Depth> 3.30"

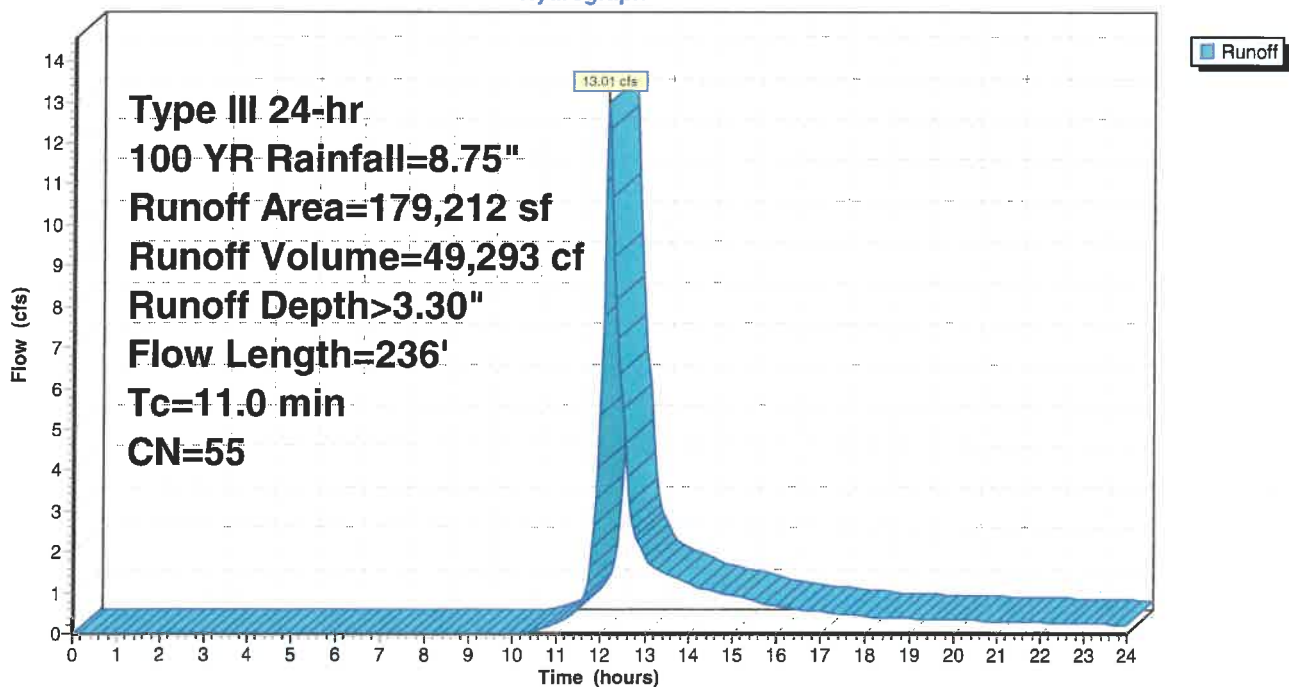
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-----------------------|
| 179,212 | 55 | Woods, Good, HSG B |
| 179,212 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|--|
| 8.5 | 50 | 0.0500 | 0.10 | | Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.22" |
| 2.5 | 186 | 0.0600 | 1.22 | | Shallow Concentrated Flow, Woodland Kv= 5.0 fps |
| 11.0 | 236 | Total | | | |

Subcatchment 17S: To Off Site

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Summary for Subcatchment 19S: Area To Basin3

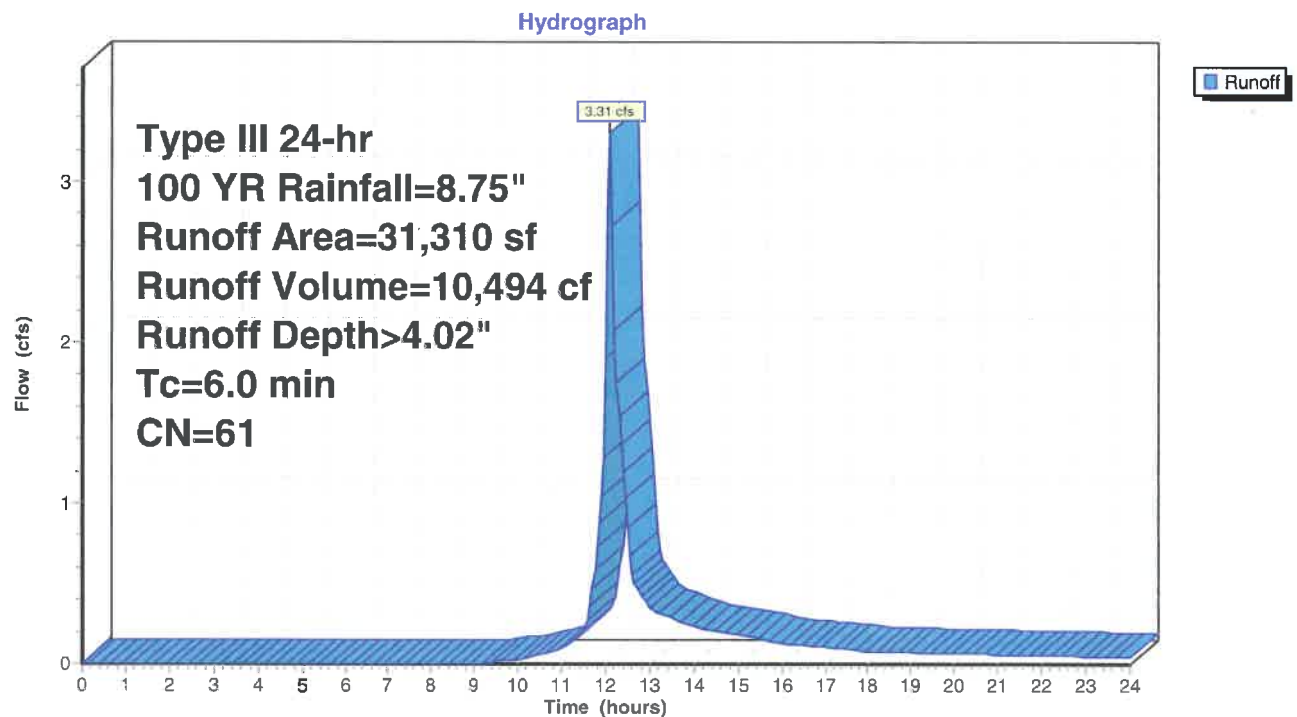
Runoff = 3.31 cfs @ 12.10 hrs, Volume= 10,494 cf, Depth> 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=8.75"

| Area (sf) | CN | Description |
|-----------|----|-------------------------------|
| 31,310 | 61 | >75% Grass cover, Good, HSG B |
| 31,310 | | 100.00% Pervious Area |

| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
|----------|---------------|---------------|-------------------|----------------|-------------------|
| 6.0 | | | | | Direct Entry, a-b |

Subcatchment 19S: Area To Basin3



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Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 4.00" for 100 YR event
Inflow = 30.99 cfs @ 12.40 hrs, Volume= 164,278 cf
Outflow = 30.97 cfs @ 12.40 hrs, Volume= 164,258 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 10.70 fps, Min. Travel Time= 0.1 min

Avg. Velocity = 3.87 fps, Avg. Travel Time= 0.3 min

Peak Storage= 177 cf @ 12.40 hrs

Average Depth at Peak Storage= 0.72' , Surface Width= 4.00'

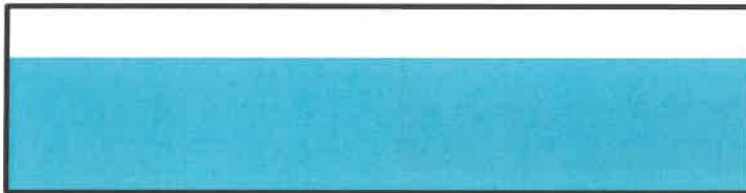
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe

n= 0.013 Concrete, trowel finish

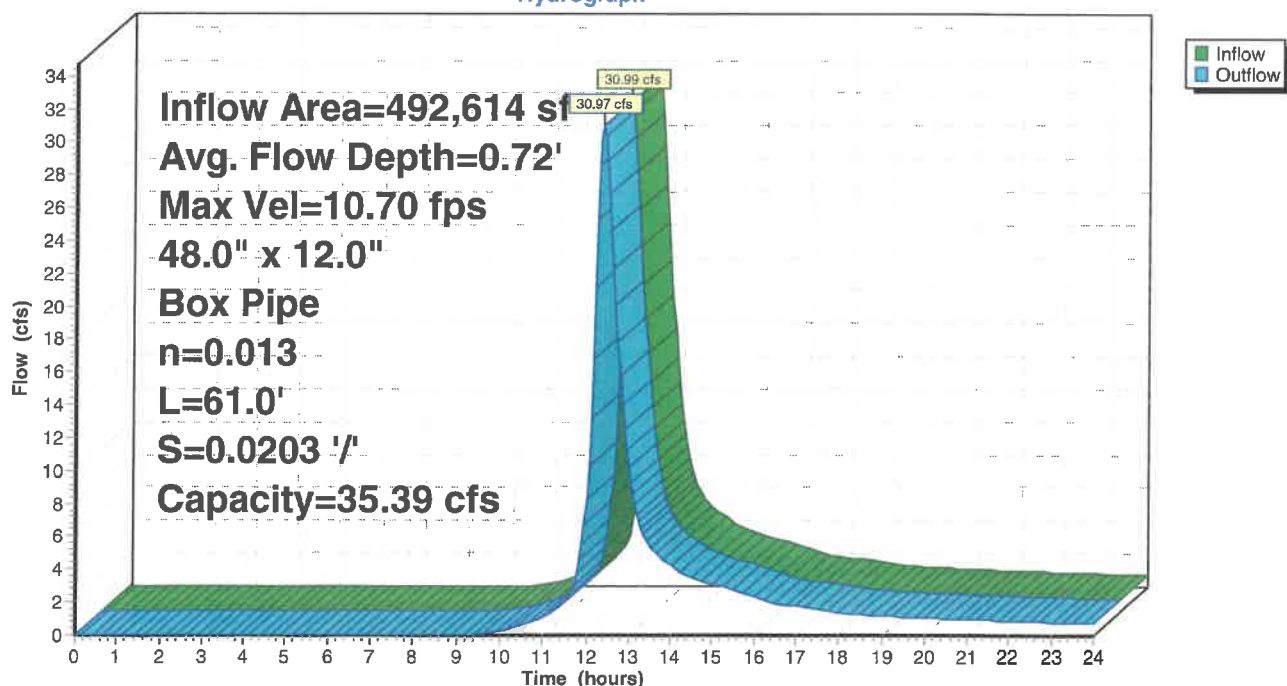
Length= 61.0' Slope= 0.0203 '/'

Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

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Summary for Reach 2R: New Box Cumvert

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 3.53" for 100 YR event
Inflow = 134.84 cfs @ 12.67 hrs, Volume= 809,562 cf
Outflow = 127.38 cfs @ 12.65 hrs, Volume= 809,525 cf, Atten= 6%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 18.60 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 7.85 fps, Avg. Travel Time= 0.1 min

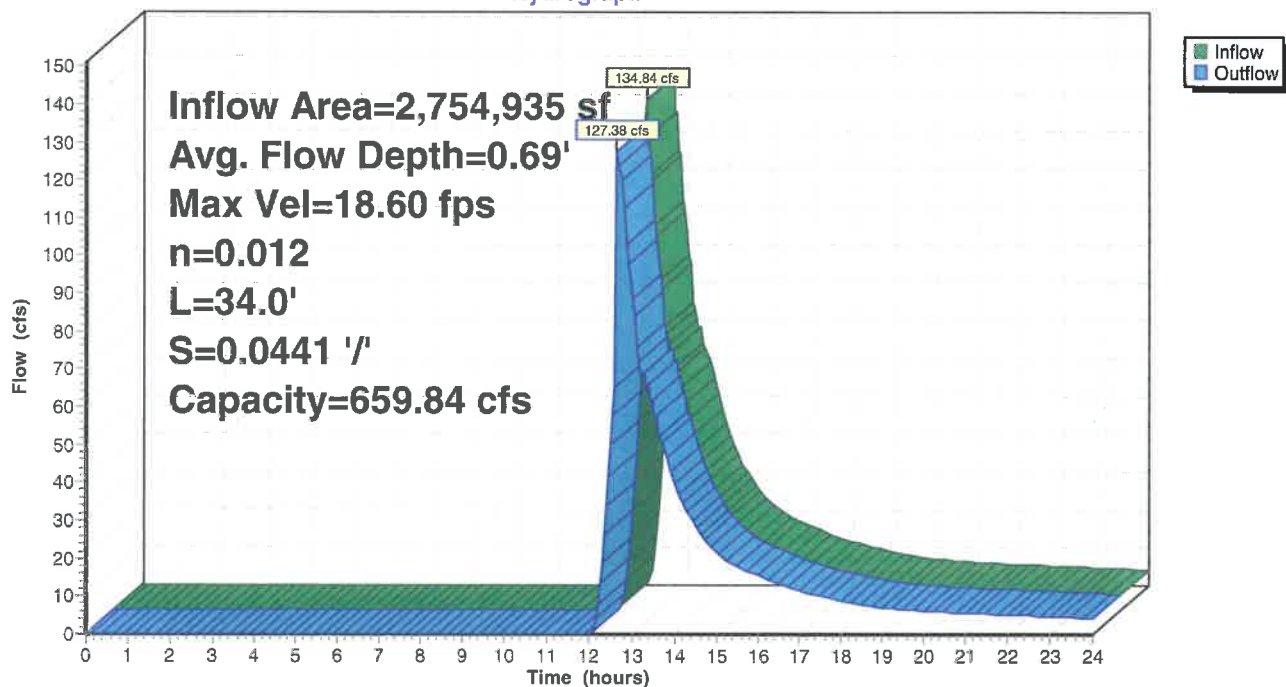
Peak Storage= 234 cf @ 12.65 hrs
Average Depth at Peak Storage= 0.69' , Surface Width= 10.00'
Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 659.84 cfs

10.00' x 2.00' deep channel, n= 0.012 Concrete pipe, finished
Length= 34.0' Slope= 0.0441 '/'
Inlet Invert= 294.00', Outlet Invert= 292.50'



Reach 2R: New Box Cumvert

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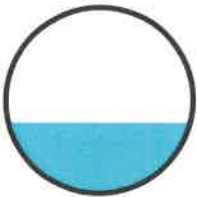
Summary for Reach 3R: CB1 to DMH 1

Inflow Area = 13,094 sf, 84.66% Impervious, Inflow Depth > 7.78" for 100 YR event
Inflow = 2.45 cfs @ 12.09 hrs, Volume= 8,491 cf
Outflow = 2.45 cfs @ 12.09 hrs, Volume= 8,491 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.97 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 3.00 fps, Avg. Travel Time= 0.0 min

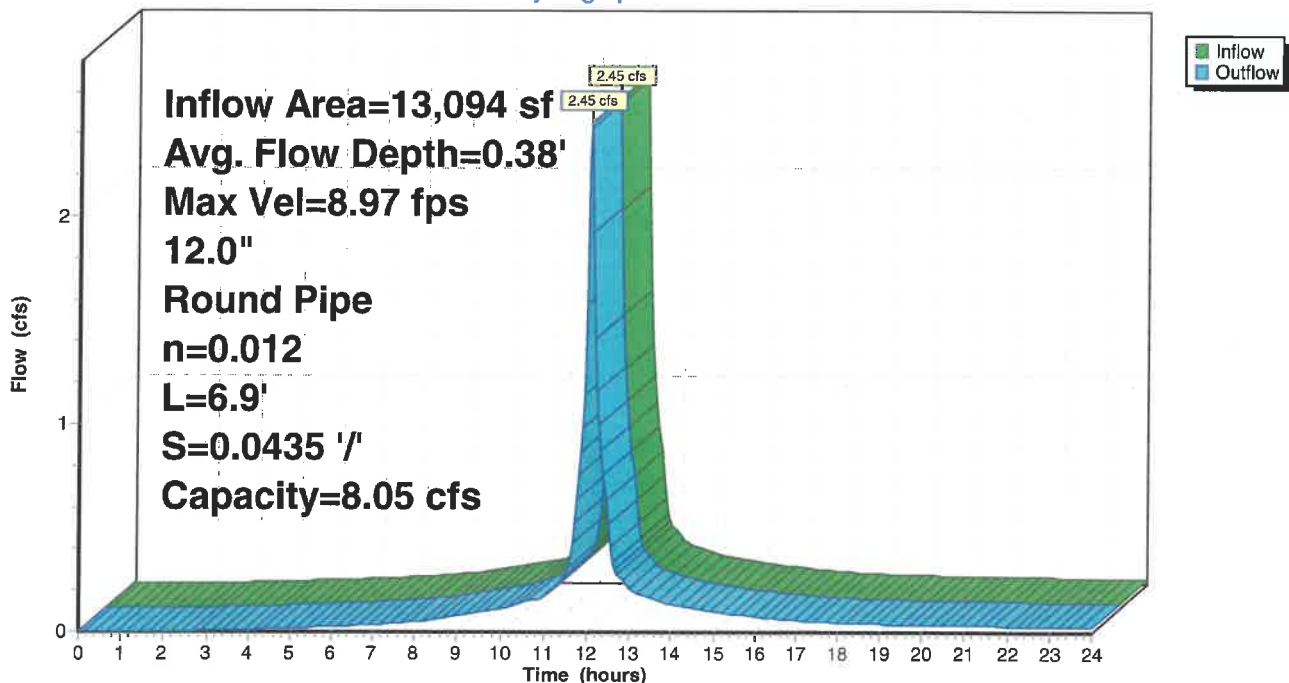
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.38', Surface Width= 0.97'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.9' Slope= 0.0435 '/'
Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 3R: CB1 to DMH 1

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Summary for Reach 4R: CB2 to DMH 1

Inflow Area = 11,319 sf, 94.70% Impervious, Inflow Depth > 8.26" for 100 YR event
Inflow = 2.17 cfs @ 12.09 hrs, Volume= 7,795 cf
Outflow = 2.17 cfs @ 12.09 hrs, Volume= 7,795 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 8.67 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 2.91 fps, Avg. Travel Time= 0.0 min

Peak Storage= 2 cf @ 12.09 hrs

Average Depth at Peak Storage= 0.35', Surface Width= 0.96'

Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe

n= 0.012 Corrugated PP, smooth interior

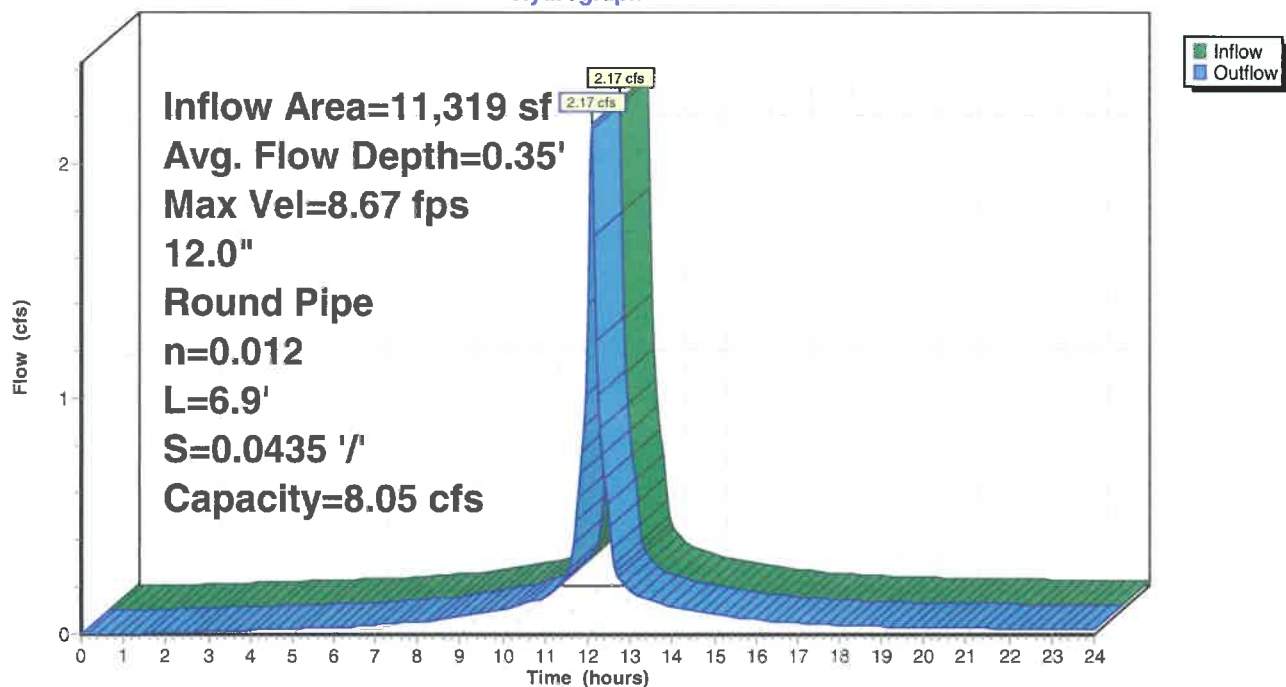
Length= 6.9' Slope= 0.0435 '/'

Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 4R: CB2 to DMH 1

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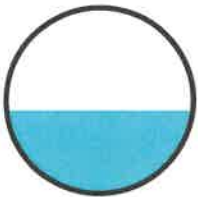
Summary for Reach 5R: DMH1 to DMH 2

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 8.01" for 100 YR event
Inflow = 4.62 cfs @ 12.09 hrs, Volume= 16,286 cf
Outflow = 4.59 cfs @ 12.09 hrs, Volume= 16,283 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.07 fps, Min. Travel Time= 0.2 min
Avg. Velocity= 2.02 fps, Avg. Travel Time= 0.6 min

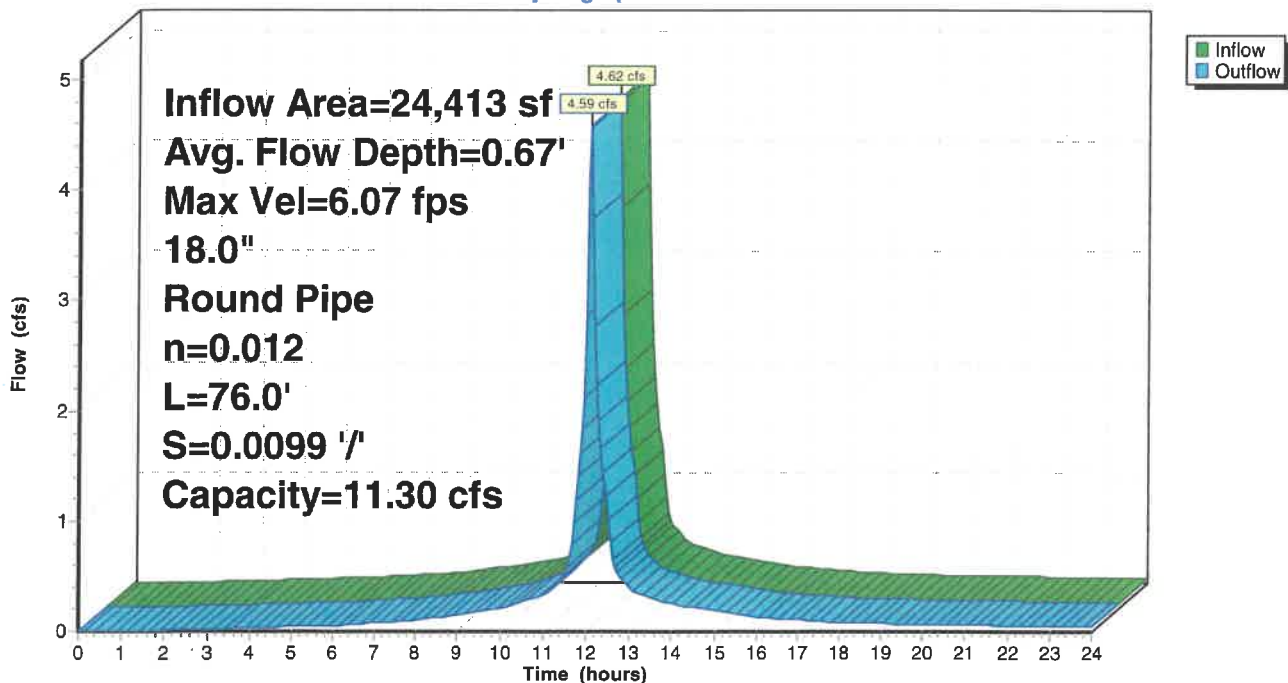
Peak Storage= 58 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.67', Surface Width= 1.49'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 11.30 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 76.0' Slope= 0.0099 '/'
Inlet Invert= 291.75', Outlet Invert= 291.00'



Reach 5R: DMH1 to DMH 2

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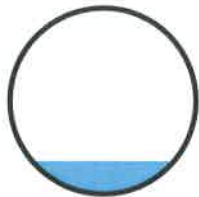
Summary for Reach 6R: CB3 to DMH 2

Inflow Area = 3,124 sf, 75.00% Impervious, Inflow Depth > 7.42" for 100 YR event
Inflow = 0.57 cfs @ 12.09 hrs, Volume= 1,931 cf
Outflow = 0.57 cfs @ 12.09 hrs, Volume= 1,931 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.08 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.01 fps, Avg. Travel Time= 0.1 min

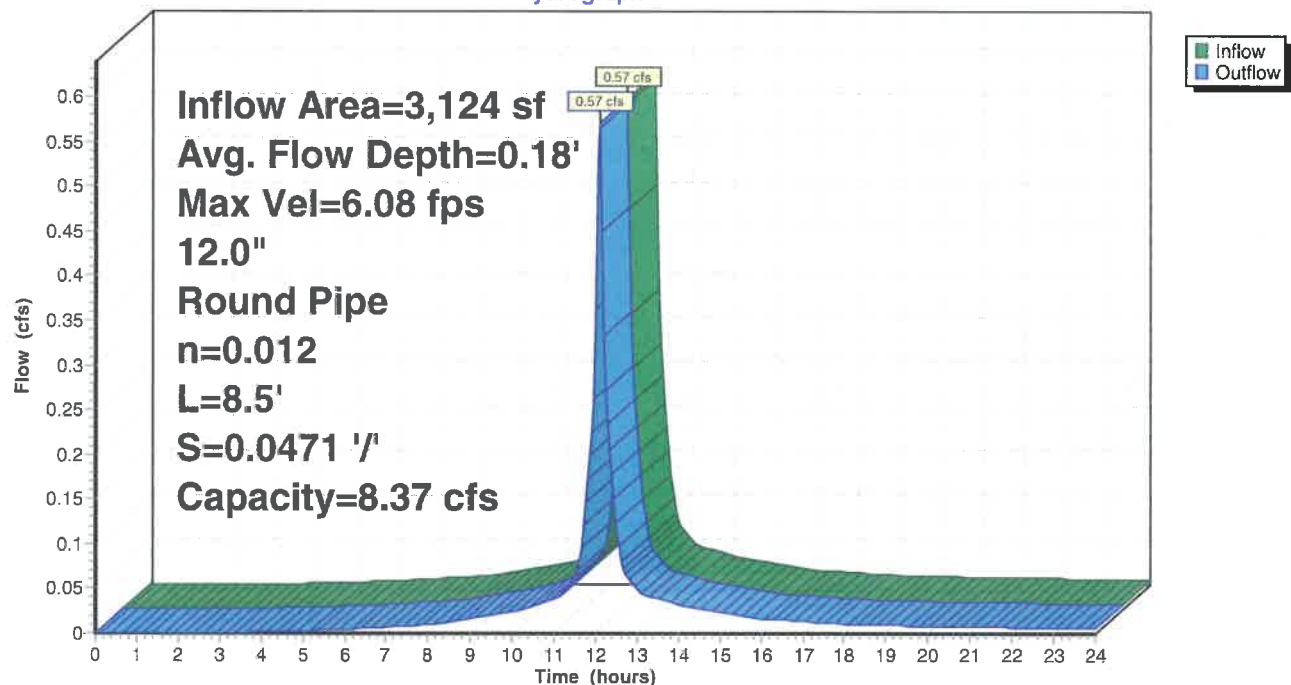
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.18', Surface Width= 0.76'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 8.5' Slope= 0.0471 1/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 6R: CB3 to DMH 2

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Summary for Reach 7R: CB4 to DMH 2

Inflow Area = 2,550 sf, 75.02% Impervious, Inflow Depth > 7.42" for 100 YR event
Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,576 cf
Outflow = 0.47 cfs @ 12.09 hrs, Volume= 1,576 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.80 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.59 fps, Avg. Travel Time= 0.1 min

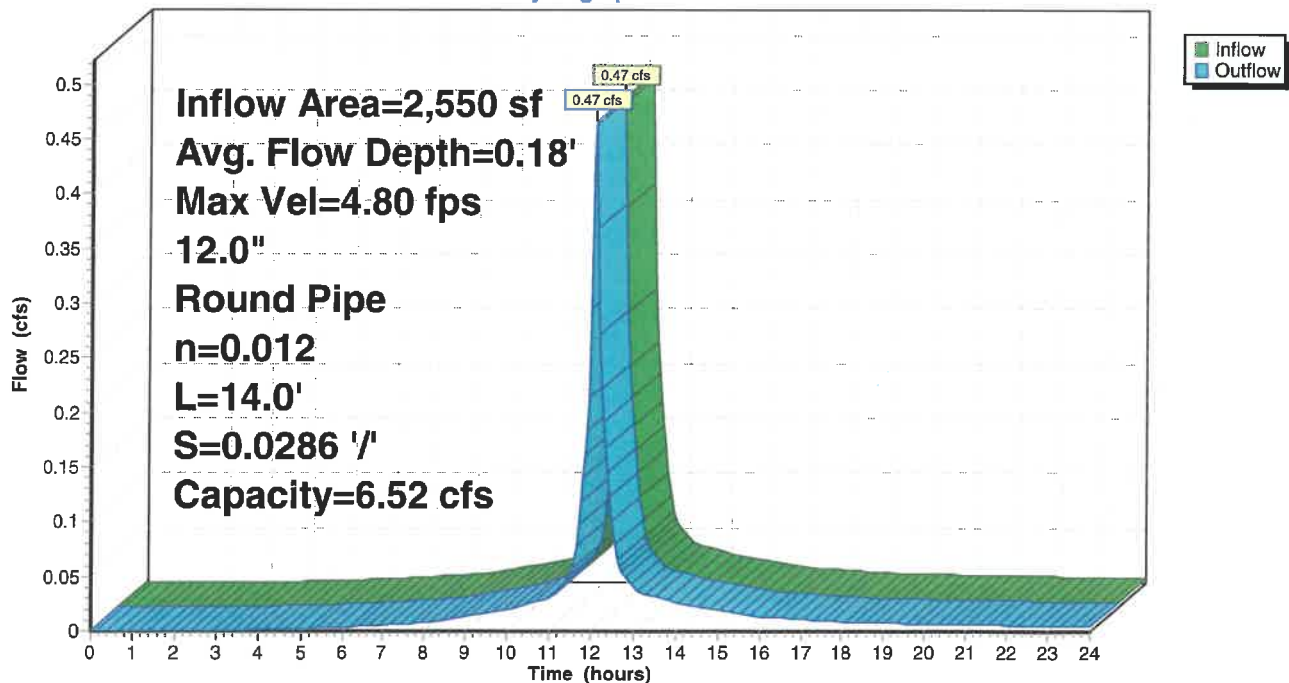
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.18', Surface Width= 0.77'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 6.52 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 14.0' Slope= 0.0286 '/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 7R: CB4 to DMH 2

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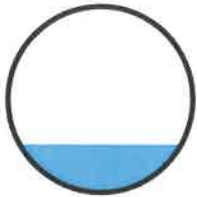
Summary for Reach 8R: DMH 2 TO DMH 7

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 7.44" for 100 YR event
Inflow = 2.33 cfs @ 12.09 hrs, Volume= 7,979 cf
Outflow = 2.26 cfs @ 12.11 hrs, Volume= 7,975 cf, Atten= 3%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.48 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 2.09 fps, Avg. Travel Time= 1.9 min

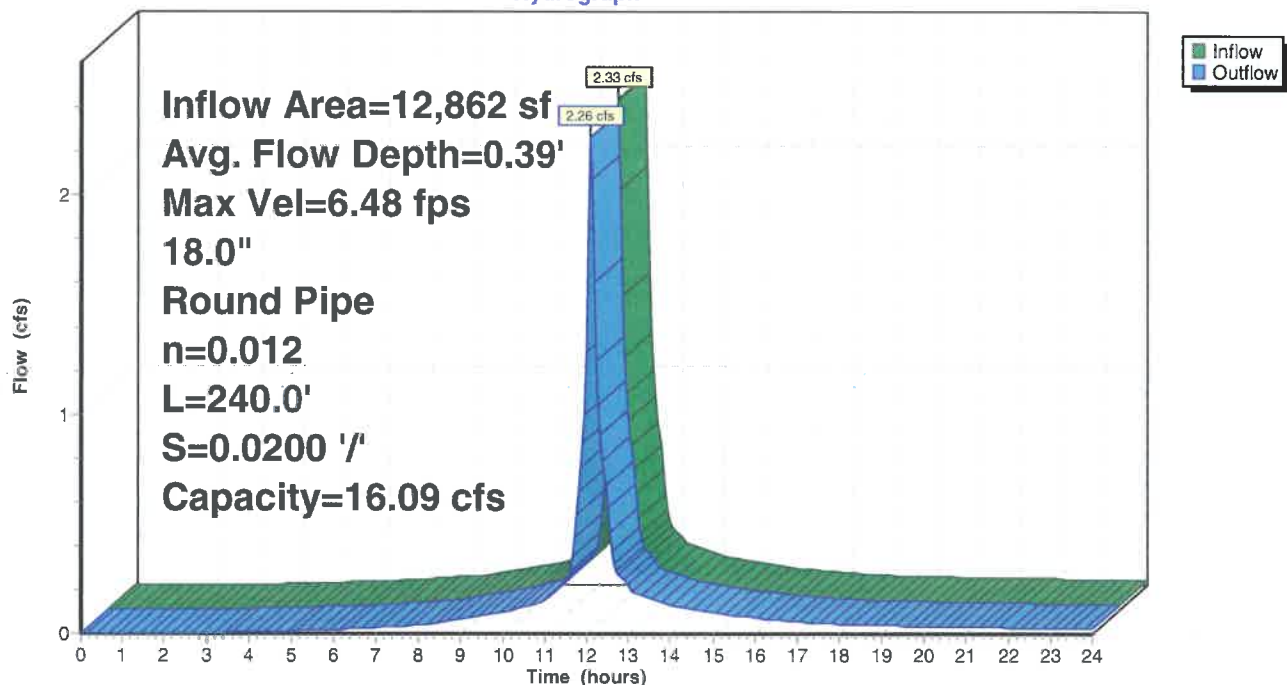
Peak Storage= 86 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.39' , Surface Width= 1.31'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 16.09 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0200 '/'
Inlet Invert= 294.80', Outlet Invert= 290.00'



Reach 8R: DMH 2 TO DMH 7

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Summary for Reach 9R: DMH 3 to DMH 2

Inflow Area = 7,188 sf, 76.42% Impervious, Inflow Depth > 7.47" for 100 YR event
Inflow = 1.31 cfs @ 12.09 hrs, Volume= 4,473 cf
Outflow = 1.29 cfs @ 12.10 hrs, Volume= 4,471 cf, Atten= 1%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 9.38 fps, Min. Travel Time= 0.4 min
Avg. Velocity = 3.02 fps, Avg. Travel Time= 1.2 min

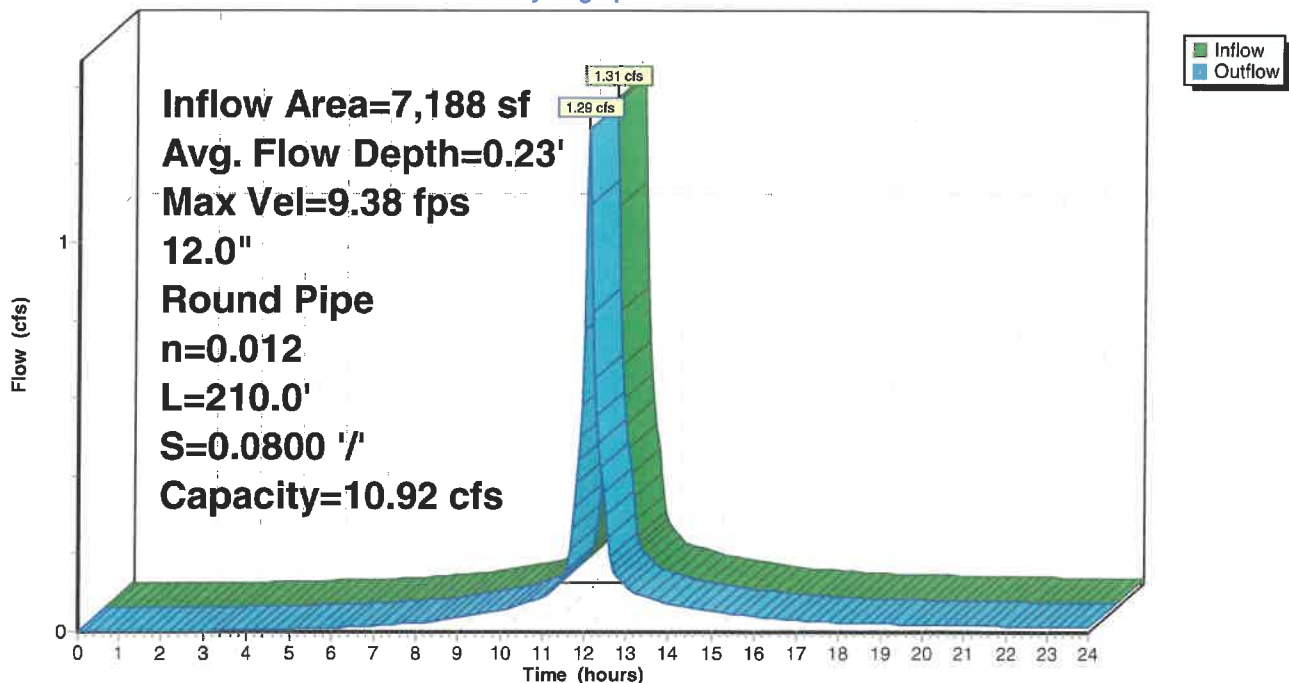
Peak Storage= 29 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.23' , Surface Width= 0.85'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 210.0' Slope= 0.0800 '/'
Inlet Invert= 315.40', Outlet Invert= 298.60'



Reach 9R: DMH 3 to DMH 2

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Summary for Reach 10R: CB5 to DMH 3

Inflow Area = 3,692 sf, 67.52% Impervious, Inflow Depth > 7.06" for 100 YR event
Inflow = 0.65 cfs @ 12.09 hrs, Volume= 2,171 cf
Outflow = 0.65 cfs @ 12.09 hrs, Volume= 2,171 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.18 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.05 fps, Avg. Travel Time= 0.1 min

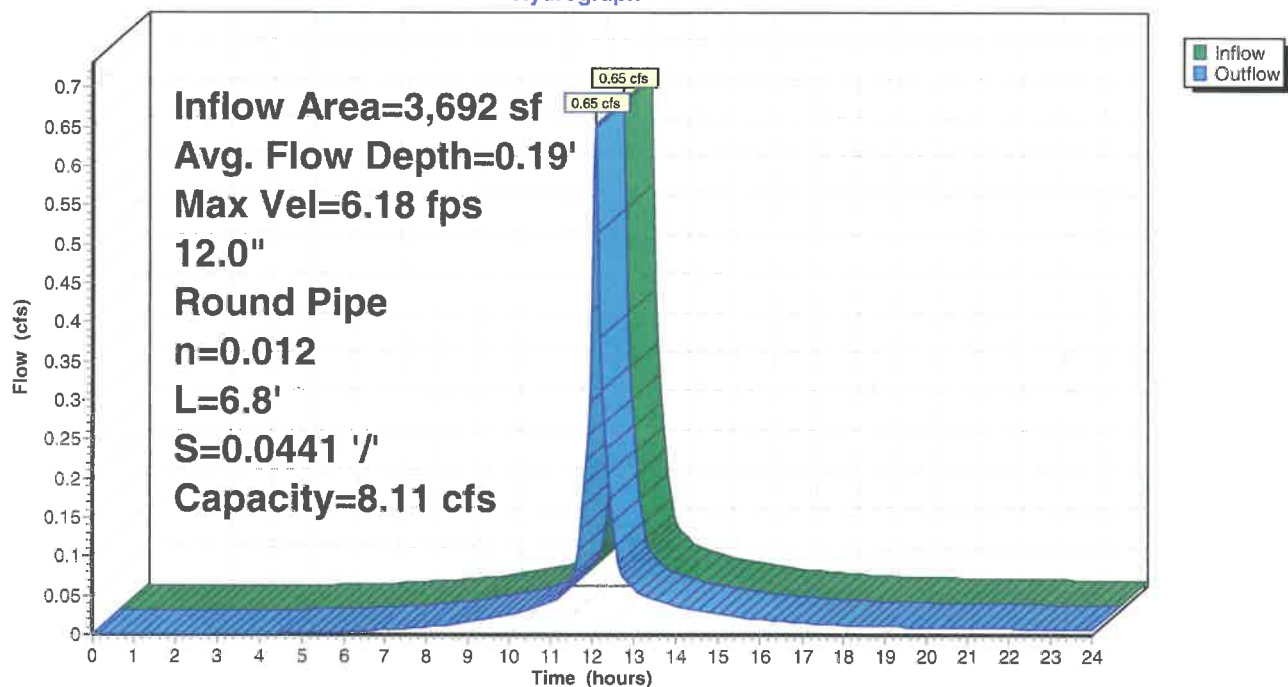
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.19' , Surface Width= 0.79'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.11 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 6.8' Slope= 0.0441 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 10R: CB5 to DMH 3

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Summary for Reach 11R: CB6 to DMH 3

Inflow Area = 3,496 sf, 85.81% Impervious, Inflow Depth > 7.90" for 100 YR event
Inflow = 0.66 cfs @ 12.09 hrs, Volume= 2,302 cf
Outflow = 0.66 cfs @ 12.09 hrs, Volume= 2,302 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.88 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 1.61 fps, Avg. Travel Time= 0.1 min

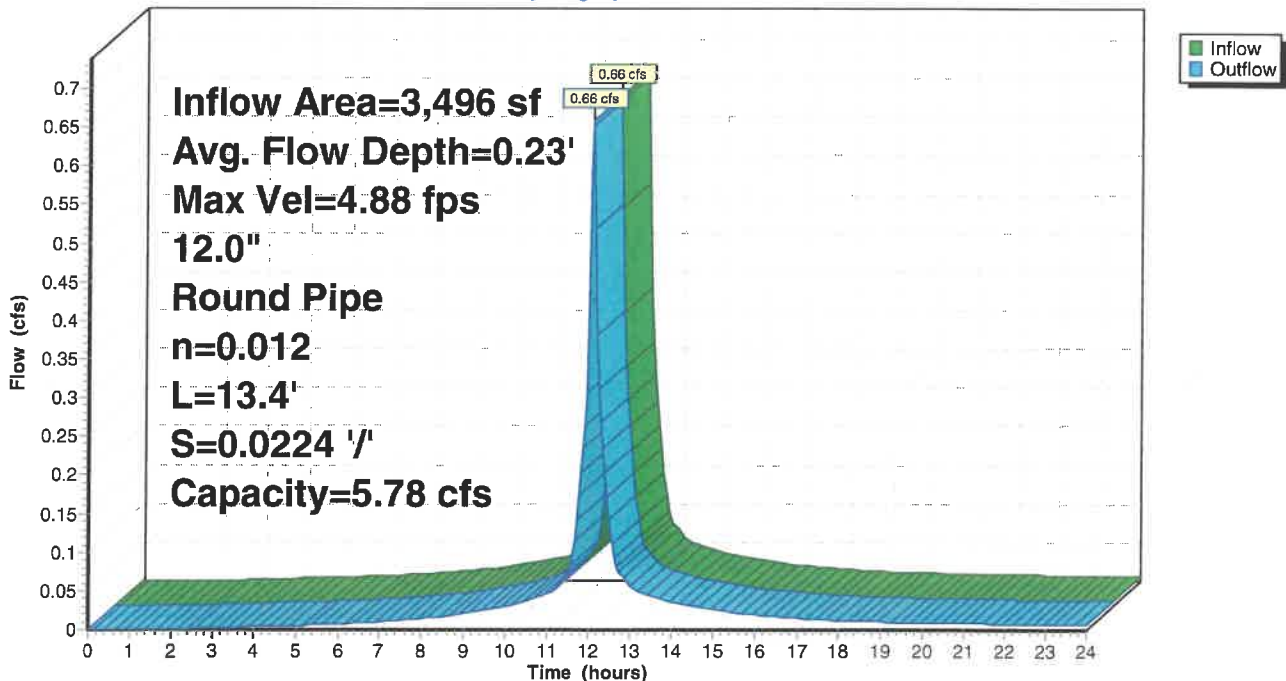
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.23', Surface Width= 0.84'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.78 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0224 '/'
Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 11R: CB6 to DMH 3

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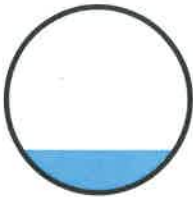
Summary for Reach 12R: DMH 7 TO BASIN

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 7.44" for 100 YR event
Inflow = 2.26 cfs @ 12.11 hrs, Volume= 7,975 cf
Outflow = 2.19 cfs @ 12.13 hrs, Volume= 7,971 cf, Atten= 3%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.31 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 2.39 fps, Avg. Travel Time= 1.7 min

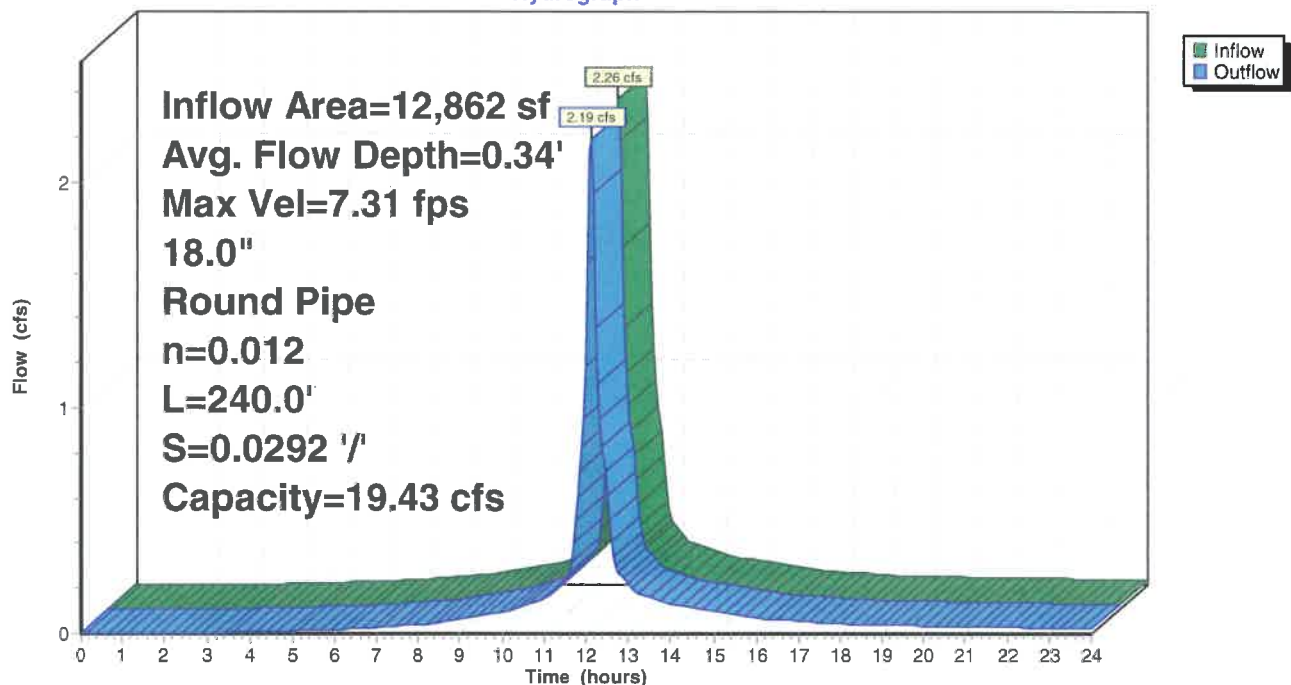
Peak Storage= 74 cf @ 12.12 hrs
Average Depth at Peak Storage= 0.34' , Surface Width= 1.26'
Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 19.43 cfs

18.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 240.0' Slope= 0.0292 '/'
Inlet Invert= 290.00', Outlet Invert= 283.00'



Reach 12R: DMH 7 TO BASIN

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Summary for Reach 13R: CB7 TO DMH 4

Inflow Area = 4,352 sf, 68.68% Impervious, Inflow Depth > 7.06" for 100 YR event
Inflow = 0.77 cfs @ 12.09 hrs, Volume= 2,559 cf
Outflow = 0.77 cfs @ 12.09 hrs, Volume= 2,559 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.99 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.98 fps, Avg. Travel Time= 0.1 min

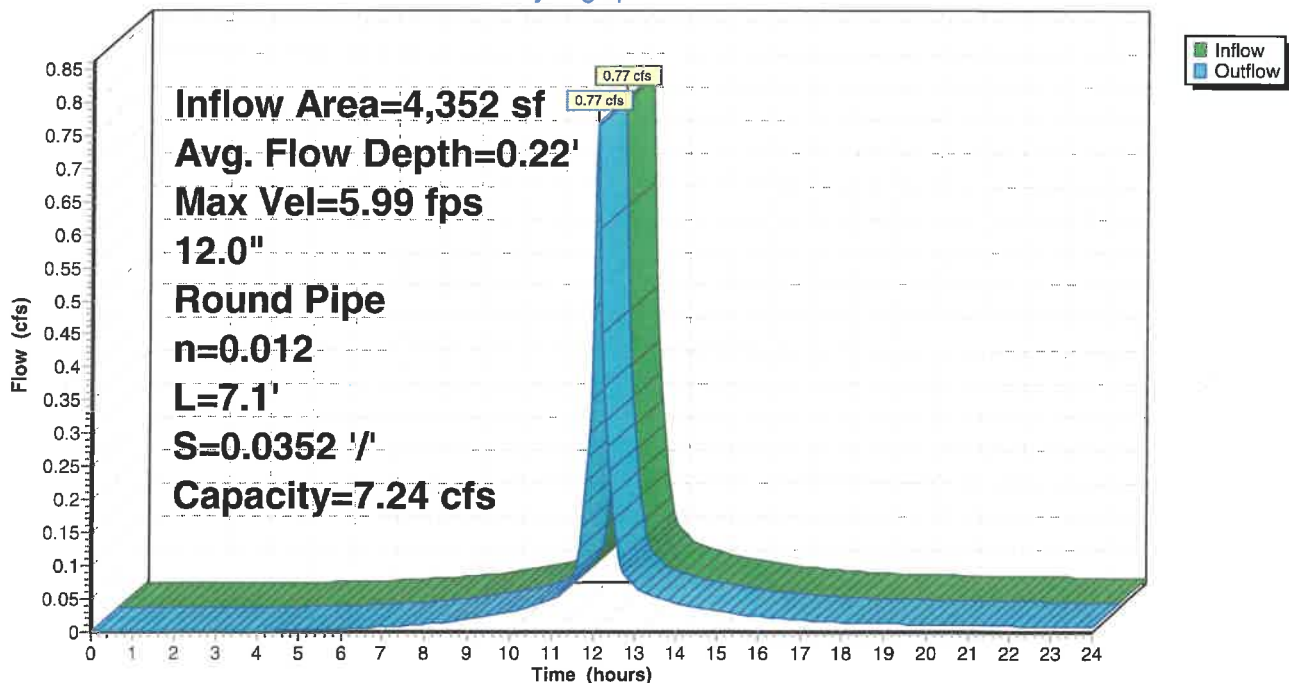
Peak Storage= 1 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.22', Surface Width= 0.83'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.24 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.1' Slope= 0.0352 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 13R: CB7 TO DMH 4

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Summary for Reach 14R: CB 8 TO DMH 4

Inflow Area = 4,078 sf, 73.30% Impervious, Inflow Depth > 7.30" for 100 YR event
Inflow = 0.74 cfs @ 12.09 hrs, Volume= 2,480 cf
Outflow = 0.74 cfs @ 12.09 hrs, Volume= 2,480 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.72 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 1.56 fps, Avg. Travel Time= 0.1 min

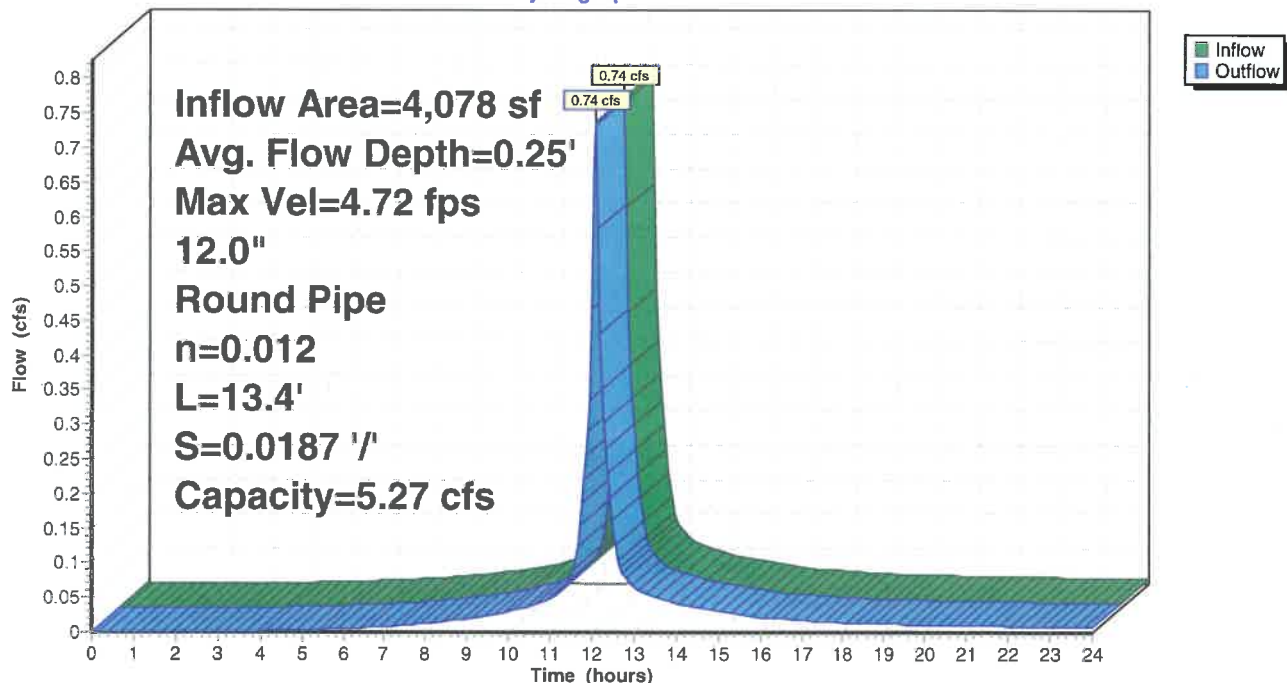
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.25', Surface Width= 0.87'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.27 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 13.4' Slope= 0.0187 '/'
Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 14R: CB 8 TO DMH 4

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Summary for Reach 15R: DMH 4 TO DMH 5

Inflow Area = 8,430 sf, 70.91% Impervious, Inflow Depth > 7.17" for 100 YR event
Inflow = 1.51 cfs @ 12.09 hrs, Volume= 5,039 cf
Outflow = 1.50 cfs @ 12.09 hrs, Volume= 5,038 cf, Atten= 1%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
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Avg. Velocity = 3.20 fps, Avg. Travel Time= 0.5 min

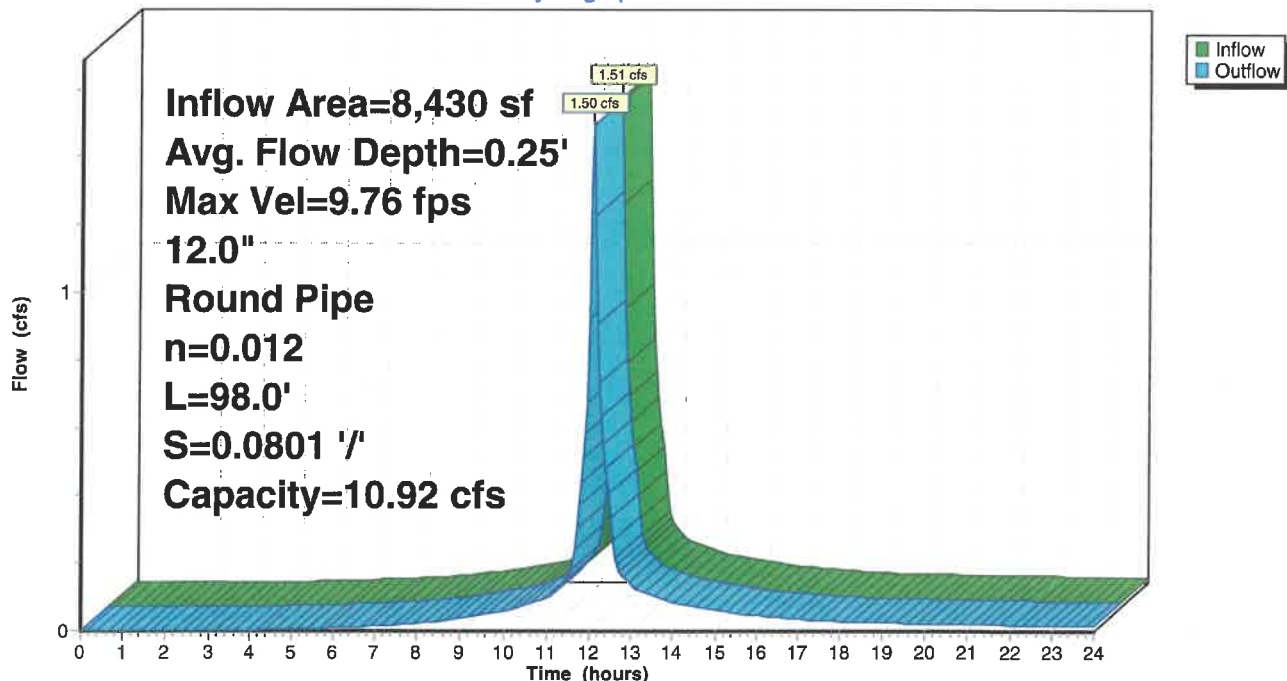
Peak Storage= 15 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.25', Surface Width= 0.87'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 98.0' Slope= 0.0801 1/'
Inlet Invert= 316.20', Outlet Invert= 308.35'



Reach 15R: DMH 4 TO DMH 5

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Summary for Reach 16R: DMH 5 TO DMH 6

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 6.80" for 100 YR event
Inflow = 6.29 cfs @ 12.09 hrs, Volume= 20,827 cf
Outflow = 6.09 cfs @ 12.11 hrs, Volume= 20,816 cf, Atten= 3%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.48 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 2.26 fps, Avg. Travel Time= 1.8 min

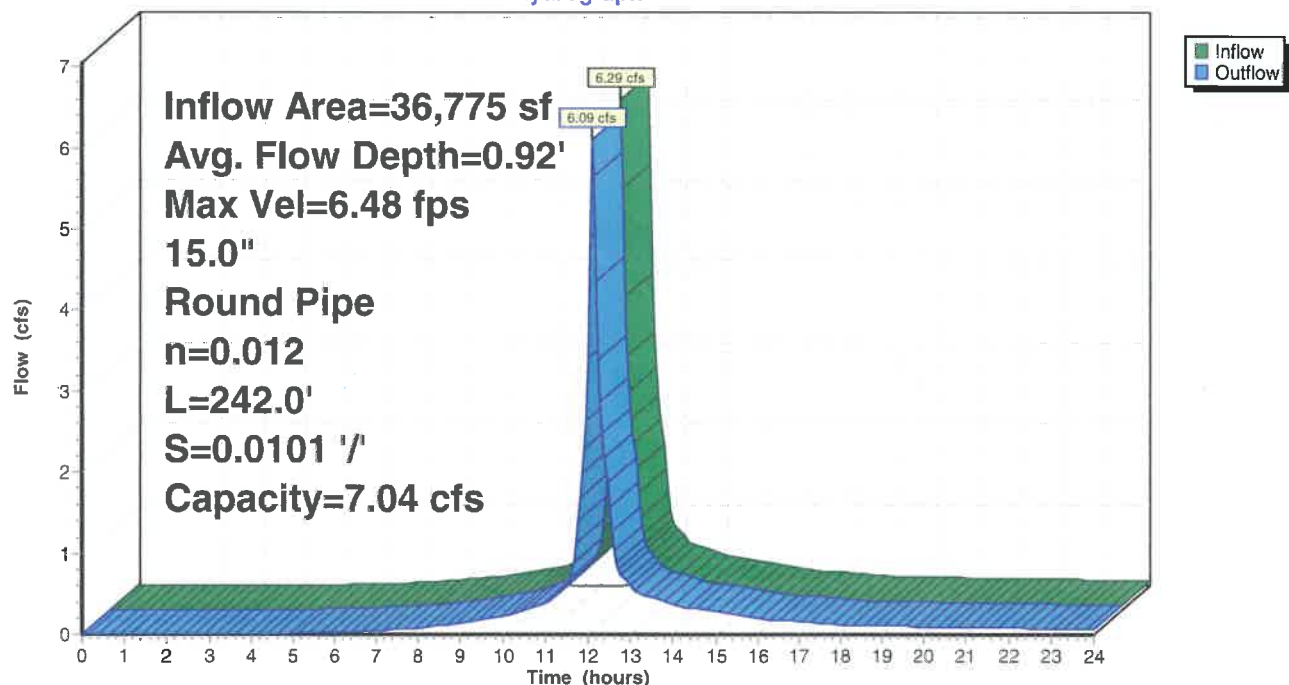
Peak Storage= 234 cf @ 12.10 hrs
Average Depth at Peak Storage= 0.92', Surface Width= 1.10'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 7.04 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 242.0' Slope= 0.0101 '/'
Inlet Invert= 308.10', Outlet Invert= 305.65'



Reach 16R: DMH 5 TO DMH 6

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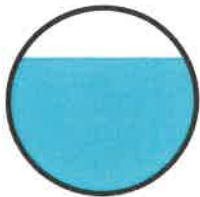
Summary for Reach 17R: DMH 6 TO DMH 5

Inflow Area = 28,345 sf, 58.80% Impervious, Inflow Depth > 6.69" for 100 YR event
Inflow = 4.83 cfs @ 12.09 hrs, Volume= 15,792 cf
Outflow = 4.79 cfs @ 12.10 hrs, Volume= 15,789 cf, Atten= 1%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.83 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.79 fps, Avg. Travel Time= 0.6 min

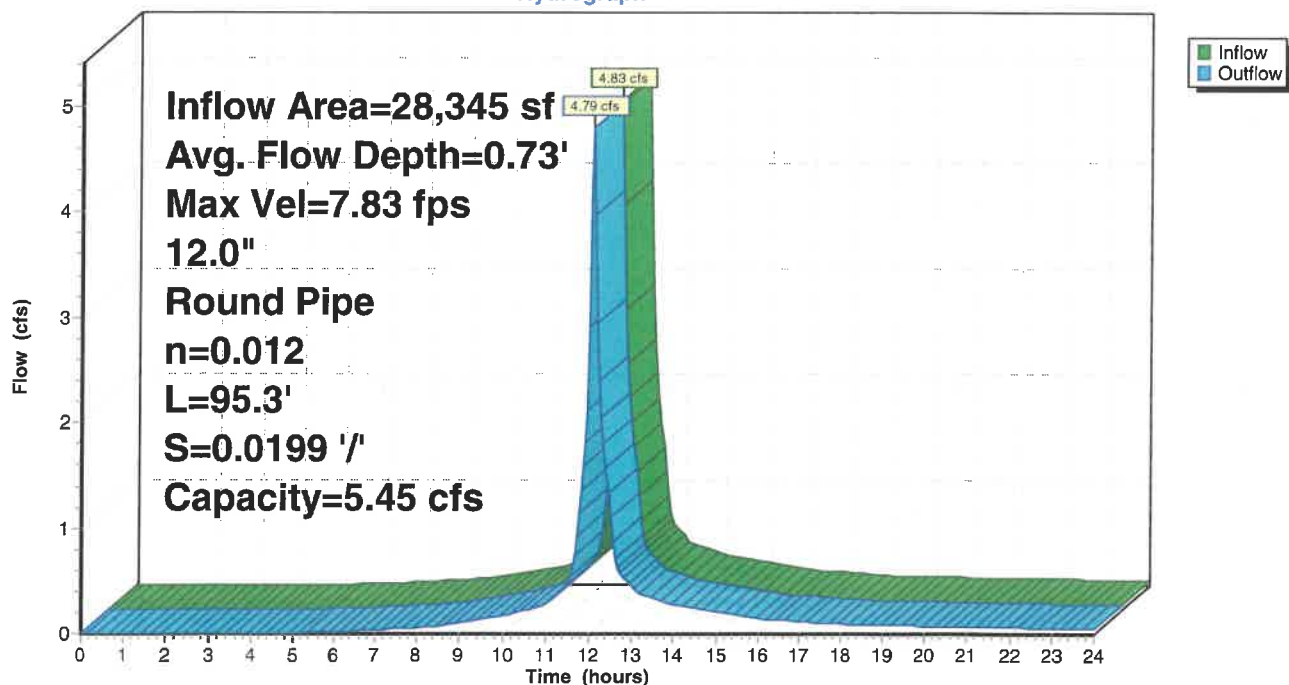
Peak Storage= 59 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.73', Surface Width= 0.89'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.45 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 95.3' Slope= 0.0199 '/'
Inlet Invert= 310.25', Outlet Invert= 308.35'



Reach 17R: DMH 6 TO DMH 5

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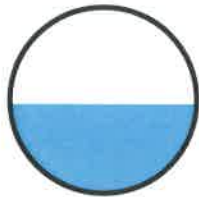
Summary for Reach 18R: CB 10 TO DMH 6

Inflow Area = 14,794 sf, 56.33% Impervious, Inflow Depth > 6.57" for 100 YR event
Inflow = 2.49 cfs @ 12.09 hrs, Volume= 8,099 cf
Outflow = 2.49 cfs @ 12.09 hrs, Volume= 8,099 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.70 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.30 fps, Avg. Travel Time= 0.1 min

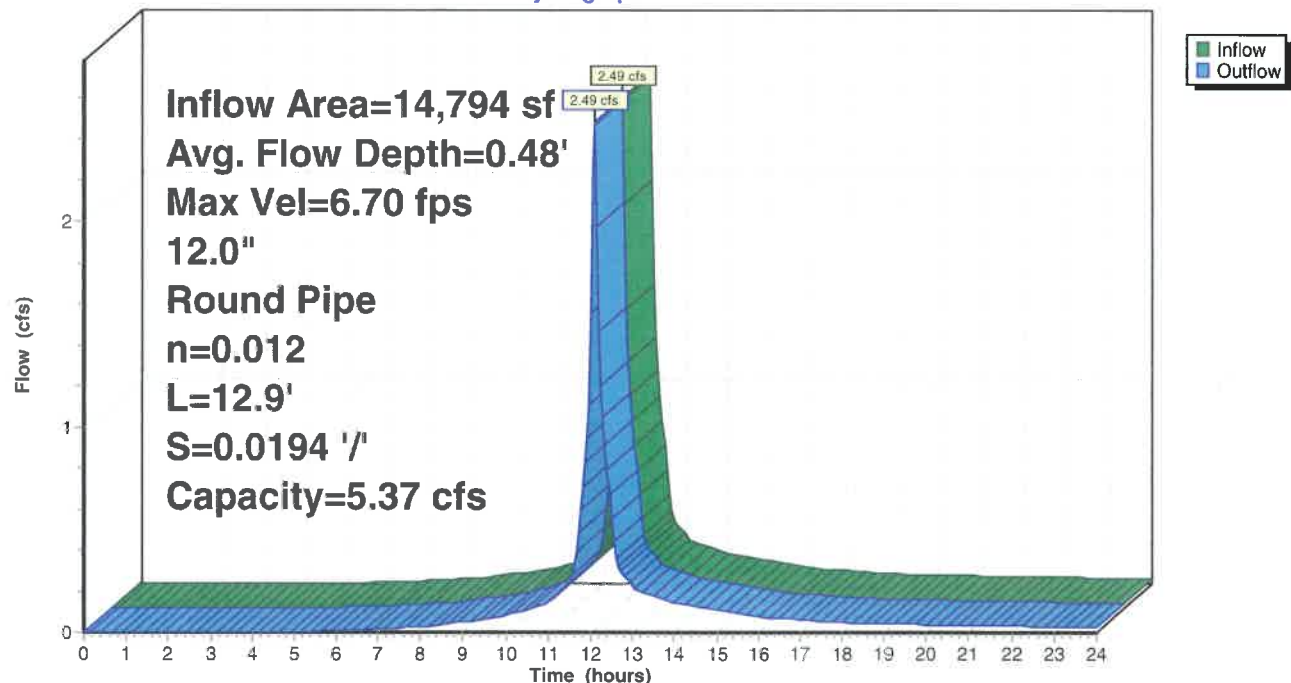
Peak Storage= 5 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.48' , Surface Width= 1.00'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.37 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 12.9' Slope= 0.0194 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 18R: CB 10 TO DMH 6

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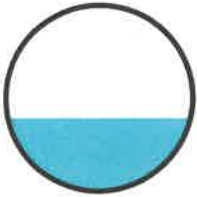
Summary for Reach 19R: CB 9 TO DMH 6

Inflow Area = 13,551 sf, 61.49% Impervious, Inflow Depth > 6.81" for 100 YR event
Inflow = 2.34 cfs @ 12.09 hrs, Volume= 7,693 cf
Outflow = 2.34 cfs @ 12.09 hrs, Volume= 7,693 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.25 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 2.78 fps, Avg. Travel Time= 0.0 min

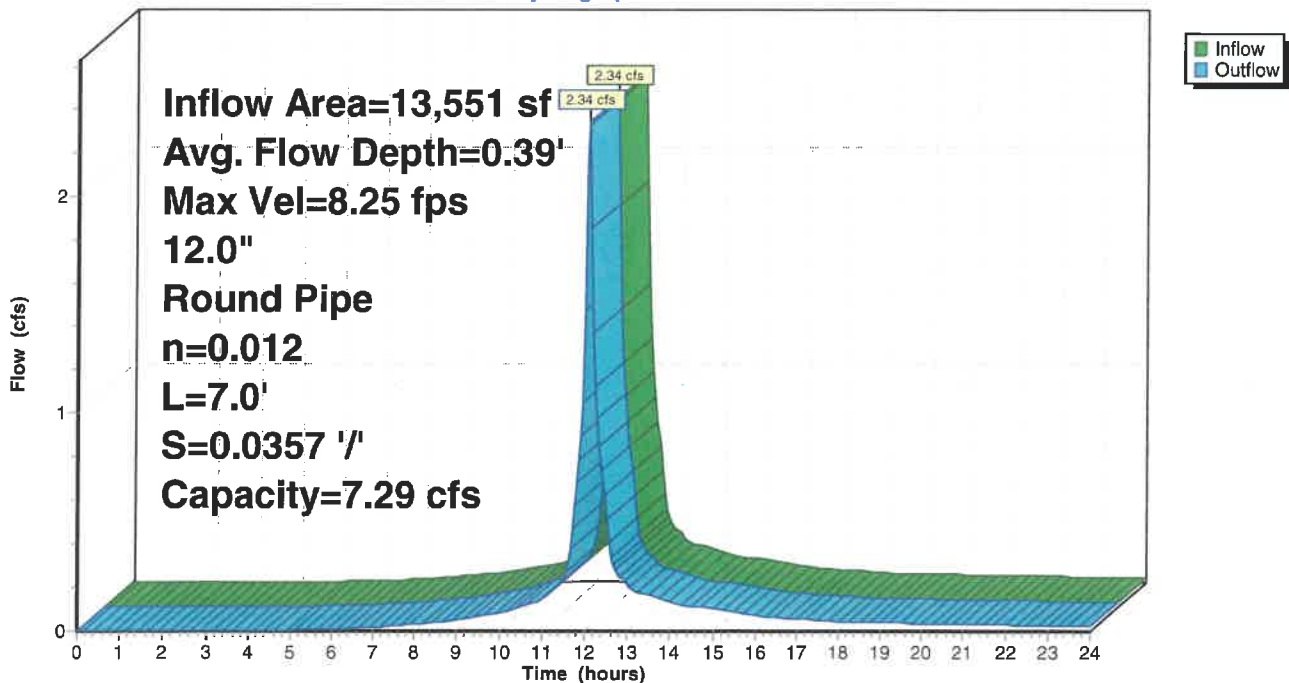
Peak Storage= 2 cf @ 12.09 hrs
Average Depth at Peak Storage= 0.39' , Surface Width= 0.98'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.29 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.0' Slope= 0.0357 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 19R: CB 9 TO DMH 6

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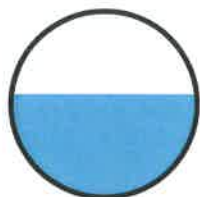
Summary for Reach 20R: DMH 6 to Basin

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 6.79" for 100 YR event
Inflow = 6.09 cfs @ 12.11 hrs, Volume= 20,816 cf
Outflow = 6.08 cfs @ 12.11 hrs, Volume= 20,815 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.69 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 2.95 fps, Avg. Travel Time= 0.2 min

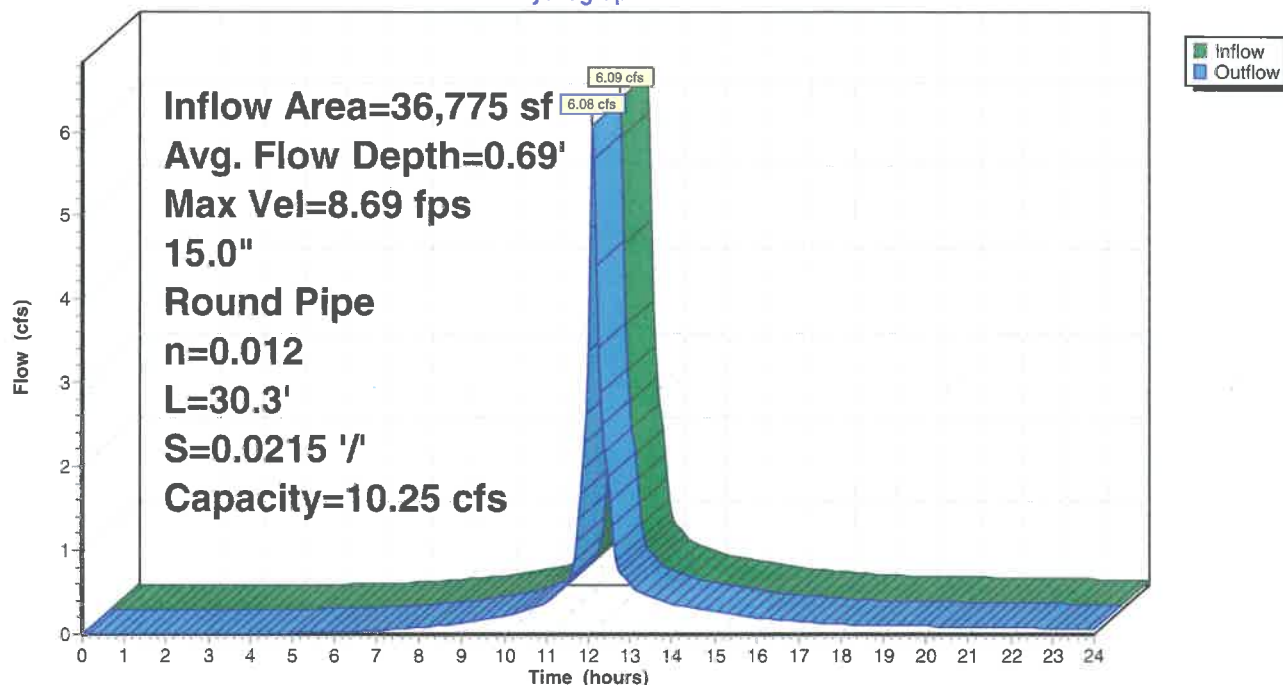
Peak Storage= 21 cf @ 12.11 hrs
Average Depth at Peak Storage= 0.69' , Surface Width= 1.24'
Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 10.25 cfs

15.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 30.3' Slope= 0.0215 '/'
Inlet Invert= 305.65', Outlet Invert= 305.00'



Reach 20R: DMH 6 to Basin

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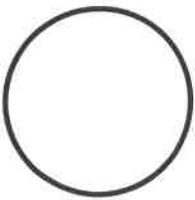
Summary for Reach 21R: 2-8" DI

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth = 0.00" for 100 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

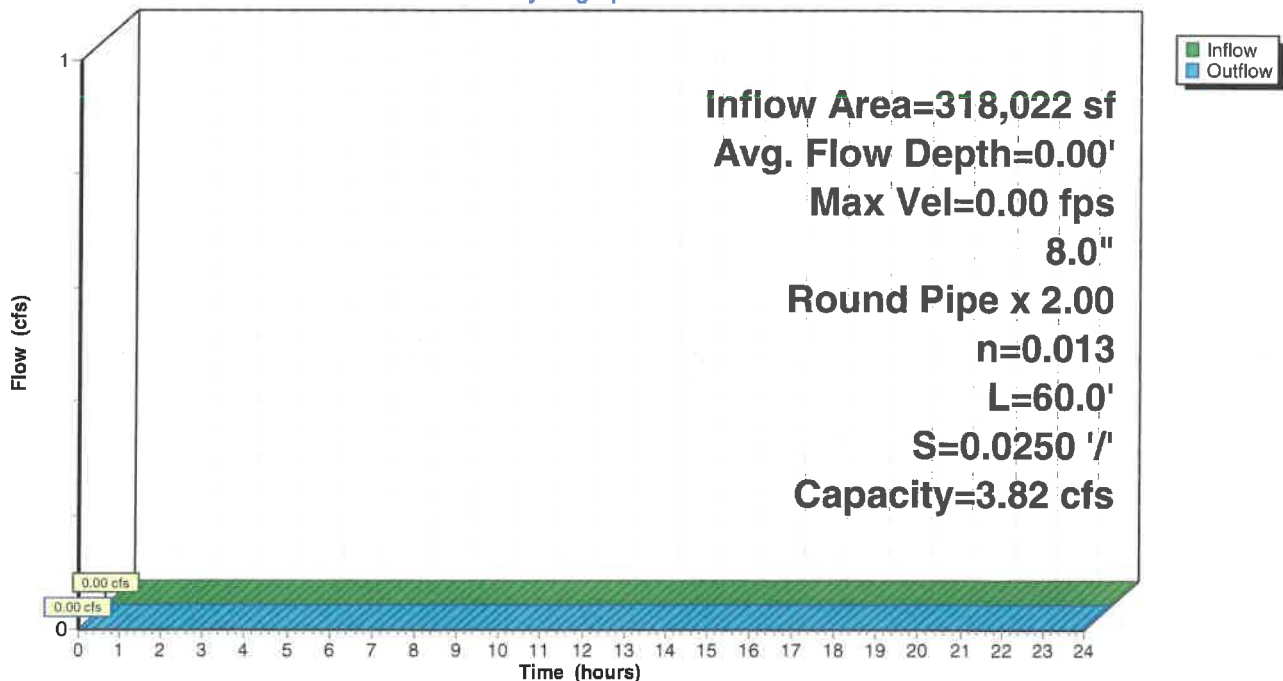
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.67' Flow Area= 0.7 sf, Capacity= 3.82 cfs

A factor of 2.00 has been applied to the storage and discharge capacity
8.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 60.0' Slope= 0.0250 '/'
Inlet Invert= 312.50', Outlet Invert= 311.00'



Reach 21R: 2-8" DI

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Summary for Reach 22R: Basin 2 Outfall

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 3.09" for 100 YR event
Inflow = 4.34 cfs @ 12.50 hrs, Volume= 29,907 cf
Outflow = 4.34 cfs @ 12.50 hrs, Volume= 29,905 cf, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.42 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 3.95 fps, Avg. Travel Time= 0.2 min

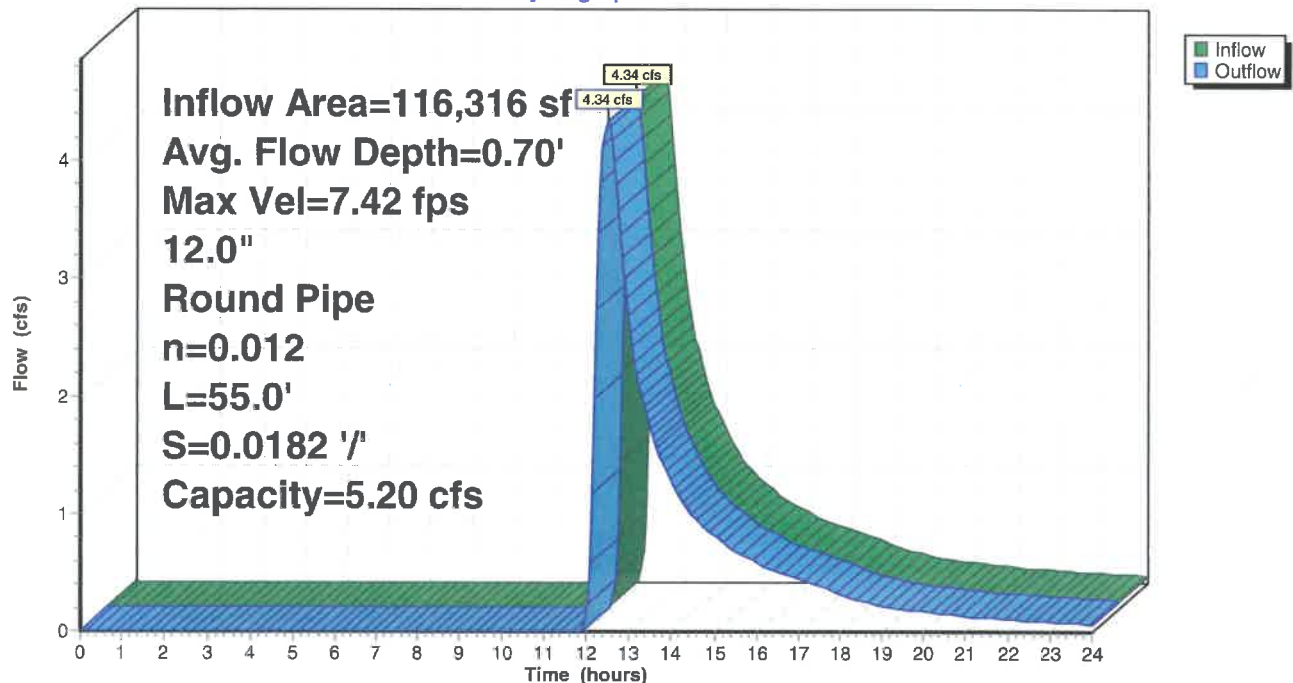
Peak Storage= 32 cf @ 12.50 hrs
Average Depth at Peak Storage= 0.70' , Surface Width= 0.92'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.20 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 55.0' Slope= 0.0182 '/'
Inlet Invert= 304.00', Outlet Invert= 303.00'



Reach 22R: Basin 2 Outfall

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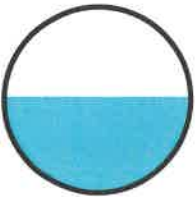
Summary for Reach 23R: Basin 1 Outfall

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 2.63" for 100 YR event
Inflow = 2.89 cfs @ 12.62 hrs, Volume= 27,199 cf
Outflow = 2.89 cfs @ 12.63 hrs, Volume= 27,196 cf, Atten= 0%, Lag= 0.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.05 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 4.15 fps, Avg. Travel Time= 0.2 min

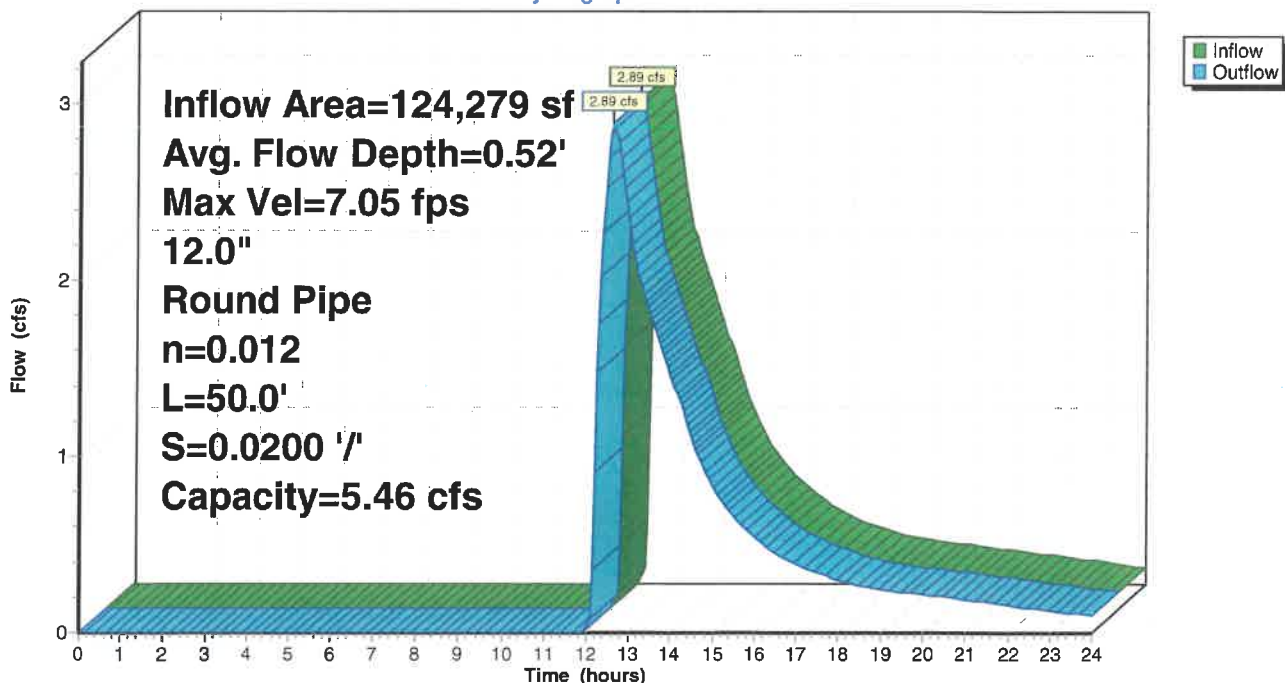
Peak Storage= 20 cf @ 12.63 hrs
Average Depth at Peak Storage= 0.52' , Surface Width= 1.00'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.46 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 50.0' Slope= 0.0200 '/'
Inlet Invert= 281.00', Outlet Invert= 280.00'



Reach 23R: Basin 1 Outfall

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Summary for Reach 24R: Basin 3 Outfall

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 4.14" for 100 YR event
Inflow = 5.42 cfs @ 12.21 hrs, Volume= 19,204 cf
Outflow = 5.44 cfs @ 12.21 hrs, Volume= 19,203 cf, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.25 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 3.64 fps, Avg. Travel Time= 0.2 min

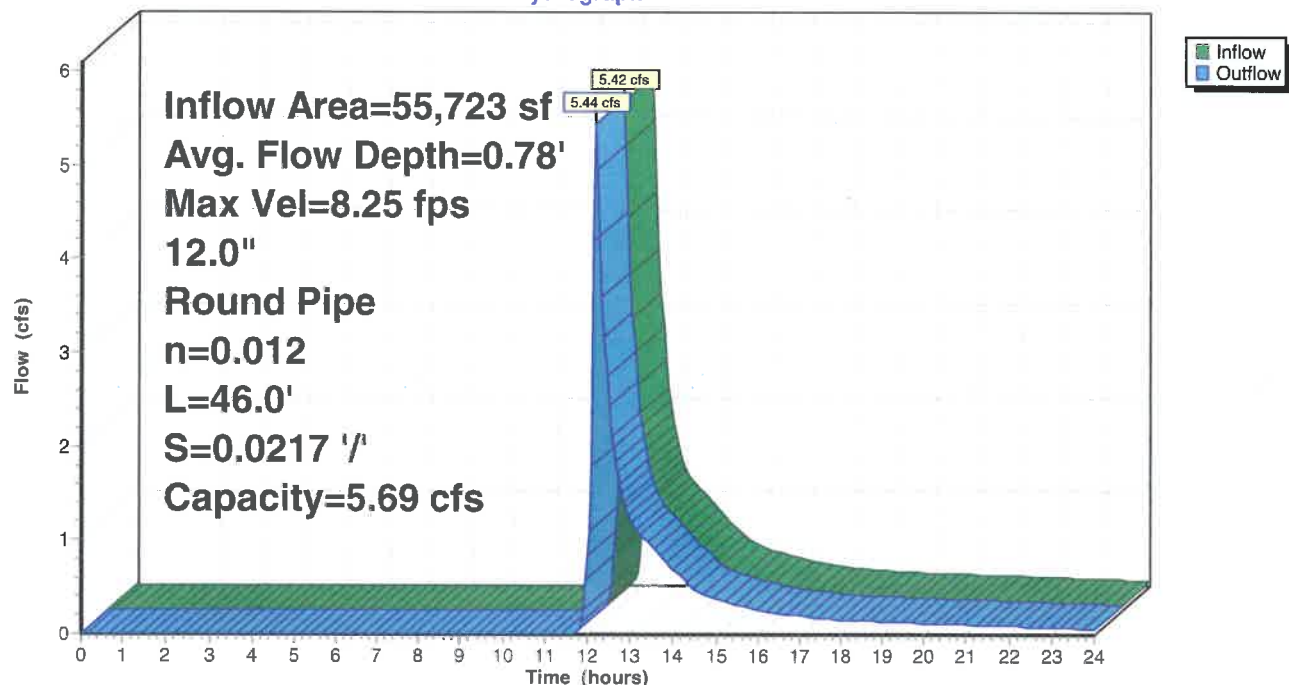
Peak Storage= 30 cf @ 12.21 hrs
Average Depth at Peak Storage= 0.78' , Surface Width= 0.82'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.69 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 46.0' Slope= 0.0217 '/'
Inlet Invert= 289.00', Outlet Invert= 288.00'



Reach 24R: Basin 3 Outfall

Hydrograph



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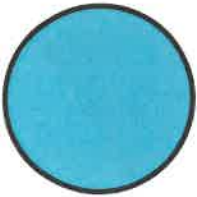
Summary for Reach 25R: 24" Bypass Culvert

Inflow Area = 492,614 sf, 9.97% Impervious, Inflow Depth > 4.00" for 100 YR event
Inflow = 30.97 cfs @ 12.40 hrs, Volume= 164,258 cf
Outflow = 30.26 cfs @ 12.35 hrs, Volume= 164,239 cf, Atten= 2%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 10.27 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 4.98 fps, Avg. Travel Time= 0.3 min

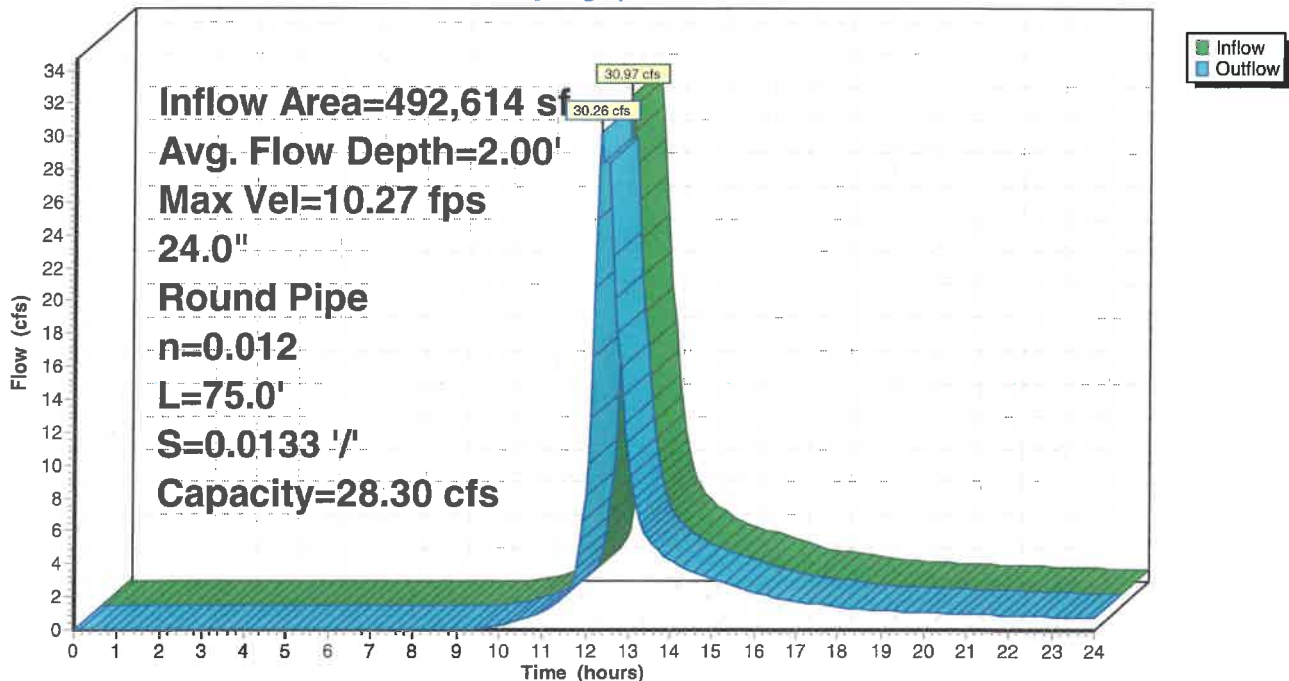
Peak Storage= 236 cf @ 12.40 hrs
Average Depth at Peak Storage= 2.00'
Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 28.30 cfs

24.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 75.0' Slope= 0.0133 '/'
Inlet Invert= 292.50', Outlet Invert= 291.50'



Reach 25R: 24" Bypass Culvert

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Summary for Reach EV1: Road Berm

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 3.53" for 100 YR event
Inflow = 135.67 cfs @ 12.67 hrs, Volume= 809,588 cf
Outflow = 134.84 cfs @ 12.67 hrs, Volume= 809,562 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.42 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 2.62 fps, Avg. Travel Time= 0.1 min

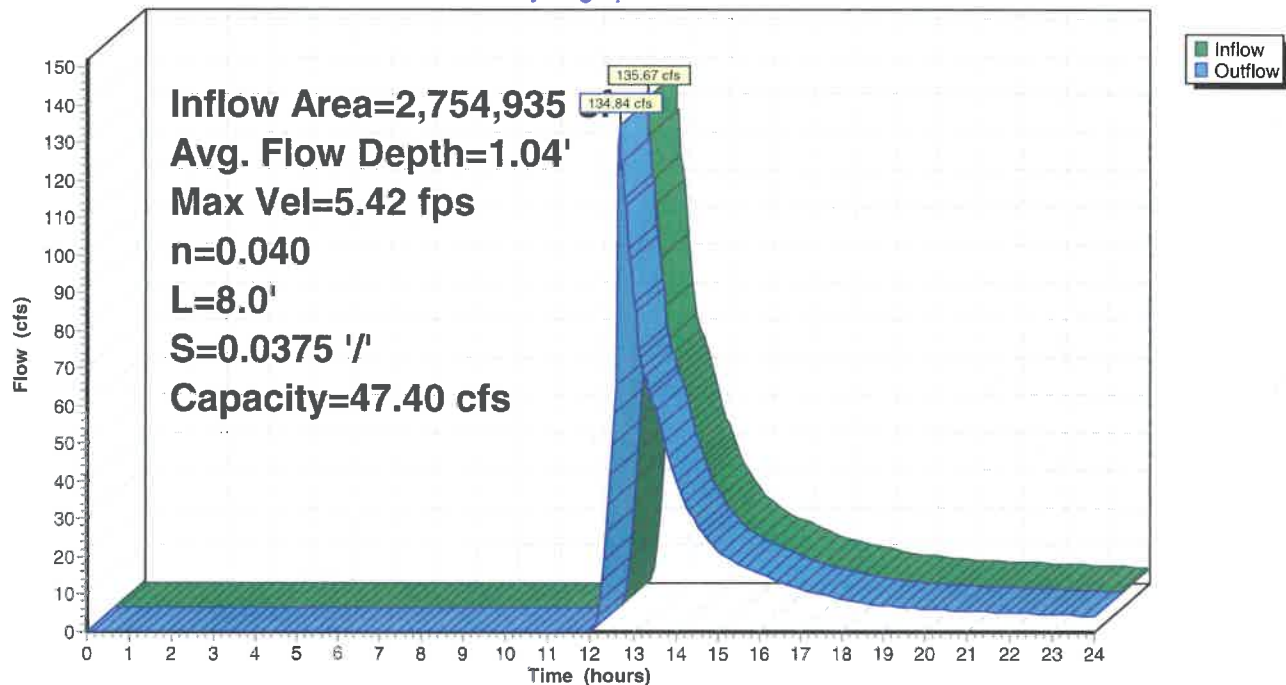
Peak Storage= 198 cf @ 12.67 hrs
Average Depth at Peak Storage= 1.04' , Surface Width= 30.42'
Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 47.40 cfs

20.00' x 0.50' deep channel, n= 0.040 Earth, cobble bottom, clean sides
Side Slope Z-value= 5.0 ' ' Top Width= 25.00'
Length= 8.0' Slope= 0.0375 ' '
Inlet Invert= 293.90', Outlet Invert= 293.60'



Reach EV1: Road Berm

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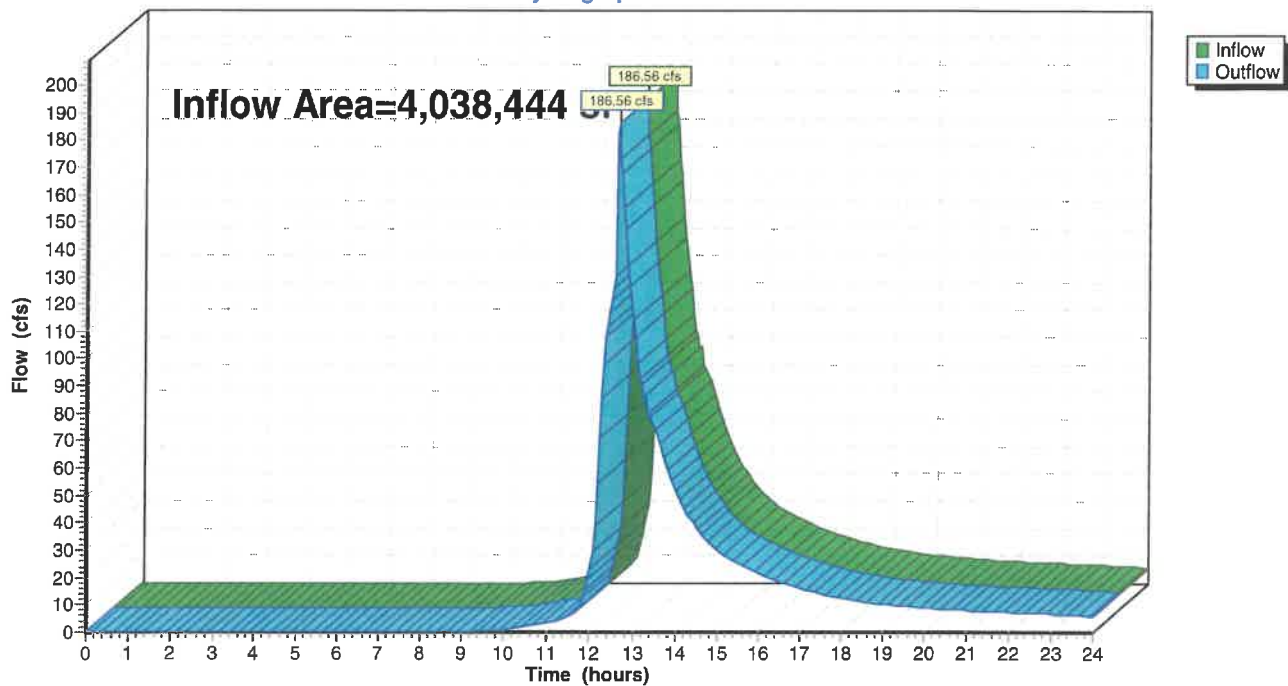
Summary for Reach EV2: To Wetland

Inflow Area = 4,038,444 sf, 4.14% Impervious, Inflow Depth > 3.71" for 100 YR event
Inflow = 186.56 cfs @ 12.67 hrs, Volume= 1,248,553 cf
Outflow = 186.56 cfs @ 12.67 hrs, Volume= 1,248,553 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV2: To Wetland

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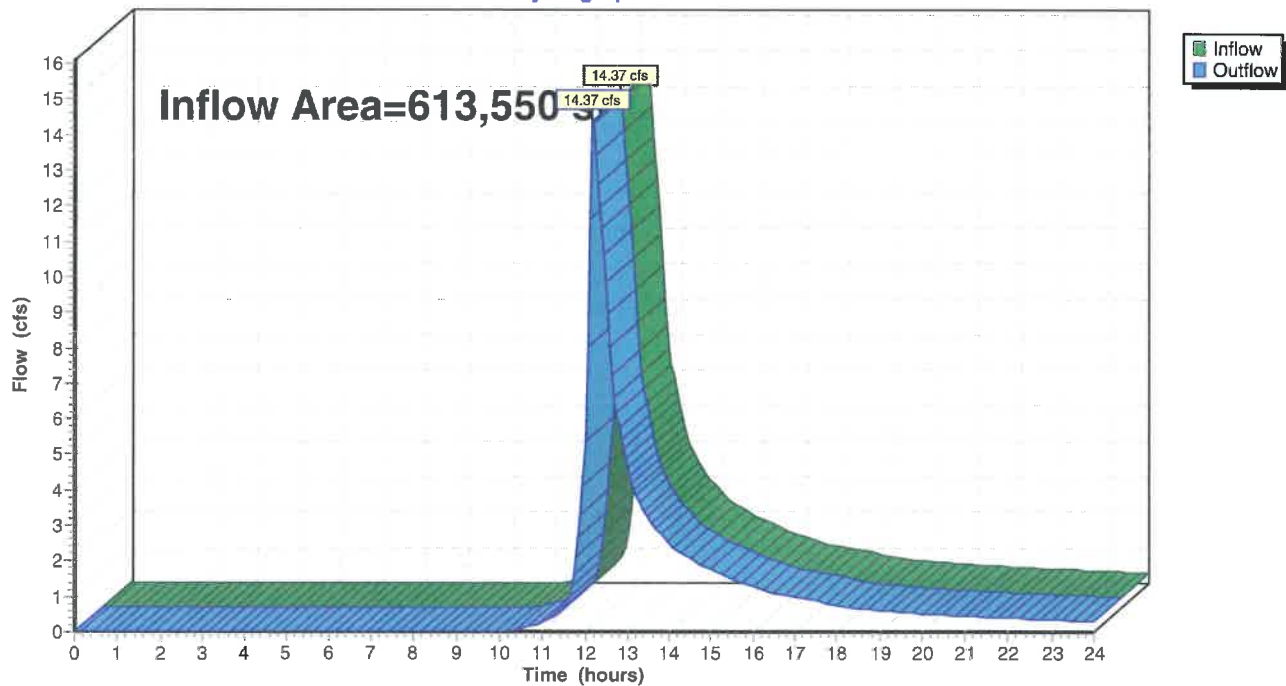
Summary for Reach EV3: To Offsite

Inflow Area = 613,550 sf, 3.69% Impervious, Inflow Depth > 1.55" for 100 YR event
Inflow = 14.37 cfs @ 12.18 hrs, Volume= 79,198 cf
Outflow = 14.37 cfs @ 12.18 hrs, Volume= 79,198 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach EV3: To Offsite

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Summary for Pond 1P: Forebay

Inflow Area = 12,862 sf, 75.80% Impervious, Inflow Depth > 7.44" for 100 YR event
 Inflow = 2.19 cfs @ 12.13 hrs, Volume= 7,971 cf
 Outflow = 2.15 cfs @ 12.14 hrs, Volume= 6,492 cf, Atten= 2%, Lag= 1.0 min
 Primary = 2.15 cfs @ 12.14 hrs, Volume= 6,492 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 282.62' @ 12.14 hrs Surf.Area= 1,307 sf Storage= 1,640 cf

Plug-Flow detention time= 121.0 min calculated for 6,478 cf (81% of inflow)
 Center-of-Mass det. time= 50.4 min (828.4 - 778.0)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 281.00' | 3,838 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 281.00 | 737 | 101.0 | 0 | 0 | 737 |
| 282.00 | 1,071 | 120.0 | 899 | 899 | 1,089 |
| 284.00 | 1,908 | 158.0 | 2,939 | 3,838 | 1,974 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 282.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=2.11 cfs @ 12.14 hrs HW=282.62' (Free Discharge)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 2.11 cfs @ 0.85 fps)

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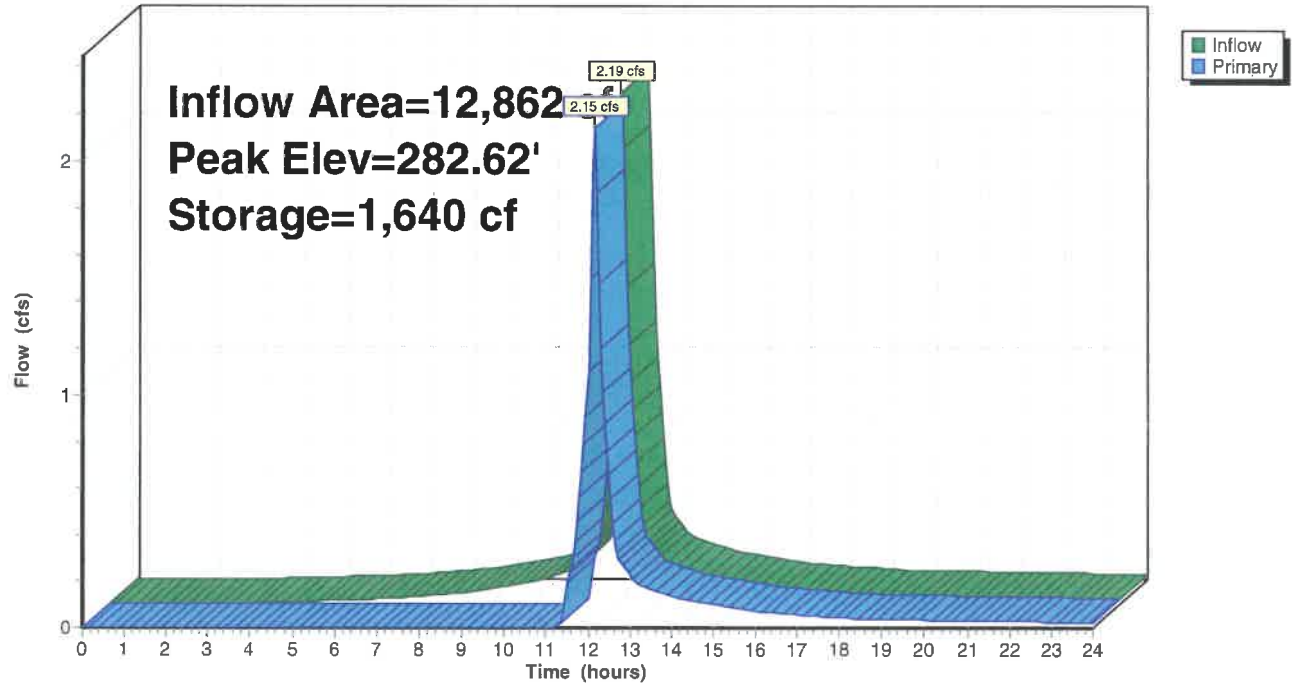
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Pond 1P: Forebay

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Summary for Pond 2P: Basin 1

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 3.69" for 100 YR event
 Inflow = 9.85 cfs @ 12.18 hrs, Volume= 38,223 cf
 Outflow = 3.04 cfs @ 12.62 hrs, Volume= 33,182 cf, Atten= 69%, Lag= 26.6 min
 Discarded = 0.16 cfs @ 12.62 hrs, Volume= 5,983 cf
 Primary = 2.89 cfs @ 12.62 hrs, Volume= 27,199 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 283.94' @ 12.62 hrs Surf.Area= 6,002 sf Storage= 14,160 cf

Plug-Flow detention time= 132.8 min calculated for 33,182 cf (87% of inflow)
 Center-of-Mass det. time= 73.6 min (924.2 - 850.6)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 281.00' | 51,257 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 281.00 | 3,708 | 232.0 | 0 | 0 | 3,708 |
| 282.00 | 4,433 | 251.0 | 4,065 | 4,065 | 4,477 |
| 284.00 | 6,052 | 288.0 | 10,443 | 14,508 | 6,153 |
| 286.00 | 7,890 | 326.0 | 13,901 | 28,410 | 8,109 |
| 288.00 | 9,968 | 364.0 | 17,818 | 46,227 | 10,306 |
| 288.50 | 10,153 | 374.0 | 5,030 | 51,257 | 10,922 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 287.50' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 281.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 281.00' | 12.0" Round Culvert L= 44.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 281.00' / 280.00' S= 0.0227 ' S Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 282.00' | 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 282.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 283.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 286.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Discarded OutFlow Max=0.16 cfs @ 12.62 hrs HW=283.94' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.16 cfs)

Primary OutFlow Max=2.88 cfs @ 12.62 hrs HW=283.94' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 2.88 cfs of 5.21 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.56 cfs @ 6.41 fps)

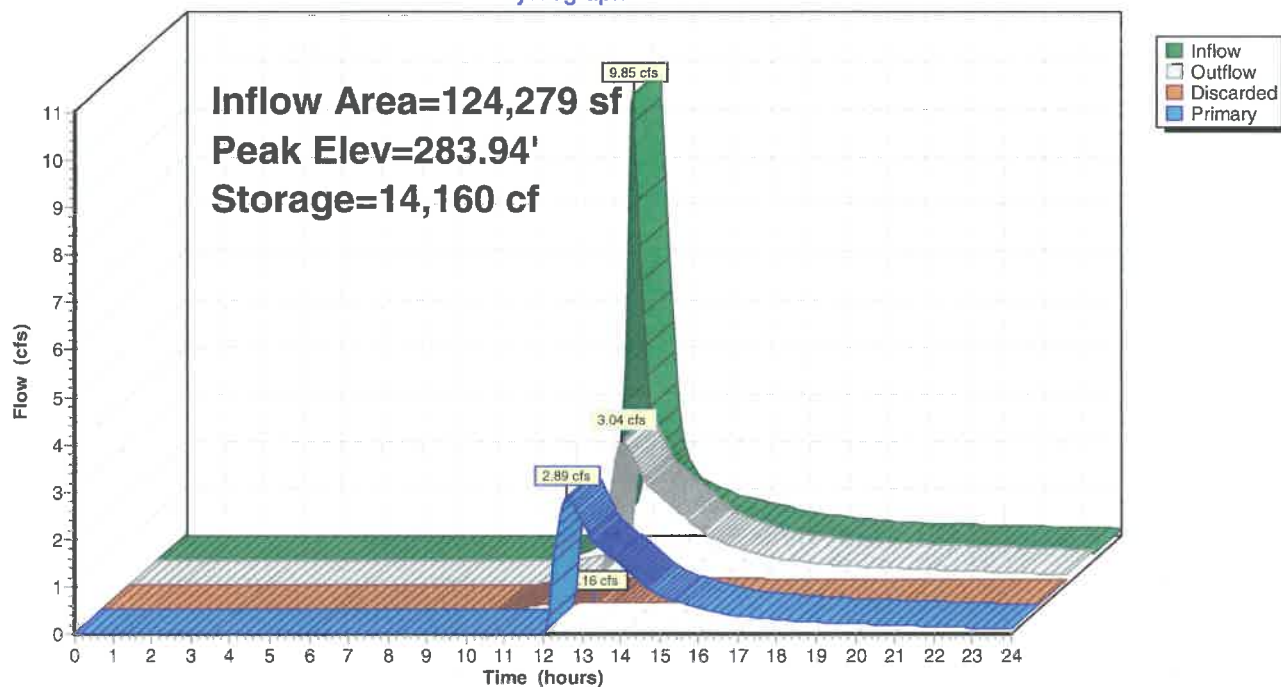
↳ **5=Orifice/Grate** (Orifice Controls 1.77 cfs @ 5.07 fps)

↳ **6=Orifice/Grate** (Orifice Controls 0.55 cfs @ 2.26 fps)

↳ **7=Orifice/Grate** (Controls 0.00 cfs)

Pond 2P: Basin 1

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Summary for Pond 3P: Forebay

Inflow Area = 36,775 sf, 61.57% Impervious, Inflow Depth > 6.79" for 100 YR event
 Inflow = 6.08 cfs @ 12.11 hrs, Volume= 20,815 cf
 Outflow = 5.96 cfs @ 12.13 hrs, Volume= 18,598 cf, Atten= 2%, Lag= 1.1 min
 Primary = 5.96 cfs @ 12.13 hrs, Volume= 18,598 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.74' @ 12.13 hrs Surf.Area= 1,921 sf Storage= 2,660 cf

Plug-Flow detention time= 80.7 min calculated for 18,598 cf (89% of inflow)
 Center-of-Mass det. time= 30.6 min (822.3 - 791.7)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 10,058 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 1,160 | 129.0 | 0 | 0 | 1,160 |
| 306.00 | 2,048 | 166.0 | 3,166 | 3,166 | 2,077 |
| 308.00 | 3,164 | 204.0 | 5,172 | 8,338 | 3,256 |
| 308.50 | 3,725 | 227.0 | 1,720 | 10,058 | 4,052 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 305.50' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=5.80 cfs @ 12.13 hrs HW=305.74' (Free Discharge)

↑ **1=Broad-Crested Rectangular Weir** (Weir Controls 5.80 cfs @ 1.20 fps)

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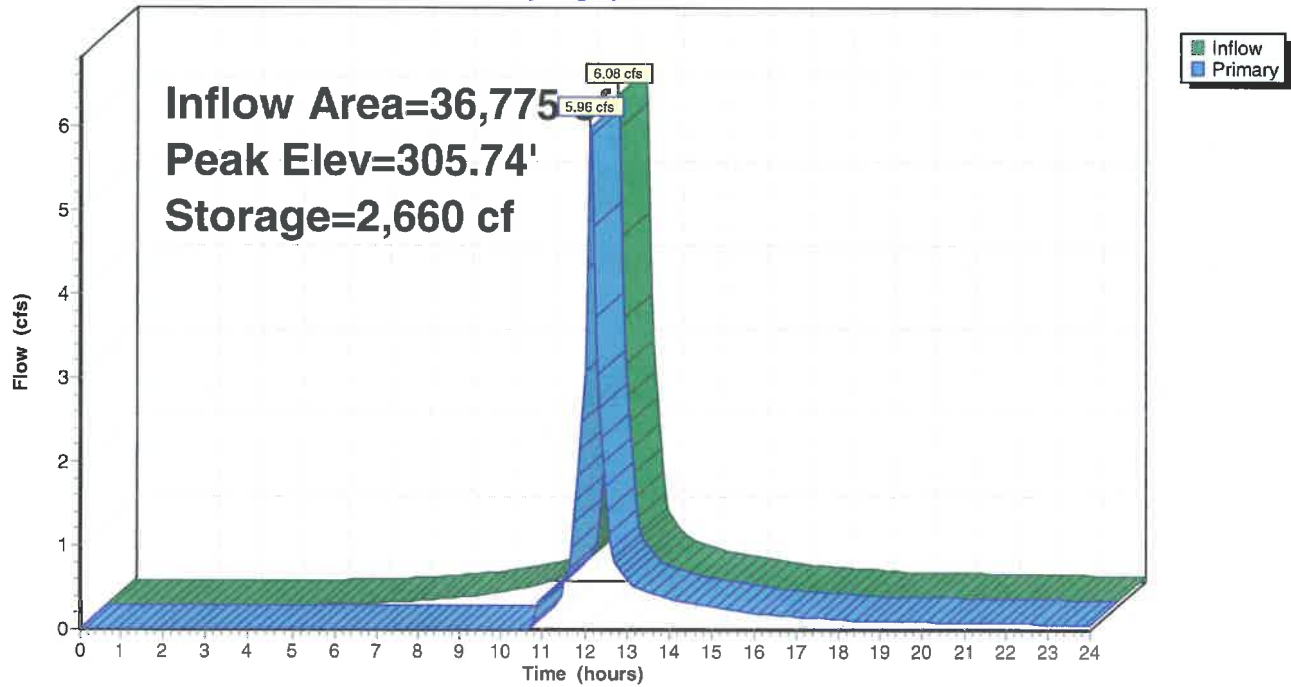
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Pond 3P: Forebay

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Summary for Pond 4P: Basin 2

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 4.42" for 100 YR event
 Inflow = 12.06 cfs @ 12.15 hrs, Volume= 42,837 cf
 Outflow = 4.52 cfs @ 12.50 hrs, Volume= 37,080 cf, Atten= 62%, Lag= 21.0 min
 Discarded = 0.18 cfs @ 12.50 hrs, Volume= 7,173 cf
 Primary = 4.34 cfs @ 12.50 hrs, Volume= 29,907 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 306.69' @ 12.50 hrs Surf.Area= 7,146 sf Storage= 15,587 cf

Plug-Flow detention time= 124.3 min calculated for 37,080 cf (87% of inflow)
 Center-of-Mass det. time= 64.8 min (902.7 - 837.9)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 30,313 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 4,504 | 302.0 | 0 | 0 | 4,504 |
| 306.00 | 6,434 | 340.0 | 10,881 | 10,881 | 6,549 |
| 308.00 | 8,590 | 378.0 | 14,972 | 25,853 | 8,836 |
| 308.50 | 9,256 | 397.0 | 4,460 | 30,313 | 10,023 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 307.70' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 304.00' | 1.100 in/hr Exfiltration over Surface area |
| #3 | Primary | 304.00' | 12.0" Round Culvert L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 304.00' / 303.00' S= 0.0286 ' S= 0.0286 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 305.00' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 305.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 306.00' | 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

Discarded OutFlow Max=0.18 cfs @ 12.50 hrs HW=306.69' (Free Discharge)

2=Exfiltration (Exfiltration Controls 0.18 cfs)

Primary OutFlow Max=4.34 cfs @ 12.50 hrs HW=306.69' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

3=Culvert (Passes 4.34 cfs of 4.94 cfs potential flow)

4=Orifice/Grate (Orifice Controls 1.14 cfs @ 5.78 fps)

5=Orifice/Grate (Orifice Controls 1.56 cfs @ 4.47 fps)

6=Orifice/Grate (Orifice Controls 1.65 cfs @ 2.84 fps)

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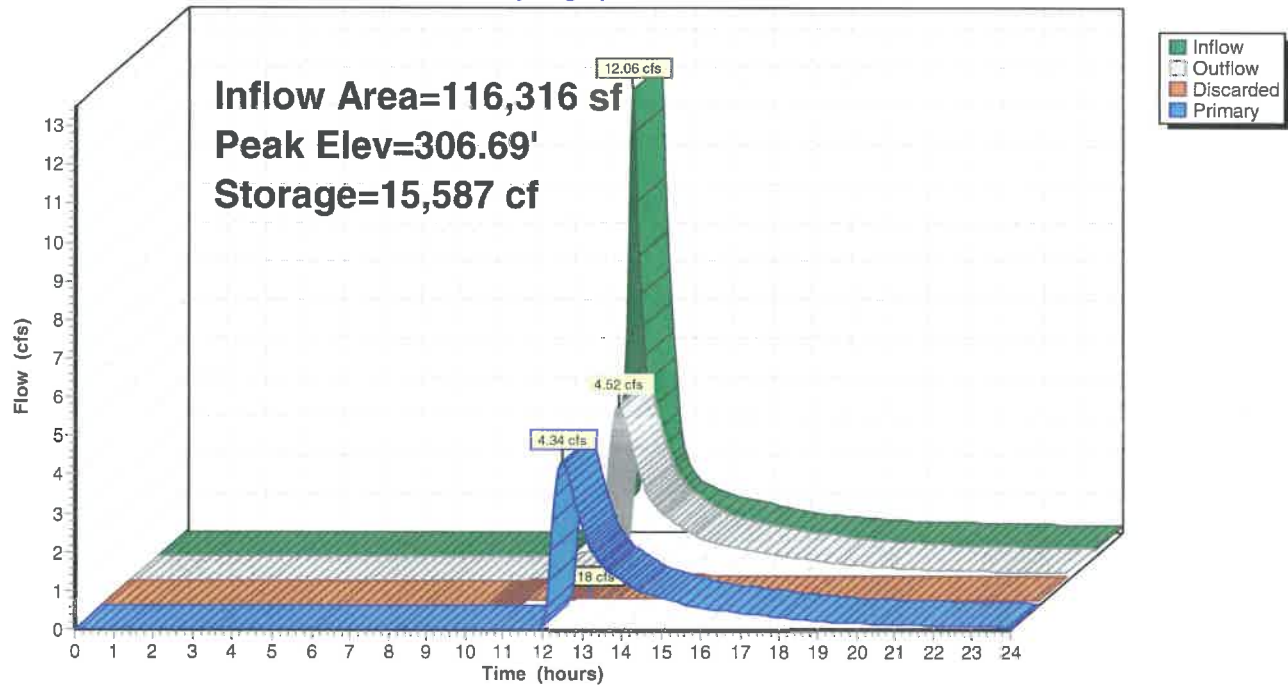
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Pond 4P: Basin 2

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Summary for Pond 5P: Forebay

Inflow Area = 24,413 sf, 89.32% Impervious, Inflow Depth > 8.00" for 100 YR event
 Inflow = 4.59 cfs @ 12.09 hrs, Volume= 16,283 cf
 Outflow = 4.39 cfs @ 12.11 hrs, Volume= 12,421 cf, Atten= 4%, Lag= 1.1 min
 Primary = 4.39 cfs @ 12.11 hrs, Volume= 12,421 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 2
 Peak Elev= 291.40' @ 12.11 hrs Surf.Area= 2,357 sf Storage= 4,311 cf

Plug-Flow detention time= 150.3 min calculated for 12,395 cf (76% of inflow)
 Center-of-Mass det. time= 68.7 min (827.9 - 759.2)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 289.00' | 5,821 cf | Forebay (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 289.00 | 1,270 | 132.0 | 0 | 0 | 1,270 |
| 290.00 | 1,710 | 153.0 | 1,485 | 1,485 | 1,767 |
| 291.00 | 2,145 | 172.0 | 1,923 | 3,408 | 2,285 |
| 292.00 | 2,692 | 191.0 | 2,413 | 5,821 | 2,863 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 291.20' | 20.0' long x 8.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.43 2.54 2.70 2.69 2.68 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.66 2.66 2.68 2.70 2.74 |

Primary OutFlow Max=4.29 cfs @ 12.11 hrs HW=291.40' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 4.29 cfs @ 1.08 fps)

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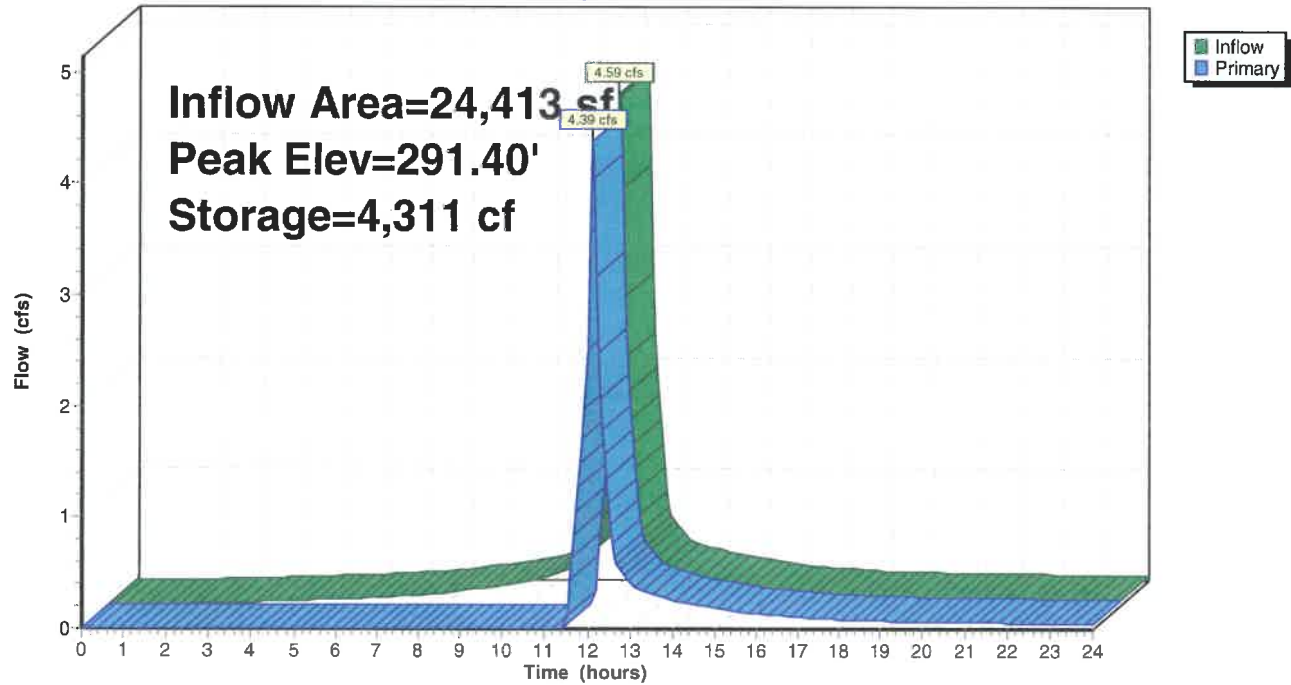
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Pond 5P: Forebay

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Summary for Pond 6P: Basin 3

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 4.93" for 100 YR event
 Inflow = 7.67 cfs @ 12.10 hrs, Volume= 22,915 cf
 Outflow = 5.50 cfs @ 12.21 hrs, Volume= 21,522 cf, Atten= 28%, Lag= 6.4 min
 Discarded = 0.08 cfs @ 12.21 hrs, Volume= 2,318 cf
 Primary = 5.42 cfs @ 12.21 hrs, Volume= 19,204 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 292.24' @ 12.21 hrs Surf.Area= 2,937 sf Storage= 5,947 cf

Plug-Flow detention time= 71.2 min calculated for 21,522 cf (94% of inflow)
 Center-of-Mass det. time= 39.4 min (872.3 - 832.9)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 289.00' | 12,306 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 289.00 | 840 | 185.0 | 0 | 0 | 840 |
| 290.00 | 1,423 | 204.0 | 1,119 | 1,119 | 1,459 |
| 291.00 | 2,065 | 223.0 | 1,734 | 2,853 | 2,139 |
| 292.00 | 2,763 | 242.0 | 2,406 | 5,258 | 2,880 |
| 293.00 | 3,517 | 260.0 | 3,132 | 8,391 | 3,642 |
| 294.00 | 4,328 | 279.0 | 3,915 | 12,306 | 4,500 |

| Device | Routing | Invert | Outlet Devices |
|--------|-----------|---------|--|
| #1 | Primary | 293.25' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Discarded | 289.00' | 1.100 in/hr Exfiltration over Wetted area |
| #3 | Primary | 289.00' | 12.0" Round Culvert L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 289.00' / 288.00' S= 0.0222 ' S= 0.0222 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #4 | Device 3 | 290.00' | 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 3 | 290.50' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 3 | 291.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #7 | Device 3 | 292.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

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Type III 24-hr 100 YR Rainfall=8.75"

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Discarded OutFlow Max=0.08 cfs @ 12.21 hrs HW=292.23' (Free Discharge)

↳ **2=Exfiltration** (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=5.51 cfs @ 12.21 hrs HW=292.23' (Free Discharge)

↳ **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

↳ **3=Culvert** (Passes 5.51 cfs of 5.52 cfs potential flow)

↳ **4=Orifice/Grate** (Orifice Controls 0.34 cfs @ 6.99 fps)

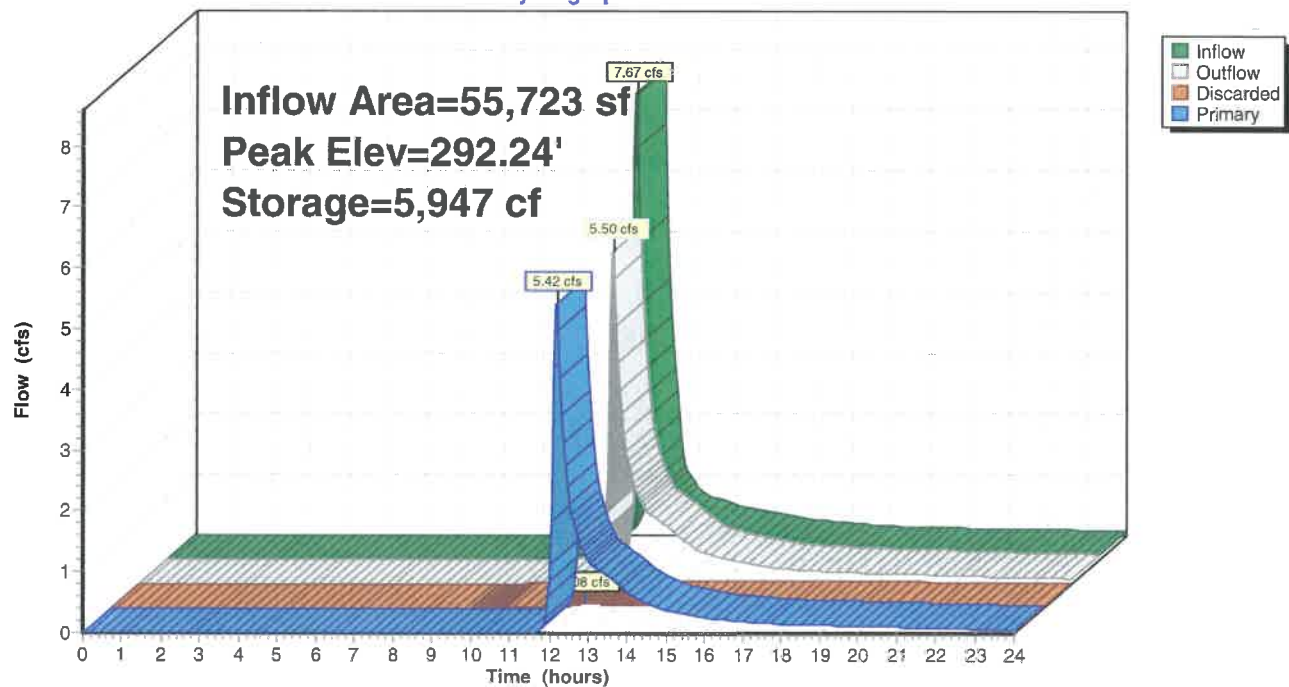
↳ **5=Orifice/Grate** (Orifice Controls 1.15 cfs @ 5.87 fps)

↳ **6=Orifice/Grate** (Orifice Controls 1.06 cfs @ 3.05 fps)

↳ **7=Orifice/Grate** (Weir Controls 2.96 cfs @ 1.58 fps)

Pond 6P: Basin 3

Hydrograph



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Summary for Pond 7P: Vernal Pool and Wetland

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 3.74" for 100 YR event
 Inflow = 126.66 cfs @ 12.65 hrs, Volume= 859,736 cf
 Outflow = 135.67 cfs @ 12.67 hrs, Volume= 809,588 cf, Atten= 0%, Lag= 1.0 min
 Primary = 135.67 cfs @ 12.67 hrs, Volume= 809,588 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 297.27' @ 12.67 hrs Surf.Area= 70,541 sf Storage= 143,915 cf

Plug-Flow detention time= 56.9 min calculated for 807,904 cf (94% of inflow)
 Center-of-Mass det. time= 28.0 min (902.1 - 874.1)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 291.30' | 143,915 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 291.30 | 3,358 | 0 | 0 |
| 292.20 | 10,815 | 6,378 | 6,378 |
| 294.00 | 32,324 | 38,825 | 45,203 |
| 295.00 | 47,280 | 39,802 | 85,005 |
| 296.00 | 70,541 | 58,911 | 143,915 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|---|
| #1 | Primary | 293.80' | 8.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=127.01 cfs @ 12.67 hrs HW=297.11' (Free Discharge)
 ↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 127.01 cfs @ 4.80 fps)

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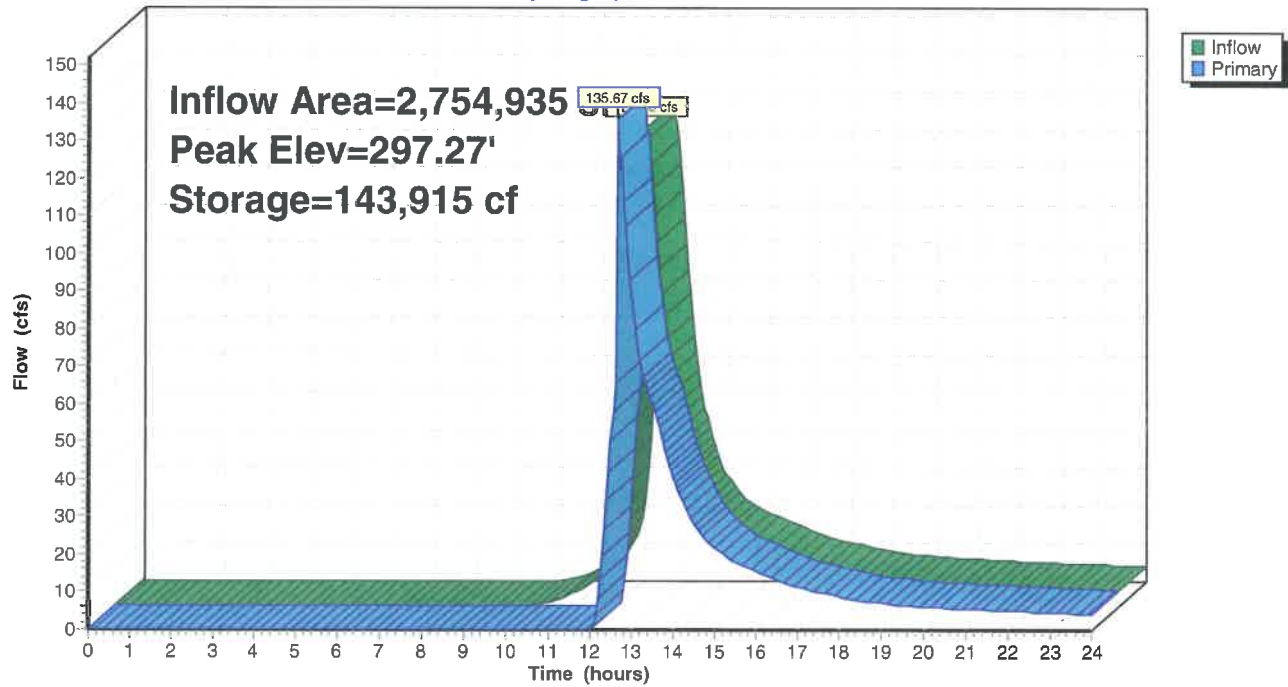
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Pond 7P: Vernal Pool and Wetland

Hydrograph



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Type III 24-hr 100 YR Rainfall=8.75"

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Summary for Pond 8P: Road Berm

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth > 3.29" for 100 YR event
 Inflow = 17.73 cfs @ 12.32 hrs, Volume= 87,225 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 312.15' @ 24.00 hrs Surf.Area= 69,589 sf Storage= 87,184 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= (not calculated: no outflow)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|---|
| #1 | 310.00' | 256,496 cf | Upgradient wetland (Prismatic) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
|---------------------|----------------------|---------------------------|---------------------------|
| 310.00 | 11,020 | 0 | 0 |
| 312.00 | 66,074 | 77,094 | 77,094 |
| 314.00 | 113,328 | 179,402 | 256,496 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 312.50' | 8.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=310.00' (Free Discharge)
 ↑ **1=Orifice/Grate** (Controls 0.00 cfs)

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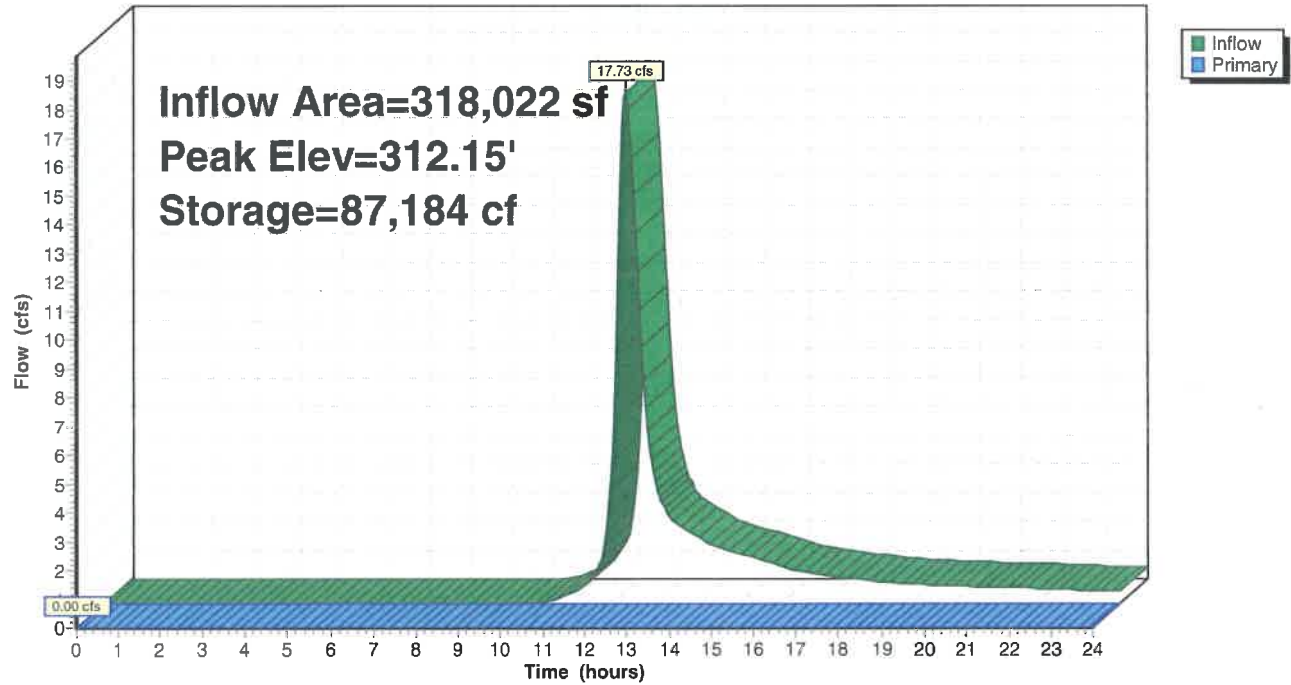
00454 - PROPOSED Conditions
Type III 24-hr 100 YR Rainfall=8.75"

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Pond 8P: Road Berm

Hydrograph



Stormwater Management



Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



 10/31/25
Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- ☐ New development
- ☐ Redevelopment
- ☒ Mix of New Development and Redevelopment



Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☐ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☐ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
 - ☐ Credit 1
 - ☐ Credit 2
 - ☐ Credit 3
- ☐ Use of "country drainage" versus curb and gutter conveyance and
- ☐ pipe Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands
- ☐ designs) Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): _____

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- ☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- ☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- ☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- ☐ Soil Analysis provided.
- ☒ Required Recharge Volume calculation provided.
- ☐ Required Recharge volume reduced through use of the LID site Design Credits.
- ☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - ☒ Static
 - ☐ Simple Dynamic
 - ☐ Dynamic Field¹
- ☒ Runoff from all impervious areas at the site discharging to the infiltration BMP.
- ☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- ☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- ☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
 - ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
 - ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- ☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- ☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- ☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- ☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- ☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - ☐ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - ☐ is within the Zone II or Interim Wellhead Protection Area
 - ☐ is near or to other critical areas
 - ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - ☐ involves runoff from land uses with higher potential pollutant loads.
 - ☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - ☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- ☐ The BMP is sized (and calculations provided) based on:
 - ☐ The ½" or 1" Water Quality Volume or
 - ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- ☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- ☒ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted *prior* to the discharge of stormwater to the post-construction stormwater BMPs.
- ☐ The NPDES Multi-Sector General Permit does *not* cover the land use.
- ☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- ☐ All exposure has been eliminated.
- ☐ All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- ☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- ☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- ☐ Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- ☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - ☐ Limited Project
 - ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - ☐ Bike Path and/or Foot Path
 - ☐ Redevelopment Project
 - ☐ Redevelopment portion of mix of new and redevelopment.
- ☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- ☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- ☐ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- ☐ The project is **not** covered by a NPDES Construction General Permit.
- ☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- ☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted **BEFORE** land disturbance begins.

Standard 9: Operation and Maintenance Plan

- ☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - ☒ Name of the stormwater management system owners;
 - ☒ Party responsible for operation and maintenance;
 - ☐ Schedule for implementation of routine and non-routine maintenance tasks;
 - ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
 - ☐ Description and delineation of public safety features;
 - ☐ Estimated operation and maintenance budget; and
 - ☐ Operation and Maintenance Log Form.
- ☐ The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- ☐ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- ☒ An Illicit Discharge Compliance Statement is attached;
- ☐ NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

STORMWATER MANAGEMENT COMPLIANCE

Standard #1 No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

- No new conveyances will discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth. The new stormwater discharges are treated and provided with hardened outfalls to avoid surface erosion.

Standard #2 Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for discharges to land subject to coastal storm flowage as defined in 310 CMR 10.04.

- See Table-Section 1, Page 1 - Summary of Peak Rates of Stormwater Runoff
Post-development peak discharge rates do not exceed existing peak discharge rates.

Standard #3 Loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures, including environmentally sensitive site design, low-impact development techniques, stormwater best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil type. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.

- Impervious area = proposed paved road, sidewalks, and houses
Flow to Infiltration Basin 1 (10-Yr Storm)
Impervious area HSG-B (0.35") = 26,755 sf = Req'd. recharge = 780.35 cf
Basin 1 Volume = 49,192 cf
Flow to Infiltration Basin 2
Impervious area HSG-B (0.35") = 22,644 sf = Req'd. recharge = 660.45 cf
Basin 2 Volume = 40,270 cf
Flow to Infiltration Basin 3
Impervious area HSG-B (0.35") = 21,804 sf = Req'd. recharge = 635.96 cf
Basin 3 Volume = 36,806 cf

Capture Area Adjustment
Site Total Impervious Area = 185,202 sf
Impervious Area to Basins = 71,203 sf
Adjustment Factor = $185,202 / 71,203 = 2.601$
 $2.601 \times 2,076 \text{ cf (total of initial req't.)} = \mathbf{5,397.07 \text{ cf total required recharge}}$
Basin 1 + 2 + 3 Vol. = **126,268 cf total provided recharge**

• **72 Hour max. drawdown (Static)**
Infiltration Basin 1 - $4,065 \text{ cf} / 3,708 \text{ sf (bottom area)} = 1.09' \text{ depth (13.2")} / 1.1''/\text{hr} = \mathbf{11.9 \text{ hours}}$
Infiltration Basin 2 - $5,100 \text{ cf} / 4,504 \text{ sf (bottom area)} = 1.13' \text{ depth (13.6")} / 1.1''/\text{hr} = \mathbf{12.4 \text{ hours}}$
Infiltration Basin 3 - $4,490 \text{ cf} / 824 \text{ sf (bottom area)} = 5.44' \text{ depth (65.3")} / 1.1''/\text{hr} = \mathbf{59.3 \text{ hours}}$

*1982 Rawls Rates – 1.1"/hr (on-site soil test pits)

Standard #4 Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). This Standard is met when:

- a. Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan, and thereafter are implemented and maintained;
- b. Structural stormwater best management practices are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook; and
- c. Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.

- **Pretreatment sizing (Forebay – 0.5”/Impervious Acre)**

Forebay 1: Impervious Paved Roadway = $26,755 \text{ sf} \times 0.5'' (0.0417') = 1,116 \text{ cf req'd}$

Forebay 1 Volume = 1,506 cf

Forebay 2: Impervious Paved Roadway = $22,644 \text{ sf} \times 0.5'' (0.0417') = 944.25 \text{ cf req'd}$

Forebay 2 Volume = 2,800 cf

Forebay 3: Impervious Paved Roadway = $21,804 \text{ sf} \times 0.5'' (0.0417') = 909.23 \text{ cf req'd}$

Forebay 3 Volume = 3,408 cf

- **Water Quality Volume – 1.0”**

- Infiltration Basin 1

- Impervious Paved Roadway = $26,755 \text{ sf} \times 1.0'' (0.083') = 2,202 \text{ cf req'd}$

- Basin 1 Volume (below lowest outlet) = 4,065 cf

- Infiltration Basin 2

- Impervious Paved Roadway = $22,644 \text{ sf} \times 1.0'' (0.083') = 1,879 \text{ cf req'd}$

- Basin 2 Volume (below lowest outlet) = 5,100 cf

- Infiltration Basin 3

- Impervious Paved Roadway = $21,804 \text{ sf} \times 1.0'' (0.083') = 1,809 \text{ cf req'd}$

- Basin 3 Volume (Below lowest outlet) = 4,490 cf

- **TSS Removal – 80% Min. – 90% by design**

SEE ATTACHED SHEETS

Standard #5 For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable.

- Locus site does not meet the criteria to be designated as a “LUHPPL

Standard #6 Stormwater discharges within the Zone II or Interim Wellhead Protection Area of a public water supply, and stormwater discharges near or to any other critical area, require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas, as provided in the Massachusetts Stormwater Handbook.

- Stormwater does not discharge within the Zone II or Interim Wellhead Protection Area of a public water supply or to any other critical area.

Standard #7 A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

- The site is not a redevelopment project.

Standard #8 A plan to control construction-related impacts including erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.

- The plan set includes notes and details to avoid sediment migration and construction period erosion. Detailed methods and schedules to be incorporated into the Storm Water Pollution Prevention Plan as required by the EPA/NPDES Construction Activities Permit prior to construction.

Standard #9 A long-term operation and maintenance plan shall be developed and implemented to ensure that stormwater management systems function as designed.

- **Long-term operation and maintenance plan**

The proposed stormwater management system and the Best Management Practices (BMP's) are to be constructed in accordance with the approved site design plans. During the construction process the general site contractor and property owner shall be designated as the owners of the BMP's and will be responsible for their operation and maintenance. Once the BMP's are constructed they are to be protected from sedimentation until the site is stabilized and vegetated. Inspections should be performed routinely and after every major storm event. Any accumulated sediments and debris are to be removed and any eroded areas are to be re-graded and re-vegetated.

Post-Development Phase Ownership:

After the completion of the site construction, the entire drainage system will be the responsibility of the property owner Wall Street Corporation, P.O. Box 272, Westwood, MA 02090. Should a homeowners association become responsible for the private roadway then the association would become the responsible party.

Emergency Fuel Spill Response:

In the event of a fuel spill the responsible party shall call 9-1-1. They shall follow local and state removal procedures for the contaminant. The responsible contractor shall also call the Bellingham Board of Health at (508) 966-5820, and the Mass DEP at (508) 792-7650. Any contaminated soil must be completely removed from the property and be delivered to a certified land fill.

Operation & Maintenance:

The following are the minimum maintenance criteria for the proposed BMP's. Responsible parties should however review the Mass DEP Stormwater Handbook for further explanation.

Street Sweeping – 7% Credit

Street cleaning shall be performed four times per year. The cleaning shall be done by mechanical methods specifically by vacuum sweepers not brush sweepers. The activity shall take place during dry weather. Disposal of sweepings to be in accordance with Reuse and Disposal of Street Sweepings MA DEP Bureau of Waste Prevention, Final Policy BWP-94-092.

Deep Sump Hooded Catch Basins and Manholes & Oil Grit Chamber

The catch basin shall be inspected and cleaned twice per year (early spring/late fall) and after each major storm event. Also, any catch basin or manhole shall be cleaned out if 12 inches of sediment has accumulated. Inspections shall include structural integrity of hood, depth of sediment in sump and amount of trash and/or debris around grate. Any leaf litter and/or debris shall be removed from catch basin grates after each major storm event.

Sediment Forebay and Infiltration Basin

In the first few months of use inspect the basin after every major storm to ensure it is stabilized and functioning properly. Thereafter mow grass and inspect at least twice per year. Remove grass clippings and any accumulated organic matter and debris. Remove sediment within forebay when within six inches of weir crest. Perform maintenance only when dry – do not compact the basin bottom.

Standard #10

Illicit Discharge Compliance Statement

Owner: Wall Street Development Corporation
Address: P.O Box 272, Westwood, MA 02090
Tel. (617) 922-8730

Responsibility


Owners are responsible for ultimate compliance with all provisions of the Massachusetts Stormwater Management Policy, the USEPA NPDES Construction General Permit and responsible for identifying and eliminating illicit discharges (as defined by the USEPA).

Engineer's Compliance Statement:


To the best of my knowledge, the submitted plans, computations and specifications meet the requirements of Standard 10 of the Massachusetts Stormwater Handbook regarding illicit discharges to the stormwater management system and that no detectable illicit discharges exist on the site. All documents and attachments were prepared under my direction and qualified personnel properly gathered and evaluated the information submitted, to the best of my knowledge.

Included with this statement are site plans, drawn to scale, that identify the location of systems for conveying stormwater on the site and show that these systems do not allow the entry of any illicit discharges into the stormwater management system. The plans also show any systems for conveying wastewater and/or groundwater on the site and show that there are no connections between the stormwater and wastewater systems.

For a redevelopment project (if applicable), all actions taken to identify and remove illicit discharges, including without limitation, visual screening, dye or smoke testing, and the removal of any sources of illicit discharges to the stormwater management system are documented and included with this statement.



Professional Engineer



Date

00454 - 25 YEAR FROZEN

Prepared by {enter your company name here}

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00454 - 25 YEAR FROZEN
Type III 24-hr 25 YR Rainfall=6.38"

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Page 4

Summary for Pond 2P: Basin 1

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 2.02" for 25 YR event
 Inflow = 5.32 cfs @ 12.19 hrs, Volume= 20,894 cf
 Outflow = 0.95 cfs @ 12.95 hrs, Volume= 15,533 cf, Atten= 82%, Lag= 45.8 min
 Primary = 0.95 cfs @ 12.95 hrs, Volume= 15,533 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 282.95' @ 12.95 hrs Surf.Area= 5,173 sf Storage= 8,638 cf

Plug-Flow detention time= 204.9 min calculated for 15,533 cf (74% of inflow)
 Center-of-Mass det. time= 110.8 min (980.3 - 869.5)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 281.00' | 51,257 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 281.00 | 3,708 | 232.0 | 0 | 0 | 3,708 |
| 282.00 | 4,433 | 251.0 | 4,065 | 4,065 | 4,477 |
| 284.00 | 6,052 | 288.0 | 10,443 | 14,508 | 6,153 |
| 286.00 | 7,890 | 326.0 | 13,901 | 28,410 | 8,109 |
| 288.00 | 9,968 | 364.0 | 17,818 | 46,227 | 10,306 |
| 288.50 | 10,153 | 374.0 | 5,030 | 51,257 | 10,922 |

| Device | Routing | Invert | Outlet Devices |
|--------|----------|---------|--|
| #1 | Primary | 287.50' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Primary | 281.00' | 12.0" Round Culvert L= 44.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 281.00' / 280.00' S= 0.0227 ' S= 0.0227 ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #3 | Device 2 | 282.00' | 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #4 | Device 2 | 282.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 2 | 283.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 2 | 286.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=0.95 cfs @ 12.95 hrs HW=282.95' (Free Discharge)

- 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
- 2=Culvert (Passes 0.95 cfs of 4.02 cfs potential flow)
- 3=Orifice/Grate (Orifice Controls 0.37 cfs @ 4.27 fps)
- 4=Orifice/Grate (Orifice Controls 0.58 cfs @ 2.29 fps)
- 5=Orifice/Grate (Controls 0.00 cfs)
- 6=Orifice/Grate (Controls 0.00 cfs)

00454 - 25 YEAR FROZEN

Prepared by {enter your company name here}

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00454 - 25 YEAR FROZEN

Type III 24-hr 25 YR Rainfall=6.38"

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Summary for Pond 4P: Basin 2

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 2.57" for 25 YR event
 Inflow = 7.18 cfs @ 12.15 hrs, Volume= 24,886 cf
 Outflow = 1.38 cfs @ 12.72 hrs, Volume= 18,727 cf, Atten= 81%, Lag= 33.9 min
 Primary = 1.38 cfs @ 12.72 hrs, Volume= 18,727 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 305.96' @ 12.72 hrs Surf.Area= 6,388 sf Storage= 10,598 cf

Plug-Flow detention time= 183.4 min calculated for 18,727 cf (75% of inflow)
 Center-of-Mass det. time= 93.9 min (948.6 - 854.7)

| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 304.00' | 30,313 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 304.00 | 4,504 | 302.0 | 0 | 0 | 4,504 |
| 306.00 | 6,434 | 340.0 | 10,881 | 10,881 | 6,549 |
| 308.00 | 8,590 | 378.0 | 14,972 | 25,853 | 8,836 |
| 308.50 | 9,256 | 397.0 | 4,460 | 30,313 | 10,023 |

| Device | Routing | Invert | Outlet Devices |
|--------|----------|---------|--|
| #1 | Primary | 307.70' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Primary | 304.00' | 12.0" Round Culvert L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 304.00' / 303.00' S= 0.0286 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #3 | Device 2 | 305.00' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #4 | Device 2 | 305.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 2 | 306.00' | 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=1.38 cfs @ 12.72 hrs HW=305.96' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
 2=Culvert (Passes 1.38 cfs of 4.03 cfs potential flow)
 3=Orifice/Grate (Orifice Controls 0.79 cfs @ 4.04 fps)
 4=Orifice/Grate (Orifice Controls 0.58 cfs @ 2.30 fps)
 5=Orifice/Grate (Controls 0.00 cfs)

00454 - 25 YEAR FROZEN00454 - 25 YEAR FROZEN
Type III 24-hr 25 YR Rainfall=6.38"

Prepared by {enter your company name here}

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Summary for Pond 6P: Basin 3

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 2.92" for 25 YR event
 Inflow = 4.93 cfs @ 12.11 hrs, Volume= 13,541 cf
 Outflow = 1.39 cfs @ 12.49 hrs, Volume= 12,109 cf, Atten= 72%, Lag= 23.0 min
 Primary = 1.39 cfs @ 12.49 hrs, Volume= 12,109 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 291.72' @ 12.49 hrs Surf.Area= 2,559 sf Storage= 4,518 cf

Plug-Flow detention time= 105.2 min calculated for 12,084 cf (89% of inflow)
 Center-of-Mass det. time= 55.7 min (907.9 - 852.1)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 289.00' | 12,306 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 289.00 | 840 | 185.0 | 0 | 0 | 840 |
| 290.00 | 1,423 | 204.0 | 1,119 | 1,119 | 1,459 |
| 291.00 | 2,065 | 223.0 | 1,734 | 2,853 | 2,139 |
| 292.00 | 2,763 | 242.0 | 2,406 | 5,258 | 2,880 |
| 293.00 | 3,517 | 260.0 | 3,132 | 8,391 | 3,642 |
| 294.00 | 4,328 | 279.0 | 3,915 | 12,306 | 4,500 |

| Device | Routing | Invert | Outlet Devices |
|--------|----------|---------|--|
| #1 | Primary | 293.25' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |
| #2 | Primary | 289.00' | 12.0" Round Culvert L= 45.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 289.00' / 288.00' S= 0.0222 ' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| #3 | Device 2 | 290.00' | 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #4 | Device 2 | 290.50' | 6.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #5 | Device 2 | 291.50' | 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads |
| #6 | Device 2 | 292.00' | 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads |

Primary OutFlow Max=1.39 cfs @ 12.49 hrs HW=291.72' (Free Discharge)

- 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
- 2=Culvert (Passes 1.39 cfs of 4.97 cfs potential flow)
- 3=Orifice/Grate (Orifice Controls 0.30 cfs @ 6.08 fps)
- 4=Orifice/Grate (Orifice Controls 0.93 cfs @ 4.74 fps)
- 5=Orifice/Grate (Orifice Controls 0.16 cfs @ 1.60 fps)
- 6=Orifice/Grate (Controls 0.00 cfs)

00454 - 50 - YEAR CULVERT

Type III 24-hr 50 YR Rainfall=7.23"

Prepared by {enter your company name here}

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Summary for Reach 2R: New Box Culvert

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

[63] Warning: Exceeded Reach EV1 INLET depth by 0.10' @ 0.00 hrs

Inflow Area = 2,754,935 sf, 3.15% Impervious, Inflow Depth > 2.43" for 50 YR event
Inflow = 62.55 cfs @ 13.01 hrs, Volume= 556,827 cf
Outflow = 62.55 cfs @ 13.01 hrs, Volume= 556,795 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Max. Velocity= 14.22 fps, Min. Travel Time= 0.0 min

Avg. Velocity= 6.95 fps, Avg. Travel Time= 0.1 min

Peak Storage= 150 cf @ 13.01 hrs

Average Depth at Peak Storage= 0.44' , Surface Width= 10.00'

Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 659.84 cfs

10.00' x 2.00' deep channel, n= 0.012 Concrete pipe, finished

Length= 34.0' Slope= 0.0441 %

Inlet Invert= 294.00', Outlet Invert= 292.50'



00454 - 50 - YEAR CULVERT

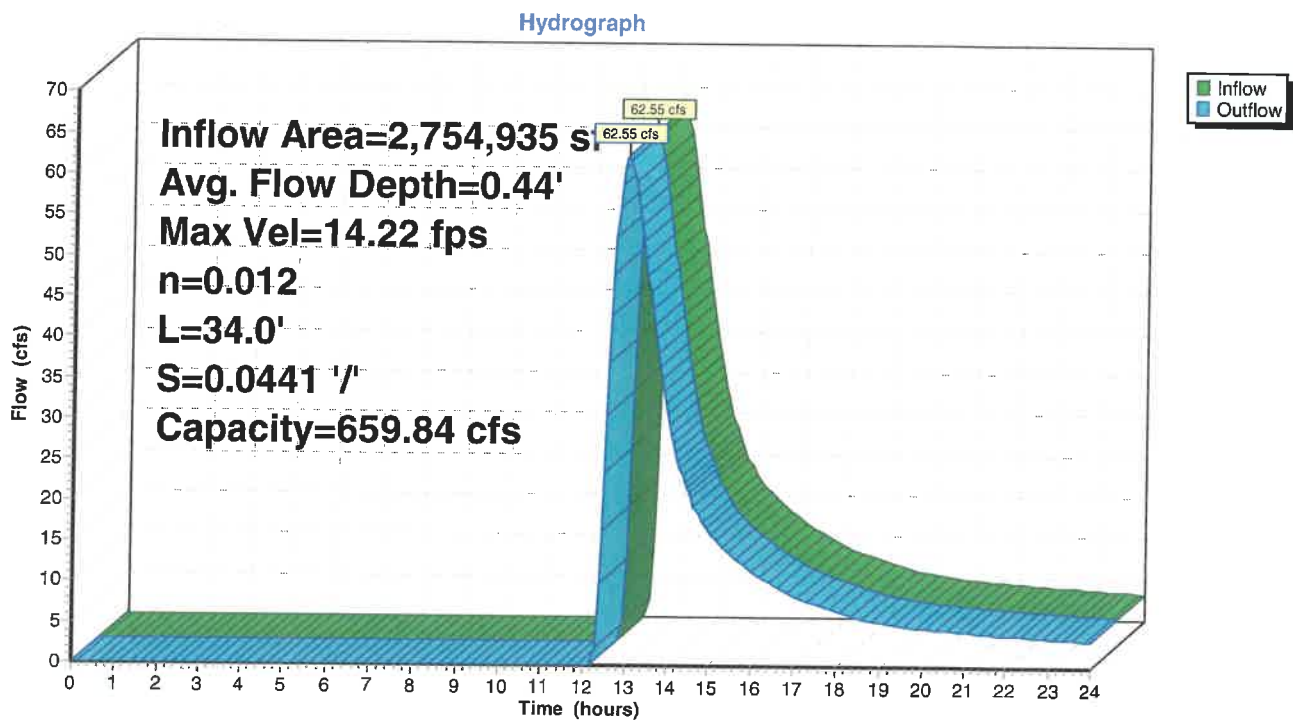
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Type III 24-hr 50 YR Rainfall=7.23"

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Reach 2R: New Box Culvert



PHOSPHORUS REMOVAL CALCULATIONS

Blackstone Street Improvements, Bellingham

Job No.: 00454
Client: Wall Street Development
Location: Blackstone Street, Bellingham MA

Calced By: M. Allen
Date: 10/31/2025
Revised:

Phosphorus Removal Analysis

1. Pre-Development Phosphorus Load (Lpre)

$$L_{pre} = (C_{pre} * P * P_j * R_v(pre) * A) * 0.227$$

- Lpre - is the average annual load of phosphorus in lbs/year
Cpre - is pollutant concentration of phosphorus in runoff from the pre-development land use (0.05 lbs/ac/year - undeveloped)
P - is the annual rainfall in inches (45)
Pj - is the fraction of rainfall events that produce runoff (0.9 typical)
Rv(pre) - is the coefficient of pre-development conditions calculated as
 $R_v = 0.05 + 0.009 * I$, where I is the percentage of impervious
A - is the area of the development site in acres
0.227 - is a unit conversion factor

2. Post-Development Phosphorus Load (Lpost)

$$L_{post} = (C_{post} * P * P_j * R_v(post) * A) * 0.227$$

- Lpost - is the average annual load of phosphorus in lbs/year
Cpost - is pollutant concentration of phosphorus in runoff from the post-development land use (0.30 lbs/ac/year - urban)
P - is the annual rainfall in inches (45)
Pj - is the fraction of rainfall events that produce runoff (0.9 typical)
Rv(pre) - is the coefficient of pre-development conditions calculated as
 $R_v = 0.05 + 0.009 * I$, where I is the percentage of impervious
A - is the area of the development site in acres
0.227 - is a unit conversion factor

3. Phosphorus Removal

$$RR = L_{post} - L_{pre} - (L_{post} * \text{reduction target})$$

RR - Required removal

LR - Load Reduction from BMP's lbs / year

$$LR = L_{post} * B_{MP} * DA$$

BMPre - removal efficiency of the BMP

DA - is the fraction of the total drainage area served by the BMP as a decimal.

4. Calculation

Total Site 106.8 acres
Existing %Impervious= 3.41%
Proposed %Impervious= 4.08%

$$L_{pre} = (0.05 * 45 * 0.9 * 0.080 * 106.8) * 0.227$$

0.037 lbs / year

$$L_{post} = (0.3 * 45 * 0.9 * 0.086 * 106.8) * 0.227$$

0.568 lbs / year

BMP Removal Rate Infiltration Basin

$$LR = 0.568 * 0.9 * 1.0$$

0.51 lbs / year removed

0.51 lbs per year / 0.568 lbs per year

= 89.9% Removal Rate

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Blackstone Street Bellingham MA - Basin 1

| B BMP ¹ | C TSS Removal Rate ¹ | D Starting TSS Load* | E Amount Removed (C*D) | F Remaining Load (D-E) |
|----------------------------------|------------------------------------|-------------------------|---------------------------|---------------------------|
| Street Sweeping - 7% | 0.07 | 1.00 | 0.07 | 0.93 |
| Deep Sump and Hooded Catch Basin | 0.25 | 0.93 | 0.23 | 0.70 |
| Oil Grit Separator | 0.25 | 0.70 | 0.17 | 0.52 |
| Infiltration Basin | 0.80 | 0.52 | 0.42 | 0.10 |
| | 0.00 | 0.10 | 0.00 | 0.10 |

TSS Removal Calculation Worksheet

Total TSS Removal =

90%

Separate Form Needs to be Completed for Each Outlet or BMP Train

| | |
|--------------|--------------------------------------|
| Project: | 00454 Blackstone Street Improvements |
| Prepared By: | Mark Allen |
| Date: | 1/22/2026 |

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed
 1. From MassDEP Stormwater Handbook Vol. 1

*Equals remaining load from previous BMP (E) which enters the BMP

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: Blackstone Street Bellingham MA - Basin 2

| B BMP ¹ | C TSS Removal Rate ¹ | D Starting TSS Load* | E Amount Removed (C*D) | F Remaining Load (D-E) |
|----------------------------------|------------------------------------|-------------------------|---------------------------|---------------------------|
| Street Sweeping - 7% | 0.07 | 1.00 | 0.07 | 0.93 |
| Deep Sump and Hooded Catch Basin | 0.25 | 0.93 | 0.23 | 0.70 |
| Oil Grit Separator | 0.25 | 0.70 | 0.17 | 0.52 |
| Infiltration Basin | 0.80 | 0.52 | 0.42 | 0.10 |
| | 0.00 | 0.10 | 0.00 | 0.10 |

Total TSS Removal =

90%

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:

Prepared By:

Date:

| |
|-------------------------|
| 00454 Blackstone Street |
| Improvements |
| Mark Allen |
| 1/22/2026 |

Non-automated TSS Calculation Sheet

must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

*Equals remaining load from previous BMP (E) which enters the BMP

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: Blackstone Street Bellingham MA - Basin 3

| B BMP ¹ | C TSS Removal Rate ¹ | D Starting TSS Load* | E Amount Removed (C*D) | F Remaining Load (D-E) |
|-------------------------------------|---------------------------------------|----------------------------|------------------------------|------------------------------|
| Street Sweeping - 7% | 0.07 | 1.00 | 0.07 | 0.93 |
| Deep Sump and Hooded Catch Basin | 0.25 | 0.93 | 0.23 | 0.70 |
| Oil Grit Separator | 0.25 | 0.70 | 0.17 | 0.52 |
| Infiltration Basin | 0.80 | 0.52 | 0.42 | 0.10 |
| | 0.00 | 0.10 | 0.00 | 0.10 |

Total TSS Removal =

90%

**Separate Form Needs to
be Completed for Each
Outlet or BMP Train**

Project:

Prepared By:

Date:

00454 Blackstone Street
Improvements

Mark Allen

1/22/2026

Non-automated TSS Calculation Sheet

must be used if Proprietary BMP Proposed

1. From MassDEP Stormwater Handbook Vol. 1

*Equals remaining load from previous BMP (E)

which enters the BMP

OIL GRIT SEPARATOR CALCULATIONS

Blackstone Street Improvements
Bellingham, Massachusetts

Minimum Sizing for Oil Grit Chambers:

1st chamber to have volume = 400 c.f. per acre of Impervious

Oil Grit Chamber 1

$$\begin{aligned} &400 \text{ c.f./Impervious Acre} \\ &= 9,743 \text{ s.f. (0.22 acres)} \\ &= 400 \times 0.22 \\ &= 90 \text{ c.f. or } \mathbf{669 \text{ gallons}} \text{ (@ } 7.48 \text{ gallons/ c.f.)} \end{aligned}$$

USE 1,000 Gallon Tank

Oil Grit Chamber 2

$$\begin{aligned} &400 \text{ c.f./Impervious Acre} \\ &= 22,646 \text{ s.f. (0.52 acres)} \\ &= 400 \times 0.52 \\ &= 208 \text{ c.f. or } \mathbf{1,555 \text{ gallons}} \text{ (@ } 7.48 \text{ gallons/ c.f.)} \end{aligned}$$

USE 2,000 Gallon Tank

Oil Grit Chamber 3

$$\begin{aligned} &400 \text{ c.f./Impervious Acre} \\ &= 21,804 \text{ s.f. (0.50 acres)} \\ &= 400 \times 0.50 \\ &= 200 \text{ c.f. or } \mathbf{1,497 \text{ gallons}} \text{ (@ } 7.48 \text{ gallons/ c.f.)} \end{aligned}$$

USE 2,000 Gallon Tank

00454 - 100 YEAR EMERGENCY SPILLWAY

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00454 - Spillway Adequacy-100 Year
Type III 24-hr 100 YR Rainfall=8.75"

Printed 1/22/2026

Page 4

Summary for Pond 2P: Basin 1

Inflow Area = 124,279 sf, 7.84% Impervious, Inflow Depth > 3.69" for 100 YR event
 Inflow = 9.85 cfs @ 12.18 hrs, Volume= 38,223 cf
 Outflow = 8.92 cfs @ 12.25 hrs, Volume= 33,852 cf, Atten= 9%, Lag= 4.4 min
 Primary = 8.92 cfs @ 12.25 hrs, Volume= 33,852 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 287.81' @ 12.25 hrs Surf.Area= 8,648 sf Storage= 6,807 cf

Plug-Flow detention time= 77.3 min calculated for 33,852 cf (89% of inflow)
 Center-of-Mass det. time= 24.2 min (874.8 - 850.6)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 281.00' | 13,252 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 281.00 | 10 | 10.0 | 0 | 0 | 10 |
| 282.00 | 10 | 10.0 | 10 | 10 | 20 |
| 284.00 | 10 | 10.0 | 20 | 30 | 40 |
| 286.00 | 10 | 10.0 | 20 | 50 | 60 |
| 287.50 | 8,000 | 345.0 | 4,146 | 4,196 | 9,527 |
| 288.50 | 10,153 | 374.0 | 9,055 | 13,252 | 11,225 |

| Device | Routing | Invert | Outlet Devices | | | | | | | | | |
|--------|---------|---------|--|--|--|--|--|--|--|--|--|--|
| #1 | Primary | 287.50' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir | | | | | | | | | |
| | | | Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 | | | | | | | | | |
| | | | Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 | | | | | | | | | |

Primary OutFlow Max=8.87 cfs @ 12.25 hrs HW=287.81' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 8.87 cfs @ 1.42 fps)

00454 - 100 YEAR EMERGENCY SPILLWAY

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00454 - Spillway Adequacy-100 Year

Type III 24-hr 100 YR Rainfall=8.75"

Printed 1/22/2026

Page 8

Summary for Pond 4P: Basin 2

Inflow Area = 116,316 sf, 19.47% Impervious, Inflow Depth > 4.42" for 100 YR event
 Inflow = 12.06 cfs @ 12.15 hrs, Volume= 42,837 cf
 Outflow = 10.37 cfs @ 12.22 hrs, Volume= 35,225 cf, Atten= 14%, Lag= 4.4 min
 Primary = 10.37 cfs @ 12.22 hrs, Volume= 35,225 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 308.05' @ 12.22 hrs Surf.Area= 8,808 sf Storage= 10,450 cf

Plug-Flow detention time= 106.3 min calculated for 35,152 cf (82% of inflow)
 Center-of-Mass det. time= 35.5 min (873.4 - 837.9)

| Volume | Invert | Avail.Storage | Storage Description | | |
|---------------------|----------------------|------------------|--|---------------------------|---------------------|
| #1 | 304.00' | 14,544 cf | Custom Stage Data (Irregular) Listed below (Recalc) | | |
| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
| 304.00 | 10 | 10.0 | 0 | 0 | 10 |
| 306.00 | 10 | 20.0 | 20 | 20 | 48 |
| 307.20 | 8,000 | 378.0 | 3,317 | 3,337 | 11,389 |
| 308.50 | 9,256 | 397.0 | 11,206 | 14,544 | 12,665 |

| Device | Routing | Invert | Outlet Devices | | | | | | | | | |
|--------|---------|---------|--|--|--|--|--|--|--|--|--|--|
| #1 | Primary | 307.70' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir | | | | | | | | | |
| | | | Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 | | | | | | | | | |
| | | | Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 | | | | | | | | | |

Primary OutFlow Max=10.07 cfs @ 12.22 hrs HW=308.04' (Free Discharge)
 ↑1=Broad-Crested Rectangular Weir (Weir Controls 10.07 cfs @ 1.48 fps)

00454 - 100 YEAR EMERGENCY SPILLWAY

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00454 - Spillway Adequacy-100 Year
Type III 24-hr 100 YR Rainfall=8.75"

Printed 1/22/2026

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Summary for Pond 6P: Basin 3

Inflow Area = 55,723 sf, 39.13% Impervious, Inflow Depth > 4.93" for 100 YR event
 Inflow = 7.67 cfs @ 12.10 hrs, Volume= 22,915 cf
 Outflow = 7.30 cfs @ 12.13 hrs, Volume= 21,310 cf, Atten= 5%, Lag= 1.8 min
 Primary = 7.30 cfs @ 12.13 hrs, Volume= 21,310 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 Peak Elev= 293.53' @ 12.13 hrs Surf.Area= 3,805 sf Storage= 2,586 cf

Plug-Flow detention time= 49.8 min calculated for 21,310 cf (93% of inflow)
 Center-of-Mass det. time= 14.1 min (847.0 - 832.9)

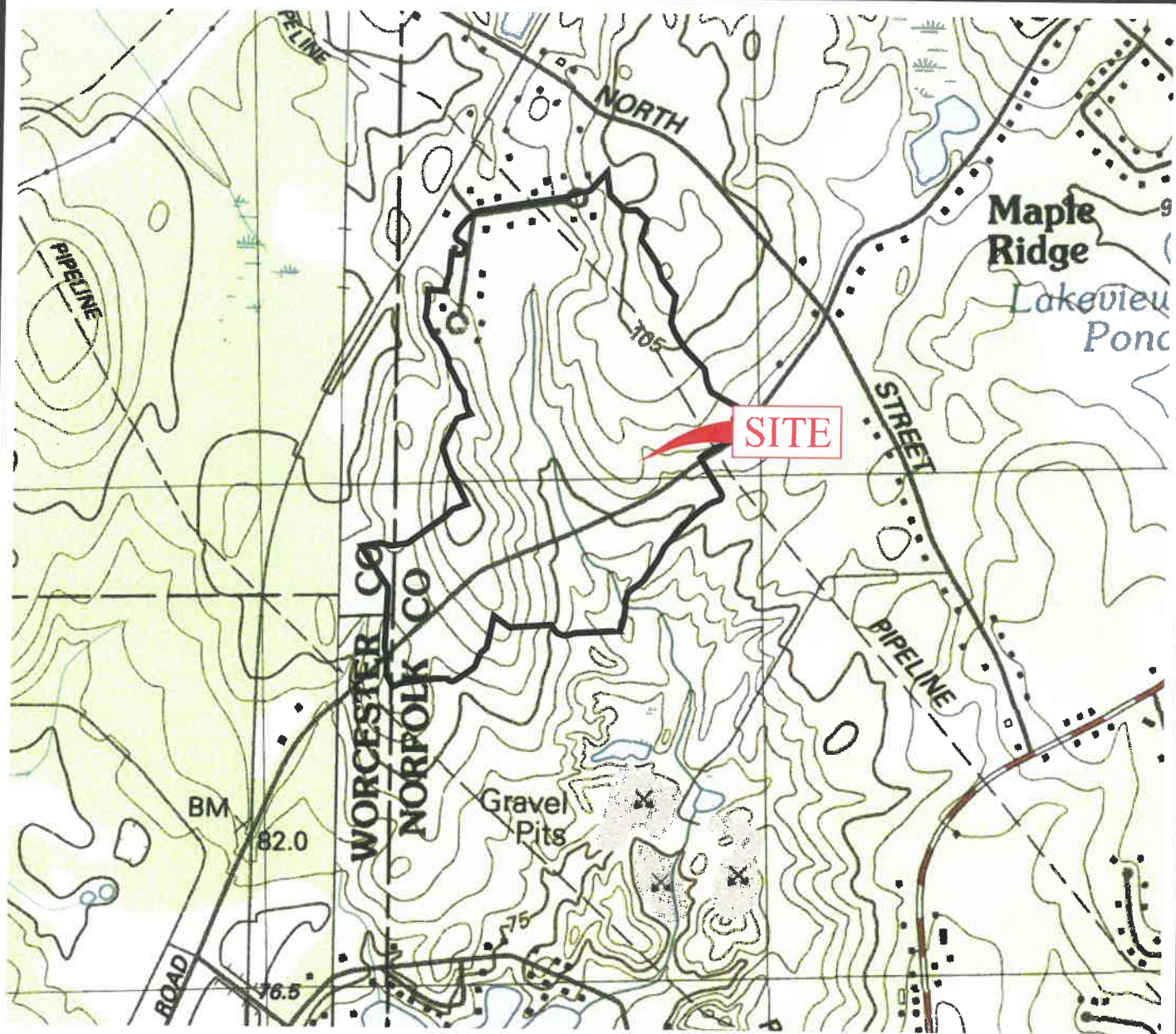
| Volume | Invert | Avail.Storage | Storage Description |
|--------|---------|---------------|--|
| #1 | 289.00' | 4,514 cf | Custom Stage Data (Irregular) Listed below (Recalc) |

| Elevation (feet) | Surf.Area (sq-ft) | Perim. (feet) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) | Wet.Area (sq-ft) |
|---------------------|----------------------|------------------|---------------------------|---------------------------|---------------------|
| 289.00 | 10 | 10.0 | 0 | 0 | 10 |
| 290.00 | 10 | 10.0 | 10 | 10 | 20 |
| 291.00 | 10 | 10.0 | 10 | 20 | 30 |
| 292.00 | 10 | 10.0 | 10 | 30 | 40 |
| 293.25 | 3,517 | 260.0 | 1,548 | 1,578 | 5,414 |
| 294.00 | 4,328 | 279.0 | 2,937 | 4,514 | 6,254 |

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 293.25' | 20.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 |

Primary OutFlow Max=7.12 cfs @ 12.13 hrs HW=293.52' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Weir Controls 7.12 cfs @ 1.31 fps)

Supplemental Information



PORTION OF U.S.G.S. GEODETIC MAP

SITE: **Blackstone Street**
Bellingham, MA 02019

PREPARED BY: **ALLEN ENGINEERING
& ASSOCIATES, INC.**



Civil Engineers • Surveyors
Land Development Consultants

140 Hartford Avenue East
Hopedale, Ma 01747
(508) 381-3212 • Phone
www.allen-ea.com

DATE: **1/28/2025**

JOB NO: **00454**

National Flood Hazard Layer FIRMette

71°30'5"W 42°48'N



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS

- Without Base Flood Elevation (BFE)
Zone A, V, A99
- With BFE or Depth Zone AE, AO, AH, VE, AR
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD

- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
- Future Conditions 1% Annual Chance Flood Hazard Zone X
- Area with Reduced Flood Risk due to Levee. See Notes, Zone X
- Area with Flood Risk due to Levee Zone D

OTHER AREAS

- NO SCREEN
- Area of Minimal Flood Hazard Zone X
- Effective LOMRS
- Area of Undetermined Flood Hazard Zone D

GENERAL STRUCTURES

- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

Cross Sections with 1% Annual Chance

- Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

OTHER FEATURES

MAP PANELS

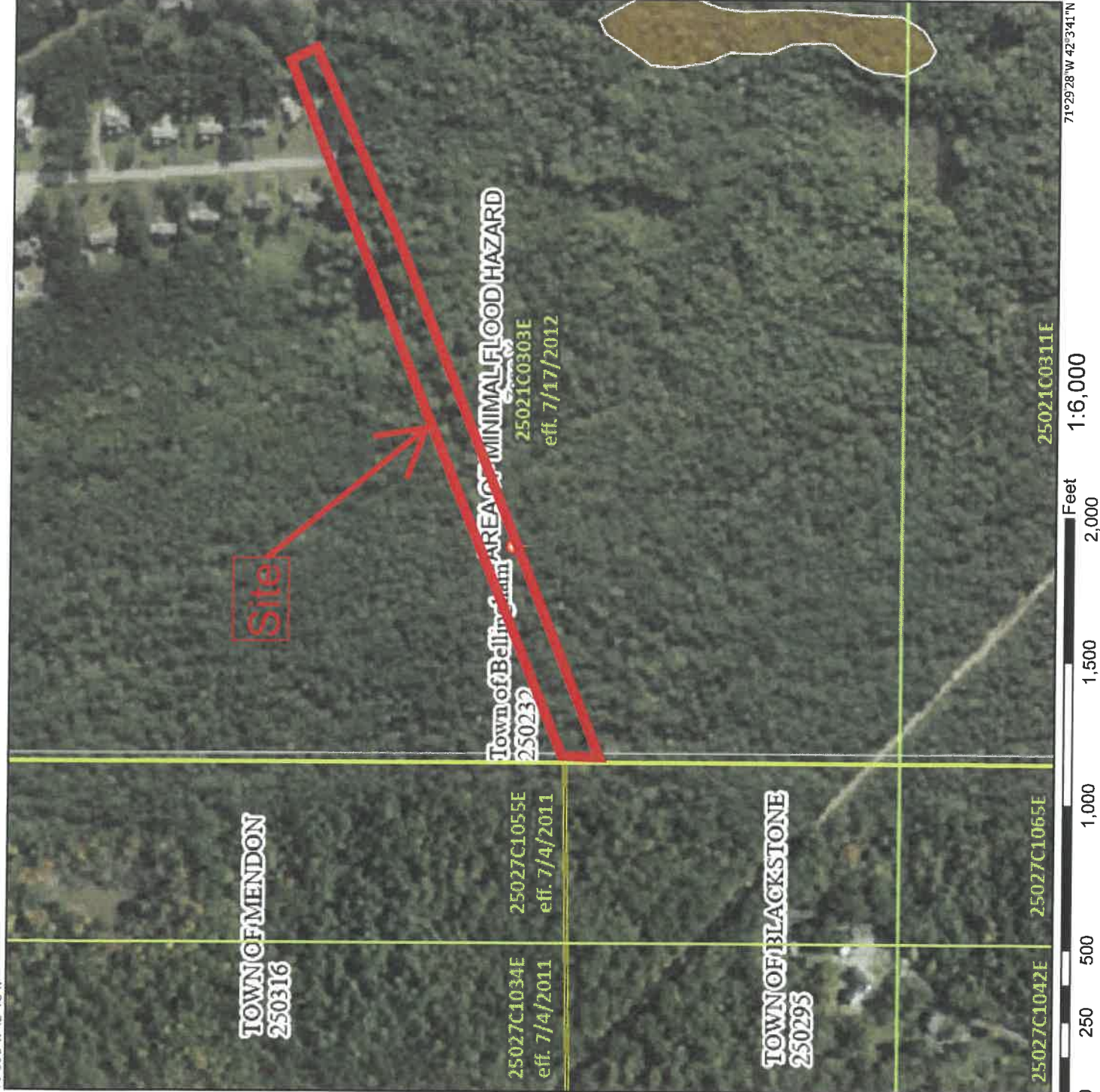
- Digital Data Available
- No Digital Data Available
- Unmapped

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/24/2025 at 9:28 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.





Commonwealth of Massachusetts
City/Town of Bellingham

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 01 Hole #

Date 11/12/24

Time Am

Longitude

Latitude

Weather

1. Land Use Woodland

(e.g., woodland, agricultural field, vacant lot, etc.)

Trees

Vegetation

Surface Stones (e.g., cobbles, stones, boulders, etc.)

Slope (%)

Description of Location:

2. Soil Parent Material:

Landform

Position on Landscape (SU, SH, BS, FS, TS, Plain)

Open Water Body >100 feet

Drainage Way >100 feet

Wetlands >100 feet

Property Line >10 feet

Drinking Water Well feet

Other feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☐ Yes ☐ No

If yes: Depth to Weeping in Hole

Depth to Standing Water in Hole

Soil Log

| Soil Log | | | | | | | | | | | | |
|-------------------|---------------------|---------------------|------------------------------------|------------------------|-------|---------|------------------------------|------------------|----------------|--------------------------|-------|--|
| Depth (in) | Soil Horizon /Layer | Soil Texture (USDA) | Soil Matrix: Color-Moist (Munsell) | Redoximorphic Features | | | Coarse Fragments % by Volume | | Soil Structure | Soil Consistence (Moist) | Other | |
| | | | | Depth | Color | Percent | Gravel | Cobbles & Stones | | | | |
| 0-7" | Ap | LS | 10YR 3/1 | Cnc : Dpl: | | | | | | | | |
| 7-18" | BW | LS | 2.5YR 6/8 | Cnc : Dpl: | | | | | | | | |
| 18-120" | C | LS | 2.5Y 6/3 | Cnc : Dpl: | | | | 25% | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| Additional Notes: | | | | | | | | | | | | |

Additional Notes:



Commonwealth of Massachusetts
City/Town of Bellingham

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 02 Hole # 11/12/24 Am Date Time

1. Land Use: Woodland

(e.g., woodland, agricultural field, vacant lot, etc.)

Trees Vegetation

Weather Latitude Longitude

Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location:

2. Soil Parent Material:

3. Distances from: Open Water Body >100 feet Landform Position on Landscape (SU, SH, BS, FS, TS, Plain) Drainage Way >100 feet Wetlands >100 feet

Property Line >10 feet Drinking Water Well feet Other feet

4. Unsuitable Materials Present: ☐ Yes ☐ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: Depth to Weeping in Hole 122" Depth Standing Water in Hole

Soil Log

| Depth (in) | Soil Horizon /Layer | Soil Texture (USDA) | Soil Matrix: Color-Moist (Munsell) | Redoximorphic Features | | Coarse Fragments % by Volume | | Soil Structure | Soil Consistence (Moist) | Other |
|-------------------|---------------------|---------------------|------------------------------------|------------------------|-------|------------------------------|--------|----------------|--------------------------|-------|
| | | | | Depth | Color | Percent | Gravel | | | |
| 0-6" | Ap | LS | 10YR 3/1 | Cnc : Dpl: | | | | | | |
| 6-19" | Bw | LS | 2.5YR 6/8 | Cnc : Dpl: | | | | | | |
| 19-124" | C | LS | 2.5Y 6/3 | Cnc : Dpl: | | | 30% | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| Additional Notes: | | | | | | | | | | |



Commonwealth of Massachusetts
City/Town of Bellingham

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: 03 Date: 11/12/24 Time: Am

1. Land Use Woodland

(e.g., woodland, agricultural field, vacant lot, etc.)

Description of Location:

Weather Latitude Longitude

Surface Stones (e.g., cobbles, stones, boulders, etc.)

Slope (%)

2. Soil Parent Material:

Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body >100 feet Drainage Way >100 feet Wetlands >100 feet

Property Line >10 feet Drinking Water Well feet Other feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☐ Yes ☒ No If yes: Depth to Weeping in Hole Depth to Standing Water in Hole

Soil Log

| Soil Log | | | | | | | | | | | | |
|-------------------|---------------------|---------------------|------------------------------------|------------------------|-------|---------|------------------------------|------------------|----------------|--------------------------|-------|--|
| Depth (in) | Soil Horizon /Layer | Soil Texture (USDA) | Soil Matrix: Color-Moist (Munsell) | Redoximorphic Features | | | Coarse Fragments % by Volume | | Soil Structure | Soil Consistence (Moist) | Other | |
| | | | | Depth | Color | Percent | Gravel | Cobbles & Stones | | | | |
| 0-120" | C | LS | 2.5Y6/3 | Cnc : Dpl: | | | 25% | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| | | | | Cnc : Dpl: | | | | | | | | |
| Additional Notes: | | | | | | | | | | | | |



Commonwealth of Massachusetts
City/Town of Bellingham

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 04 Hole #

Date 11/12/24

Time Am

1. Land Use: Woodland

(e.g., woodland, agricultural field, vacant lot, etc.)

Trees

Vegetation

Weather

Latitude

Longitude

Description of Location:

Surface Stones (e.g., cobbles, stones, boulders, etc.)

Slope (%)

2. Soil Parent Material:

Landform

Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body >100 feet

Drainage Way >100 feet

Wetlands >100 feet

Property Line >10 feet

Drinking Water Well _____ feet

Other _____ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No

If Yes: ☐ Disturbed Soil/Fill Material

☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☐ Yes ☒ No

If yes: _____ Depth to Weeping in Hole

_____ Depth Standing Water in Hole

Soil Log

| Depth (in) | Soil Horizon /Layer | Soil Texture (USDA) | Soil Matrix: Color-Moist (Munsell) | Redoximorphic Features | | Coarse Fragments % by Volume | | Soil Structure | Soil Consistence (Moist) | Other |
|-------------------|---------------------|---------------------|------------------------------------|------------------------|-------|------------------------------|--------|------------------|--------------------------|-------|
| | | | | Depth | Color | Percent | Gravel | Cobbles & Stones | | |
| 0-27" | Fill | LS | | Cnc : Dpl: | | | | | | |
| 27-122" | C | LS | | Cnc : Dpl: | | | 25% | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| | | | | Cnc : Dpl: | | | | | | |
| Additional Notes: | | | | | | | | | | |



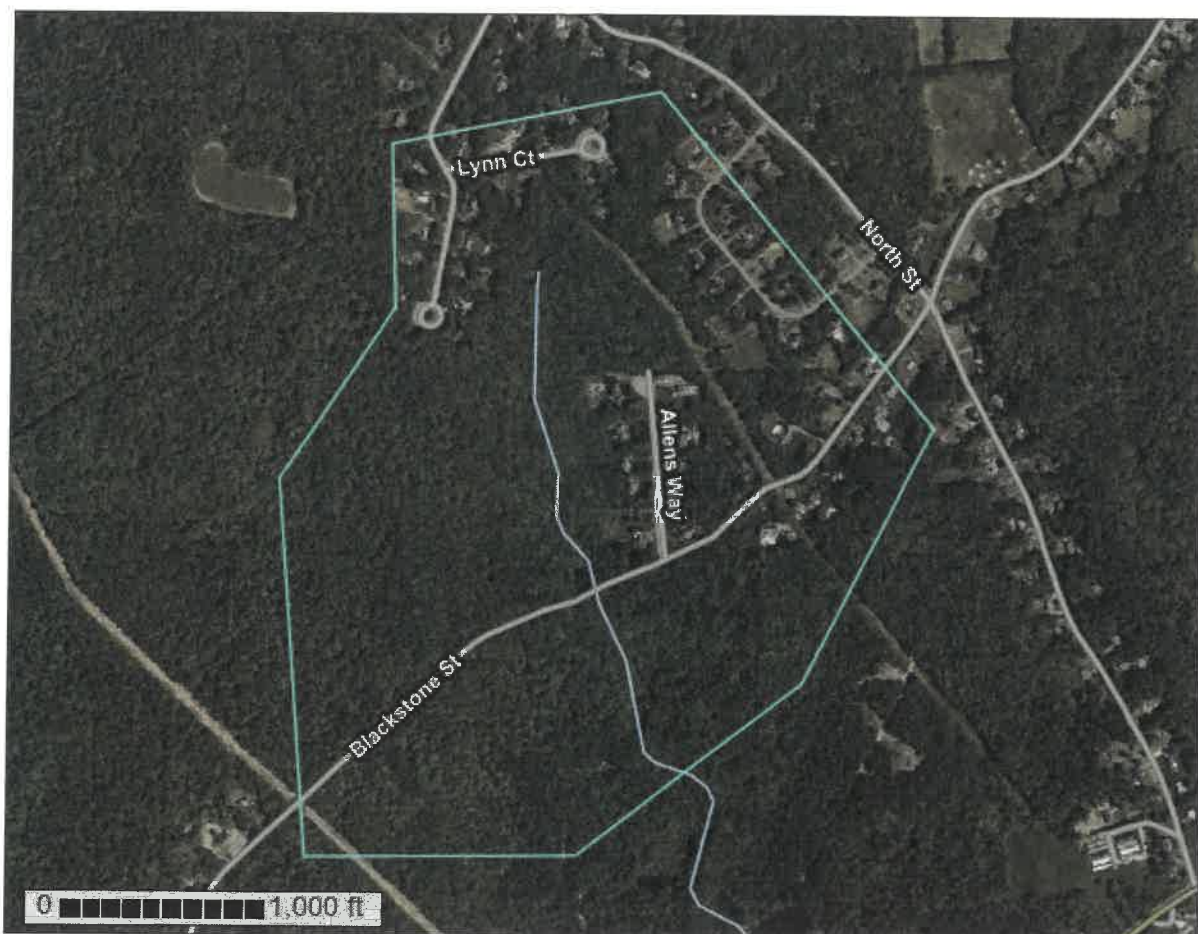
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for Norfolk and Suffolk Counties, Massachusetts; and Worcester County, Massachusetts, Southern Part



January 15, 2025

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

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identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map




































The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

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MAP LEGEND

| | | | | |
|---|------------------------|---|-----------------------|------------|
| Area of Interest (AOI) | |  | | Spoil Area |
|  | Area of Interest (AOI) |  | Stony Spot | |
| Soils | |  | Very Stony Spot | |
|  | Soil Map Unit Polygons |  | Wet Spot | |
|  | Soil Map Unit Lines |  | Other | |
|  | Soil Map Unit Points |  | Special Line Features | |
| Special Point Features | | Water Features | | |
|  | Blowout |  | Streams and Canals | |
|  | Borrow Pit | Transportation | | |
|  | Clay Spot |  | Rails | |
|  | Closed Depression |  | Interstate Highways | |
|  | Gravel Pit |  | US Routes | |
|  | Gravelly Spot |  | Major Roads | |
|  | Landfill |  | Local Roads | |
|  | Lava Flow | Background | | |
|  | Marsh or swamp |  | Aerial Photography | |
|  | Mine or Quarry | | | |
|  | Miscellaneous Water | | | |
|  | Perennial Water | | | |
|  | Rock Outcrop | | | |
|  | Saline Spot | | | |
|  | Sandy Spot | | | |
|  | Severely Eroded Spot | | | |
|  | Sinkhole | | | |
|  | Slide or Slip | | | |
| | Sodic Spot | | | |

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts
Survey Area Data: Version 20, Aug 27, 2024

Soil Survey Area: Worcester County, Massachusetts, Southern Part
Survey Area Data: Version 17, Aug 27, 2024

Your area of interest (AOI) includes more than one soil survey area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or at different levels of detail. This may result in map unit symbols, soil properties, and interpretations that do not completely agree across soil survey area boundaries.

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MAP LEGEND

MAP INFORMATION

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|---------------------------------------|---|--------------|----------------|
| 71B | Ridgebury fine sandy loam, 3 to 8 percent slopes, extremely stony | 1.3 | 0.7% |
| 73A | Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony | 18.3 | 9.6% |
| 245C | Hinckley loamy sand, 8 to 15 percent slopes | 0.3 | 0.2% |
| 420B | Canton fine sandy loam, 3 to 8 percent slopes | 8.6 | 4.5% |
| 422B | Canton fine sandy loam, 0 to 8 percent slopes, extremely stony | 138.0 | 72.5% |
| 653 | Udorthents, sandy | 10.6 | 5.6% |
| Subtotals for Soil Survey Area | | 177.1 | 93.0% |
| Totals for Area of Interest | | 190.4 | 100.0% |

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
|---------------------------------------|--|--------------|----------------|
| 422B | Canton fine sandy loam, 0 to 8 percent slopes, extremely stony | 13.3 | 7.0% |
| Subtotals for Soil Survey Area | | 13.3 | 7.0% |
| Totals for Area of Interest | | 190.4 | 100.0% |

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

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Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion

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of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Norfolk and Suffolk Counties, Massachusetts

71B—Ridgebury fine sandy loam, 3 to 8 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2w69c
Elevation: 0 to 1,290 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Ridgebury, extremely stony, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ridgebury, Extremely Stony

Setting

Landform: Drumlins, depressions, ground moraines, hills, drainageways
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material
A - 1 to 6 inches: fine sandy loam
Bw - 6 to 10 inches: sandy loam
Bg - 10 to 19 inches: gravelly sandy loam
Cd - 19 to 66 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 15 to 35 inches to densic material
Drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: D
Ecological site: F144AY009CT - Wet Till Depressions
Hydric soil rating: Yes

Minor Components

Woodbridge, extremely stony

Percent of map unit: 10 percent

Landform: Ground moraines, hills, drumlins

Landform position (two-dimensional): Summit, backslope, footslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex

Across-slope shape: Linear

Hydric soil rating: No

Whitman, extremely stony

Percent of map unit: 8 percent

Landform: Depressions

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

Paxton, extremely stony

Percent of map unit: 2 percent

Landform: Ground moraines, hills, drumlins

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Linear, convex

Hydric soil rating: No

73A—Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2w695

Elevation: 0 to 1,580 feet

Mean annual precipitation: 36 to 71 inches

Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Whitman, extremely stony, and similar soils: 81 percent

Minor components: 19 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Whitman, Extremely Stony

Setting

Landform: Drumlins, ground moraines, hills, drainageways, depressions

Landform position (two-dimensional): Toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave

Across-slope shape: Concave

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Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

Typical profile

Oi - 0 to 1 inches: peat
A - 1 to 10 inches: fine sandy loam
Bg - 10 to 17 inches: gravelly fine sandy loam
Cdg - 17 to 61 inches: fine sandy loam

Properties and qualities

Slope: 0 to 3 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 7 to 38 inches to densic material
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: D
Ecological site: F144AY041MA - Very Wet Till Depressions
Hydric soil rating: Yes

Minor Components

Ridgebury, extremely stony

Percent of map unit: 10 percent
Landform: Drumlins, depressions, ground moraines, hills, drainageways
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Scarboro

Percent of map unit: 5 percent
Landform: Drainageways, depressions, outwash terraces, outwash deltas
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Swansea

Percent of map unit: 3 percent
Landform: Marshes, bogs, swamps
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Woodbridge, extremely stony

Percent of map unit: 1 percent

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Landform: Ground moraines, hills, drumlins
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Concave
Across-slope shape: Linear
Hydric soil rating: No

245C—Hinckley loamy sand, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2svm9
Elevation: 0 to 1,480 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Hinckley and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hinckley

Setting

Landform: Outwash deltas, outwash terraces, moraines, eskers, kames, outwash plains, kame terraces
Landform position (two-dimensional): Shoulder, backslope, footslope, toeslope
Landform position (three-dimensional): Head slope, nose slope, side slope, crest, riser
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material
A - 1 to 8 inches: loamy sand
Bw1 - 8 to 11 inches: gravelly loamy sand
Bw2 - 11 to 16 inches: gravelly loamy sand
BC - 16 to 19 inches: very gravelly loamy sand
C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)

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Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: A
Ecological site: F144AY022MA - Dry Outwash
Hydric soil rating: No

Minor Components

Sudbury

Percent of map unit: 5 percent
Landform: Outwash deltas, moraines, outwash plains, kame terraces, outwash terraces
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Base slope, tread
Down-slope shape: Concave, linear
Across-slope shape: Concave, linear
Hydric soil rating: No

Windsor

Percent of map unit: 5 percent
Landform: Eskers, kames, outwash deltas, outwash terraces, outwash plains, kame terraces, moraines
Landform position (two-dimensional): Shoulder, backslope, footslope, toeslope
Landform position (three-dimensional): Head slope, nose slope, side slope, crest, riser
Down-slope shape: Concave, convex, linear
Across-slope shape: Convex, linear, concave
Hydric soil rating: No

Merrimac

Percent of map unit: 5 percent
Landform: Kames, outwash plains, outwash terraces, moraines, eskers
Landform position (two-dimensional): Shoulder, backslope, footslope, toeslope
Landform position (three-dimensional): Head slope, nose slope, side slope, crest, riser
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

420B—Canton fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2w81b
Elevation: 0 to 1,180 feet

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Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: All areas are prime farmland

Map Unit Composition

Canton and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton

Setting

Landform: Hills, moraines, ridges
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy over sandy melt-out till derived from gneiss, granite, and/or schist

Typical profile

Ap - 0 to 7 inches: fine sandy loam
Bw1 - 7 to 15 inches: fine sandy loam
Bw2 - 15 to 26 inches: gravelly fine sandy loam
2C - 26 to 65 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: 19 to 39 inches to strongly contrasting textural stratification
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2s
Hydrologic Soil Group: B
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Minor Components

Scituate

Percent of map unit: 10 percent
Landform: Hills, drumlins, ground moraines
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No

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Montauk

Percent of map unit: 5 percent
Landform: Moraines, ground moraines, hills, drumlins
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No

Charlton

Percent of map unit: 4 percent
Landform: Ridges, ground moraines, hills
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No

Swansea

Percent of map unit: 1 percent
Landform: Marshes, depressions, bogs, swamps, kettles
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

422B—Canton fine sandy loam, 0 to 8 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2w818
Elevation: 0 to 1,180 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Canton, extremely stony, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton, Extremely Stony

Setting

Landform: Moraines, hills, ridges
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy over sandy melt-out till derived from gneiss, granite, and/or schist

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Typical profile

O_i - 0 to 2 inches: slightly decomposed plant material
A - 2 to 5 inches: fine sandy loam
Bw₁ - 5 to 16 inches: fine sandy loam
Bw₂ - 16 to 22 inches: gravelly fine sandy loam
2C - 22 to 67 inches: gravelly loamy sand

Properties and qualities

Slope: 0 to 8 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 19 to 39 inches to strongly contrasting textural stratification
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (K_{sat}): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: B
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

Minor Components

Charlton, extremely stony

Percent of map unit: 6 percent
Landform: Ridges, ground moraines, hills
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No

Scituate, extremely stony

Percent of map unit: 6 percent
Landform: Hills, ground moraines, drumlins
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No

Montauk, extremely stony

Percent of map unit: 4 percent
Landform: Recessional moraines, ground moraines, hills, drumlins
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex

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Hydric soil rating: No

Swansea

Percent of map unit: 4 percent

Landform: Marshes, depressions, bogs, swamps, kettles

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

653—Udorthents, sandy

Map Unit Setting

National map unit symbol: vky8

Elevation: 0 to 3,000 feet

Mean annual precipitation: 45 to 54 inches

Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Udorthents and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents

Setting

Landform position (two-dimensional): Shoulder, summit

Landform position (three-dimensional): Riser, tread

Down-slope shape: Convex, linear

Across-slope shape: Convex, linear

Parent material: Excavated and filled sandy glaciofluvial deposits

Typical profile

H1 - 0 to 6 inches: variable

H2 - 6 to 60 inches: variable

Properties and qualities

Slope: 0 to 25 percent

Depth to restrictive feature: More than 80 inches

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to very high (0.06 to 20.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6s

Hydrologic Soil Group: A

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Hydric soil rating: Unranked

Minor Components

Udorthents

Percent of map unit: 8 percent

Hydric soil rating: Unranked

Urban land

Percent of map unit: 5 percent

Hydric soil rating: Unranked

Swansea

Percent of map unit: 2 percent

Landform: Bogs

Hydric soil rating: Yes

Worcester County, Massachusetts, Southern Part

422B—Canton fine sandy loam, 0 to 8 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2w818
Elevation: 0 to 1,180 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Canton, extremely stony, and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton, Extremely Stony

Setting

Landform: Moraines, hills, ridges
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy over sandy melt-out till derived from gneiss, granite, and/or schist

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material
A - 2 to 5 inches: fine sandy loam
Bw1 - 5 to 16 inches: fine sandy loam
Bw2 - 16 to 22 inches: gravelly fine sandy loam
2C - 22 to 67 inches: gravelly loamy sand

Properties and qualities

Slope: 0 to 8 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 19 to 39 inches to strongly contrasting textural stratification
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: B
Ecological site: F144AY034CT - Well Drained Till Uplands

Custom Soil Resource Report

Hydric soil rating: No

Minor Components

Charlton, extremely stony

Percent of map unit: 6 percent

Landform: Ridges, ground moraines, hills

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Scituate, extremely stony

Percent of map unit: 6 percent

Landform: Hills, ground moraines, drumlins

Landform position (two-dimensional): Summit, backslope, footslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Montauk, extremely stony

Percent of map unit: 4 percent

Landform: Recessionial moraines, ground moraines, hills, drumlins

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Swansea

Percent of map unit: 4 percent

Landform: Marshes, depressions, bogs, swamps, kettles

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

References

- American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.
- American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.
- Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.
- Federal Register. July 13, 1994. Changes in hydric soils of the United States.
- Federal Register. September 18, 2002. Hydric soils of the United States.
- Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.
- National Research Council. 1995. Wetlands: Characteristics and boundaries.
- Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262
- Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577
- Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580
- Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.
- United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.
- United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2_053374
- United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. <http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelpdb1043084>

Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerals](#)

PF tabular

| PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹ | | | | | | | | | | |
|--|-------------------------------------|------------------------|------------------------|------------------------|------------------------|-----------------------|-----------------------|----------------------|----------------------|----------------------|
| Duration | Average recurrence interval (years) | | | | | | | | | |
| | 1 | 2 | 5 | 10 | 25 | 50 | 100 | 200 | 500 | 1000 |
| 5-min | 0.329 (0.256-0.415) | 0.397 (0.309-0.502) | 0.508 (0.395-0.645) | 0.601 (0.464-0.766) | 0.728 (0.544-0.970) | 0.824 (0.603-1.12) | 0.924 (0.656-1.30) | 1.04 (0.697-1.49) | 1.19 (0.772-1.78) | 1.32 (0.834-2.01) |
| 10-min | 0.466 (0.363-0.588) | 0.562 (0.438-0.711) | 0.720 (0.559-0.913) | 0.851 (0.657-1.09) | 1.03 (0.770-1.37) | 1.17 (0.854-1.59) | 1.31 (0.929-1.85) | 1.47 (0.988-2.12) | 1.69 (1.10-2.52) | 1.87 (1.18-2.85) |
| 15-min | 0.548 (0.427-0.692) | 0.662 (0.515-0.837) | 0.848 (0.659-1.08) | 1.00 (0.773-1.28) | 1.21 (0.906-1.62) | 1.37 (1.00-1.87) | 1.54 (1.09-2.17) | 1.72 (1.16-2.49) | 1.99 (1.29-2.97) | 2.20 (1.39-3.35) |
| 30-min | 0.748 (0.583-0.944) | 0.904 (0.704-1.14) | 1.16 (0.900-1.47) | 1.37 (1.06-1.75) | 1.66 (1.24-2.21) | 1.88 (1.38-2.56) | 2.11 (1.50-2.97) | 2.36 (1.59-3.41) | 2.72 (1.76-4.07) | 3.01 (1.90-4.59) |
| 60-min | 0.947 (0.738-1.20) | 1.14 (0.892-1.45) | 1.47 (1.14-1.86) | 1.74 (1.34-2.22) | 2.11 (1.57-2.81) | 2.39 (1.75-3.25) | 2.68 (1.90-3.78) | 3.00 (2.02-4.33) | 3.46 (2.24-5.16) | 3.82 (2.42-5.83) |
| 2-hr | 1.21 (0.953-1.52) | 1.48 (1.16-1.86) | 1.92 (1.50-2.42) | 2.28 (1.77-2.89) | 2.78 (2.09-3.68) | 3.15 (2.32-4.26) | 3.54 (2.53-4.98) | 3.99 (2.70-5.72) | 4.63 (3.01-6.88) | 5.16 (3.27-7.82) |
| 3-hr | 1.40 (1.11-1.76) | 1.72 (1.35-2.15) | 2.23 (1.75-2.80) | 2.65 (2.07-3.35) | 3.24 (2.44-4.28) | 3.68 (2.72-4.96) | 4.14 (2.97-5.81) | 4.67 (3.16-6.68) | 5.45 (3.55-8.07) | 6.10 (3.88-9.21) |
| 6-hr | 1.81 (1.43-2.25) | 2.21 (1.75-2.74) | 2.86 (2.26-3.57) | 3.40 (2.67-4.27) | 4.15 (3.16-5.46) | 4.71 (3.51-6.34) | 5.30 (3.85-7.44) | 6.02 (4.09-8.55) | 7.10 (4.64-10.4) | 8.02 (5.12-12.0) |
| 12-hr | 2.30 (1.84-2.84) | 2.80 (2.23-3.46) | 3.61 (2.87-4.48) | 4.29 (3.38-5.34) | 5.22 (4.00-6.84) | 5.90 (4.44-7.92) | 6.65 (4.88-9.32) | 7.57 (5.16-10.7) | 8.98 (5.88-13.1) | 10.2 (6.52-15.2) |
| 24-hr | 2.76 (2.22-3.38) | 3.38 (2.71-4.14) | 4.39 (3.51-5.40) | 5.23 (4.15-6.47) | 6.38 (4.92-8.32) | 7.23 (5.48-9.67) | 8.16 (6.03-11.4) | 9.32 (6.38-13.1) | 11.1 (7.30-16.1) | 12.7 (8.13-18.7) |
| 2-day | 3.12 (2.52-3.79) | 3.87 (3.13-4.72) | 5.11 (4.11-6.24) | 6.13 (4.90-7.54) | 7.55 (5.86-9.78) | 8.58 (6.54-11.4) | 9.72 (7.23-13.5) | 11.1 (7.66-15.5) | 13.4 (8.82-19.3) | 15.3 (9.86-22.5) |
| 3-day | 3.39 (2.75-4.11) | 4.21 (3.41-5.10) | 5.54 (4.47-6.74) | 6.64 (5.33-8.13) | 8.16 (6.35-10.5) | 9.28 (7.08-12.3) | 10.5 (7.82-14.5) | 12.0 (8.30-16.7) | 14.4 (9.55-20.7) | 16.5 (10.7-24.2) |
| 4-day | 3.66 (2.98-4.42) | 4.50 (3.66-5.45) | 5.89 (4.77-7.14) | 7.03 (5.66-8.58) | 8.61 (6.71-11.1) | 9.77 (7.47-12.9) | 11.0 (8.22-15.2) | 12.6 (8.73-17.5) | 15.1 (10.0-21.6) | 17.3 (11.2-25.2) |
| 7-day | 4.40 (3.60-5.29) | 5.30 (4.34-6.38) | 6.78 (5.52-8.18) | 8.00 (6.47-9.71) | 9.68 (7.57-12.3) | 10.9 (8.36-14.2) | 12.3 (9.12-16.7) | 13.9 (9.64-19.1) | 16.4 (10.9-23.3) | 18.5 (12.0-26.8) |
| 10-day | 5.12 (4.20-6.13) | 6.05 (4.96-7.25) | 7.58 (6.19-9.11) | 8.84 (7.17-10.7) | 10.6 (8.29-13.4) | 11.9 (9.10-15.4) | 13.3 (9.84-17.8) | 14.9 (10.4-20.4) | 17.3 (11.5-24.5) | 19.4 (12.6-27.9) |
| 20-day | 7.23 (5.98-8.60) | 8.22 (6.79-9.78) | 9.84 (8.09-11.8) | 11.2 (9.13-13.4) | 13.0 (10.2-16.3) | 14.4 (11.1-18.4) | 15.9 (11.7-21.0) | 17.5 (12.2-23.7) | 19.7 (13.2-27.6) | 21.5 (14.0-30.8) |
| 30-day | 8.97 (7.45-10.6) | 10.0 (8.29-11.9) | 11.7 (9.64-13.9) | 13.1 (10.7-15.6) | 15.0 (11.8-18.6) | 16.4 (12.6-20.8) | 17.9 (13.2-23.4) | 19.5 (13.7-26.2) | 21.5 (14.4-30.0) | 23.1 (15.0-32.9) |
| 45-day | 11.1 (9.28-13.1) | 12.2 (10.2-14.4) | 13.9 (11.6-16.5) | 15.4 (12.7-18.3) | 17.4 (13.7-21.4) | 18.9 (14.6-23.7) | 20.4 (15.1-26.4) | 21.9 (15.4-29.3) | 23.7 (16.0-32.9) | 25.0 (16.3-35.4) |
| 60-day | 12.9 (10.8-15.2) | 14.0 (11.7-16.5) | 15.8 (13.2-18.7) | 17.3 (14.3-20.6) | 19.4 (15.3-23.7) | 21.0 (16.2-26.2) | 22.5 (16.6-28.8) | 23.9 (16.9-31.9) | 25.5 (17.2-35.2) | 26.5 (17.3-37.5) |

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

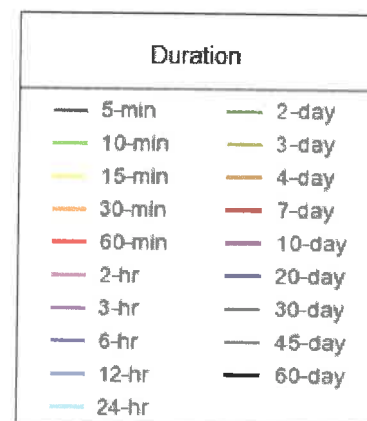
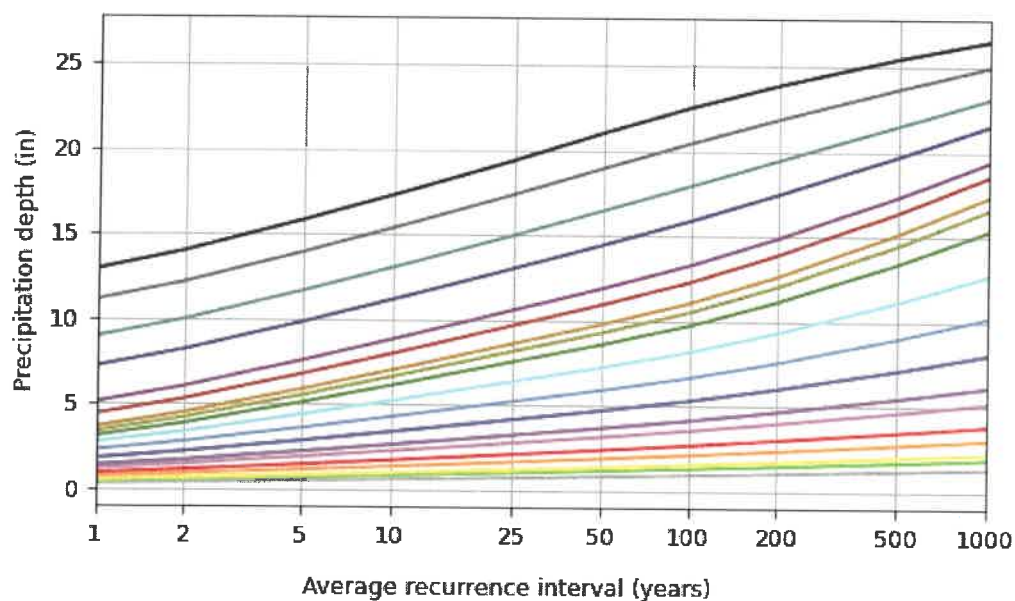
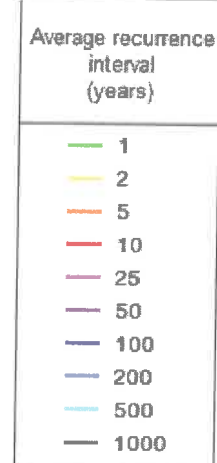
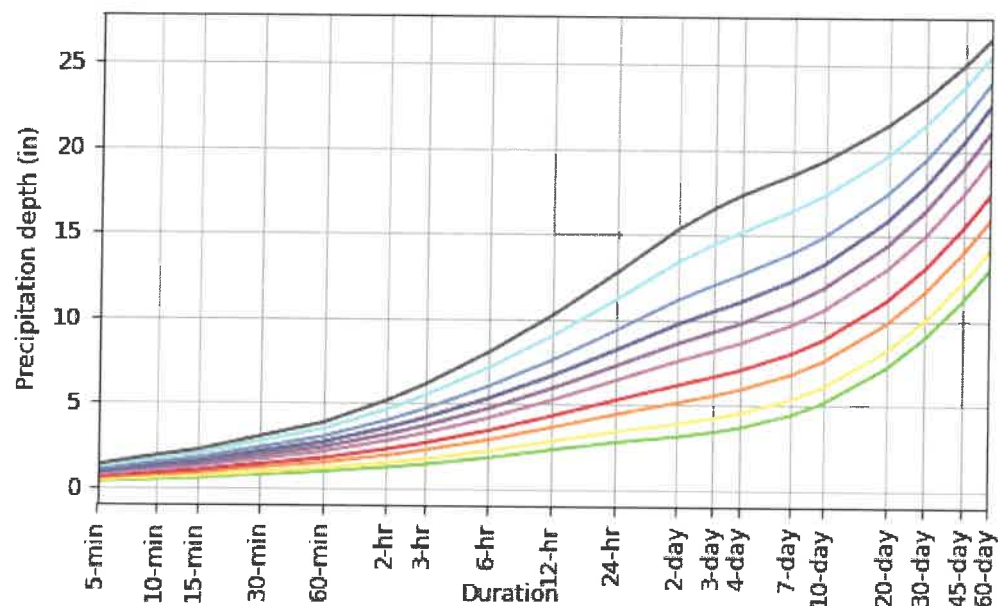
Please refer to NOAA Atlas 14 document for more information.

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PF graphical

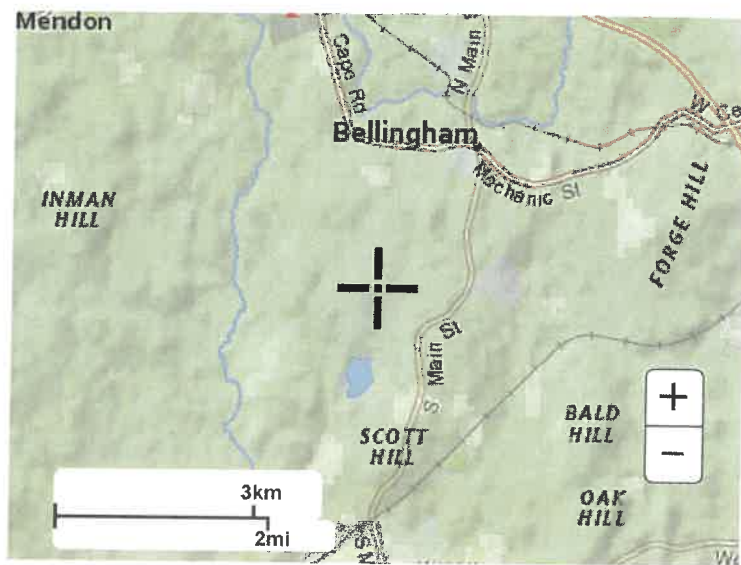
PDS-based depth-duration-frequency (DDF) curves

Latitude: 42.0668°, Longitude: -71.4929°



Maps & aerals

Small scale terrain



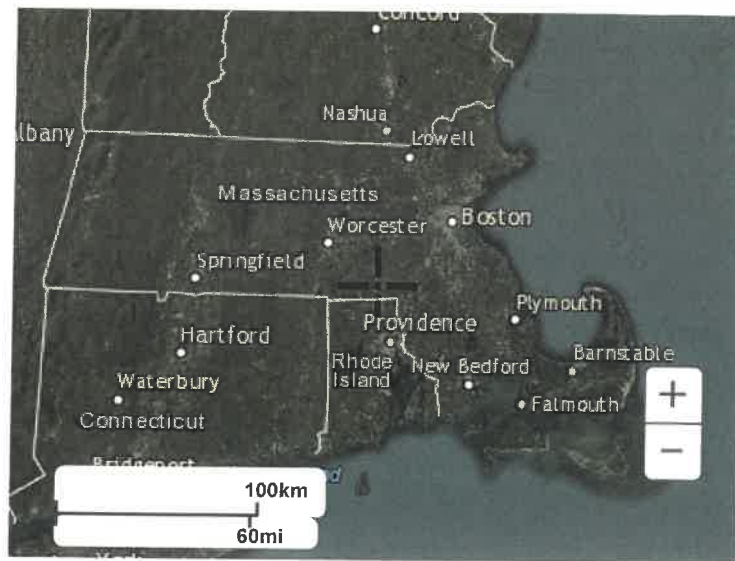
Large scale terrain



Large scale map



Large scale aerial



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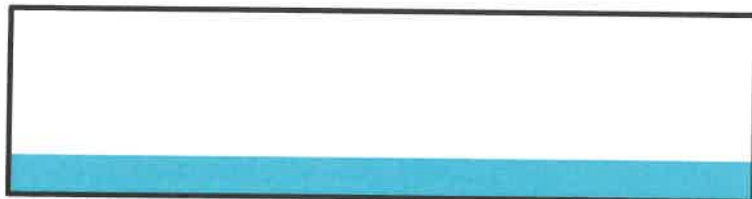
Summary for Reach 1R: 4'x 1' Box Culvert

Inflow Area = 492,614 sf, 0.00% Impervious, Inflow Depth = 0.04" for 25-yr event
Inflow = 4.70 cfs @ 0.08 hrs, Volume= 1,459 cf
Outflow = 4.59 cfs @ 0.09 hrs, Volume= 1,459 cf, Atten= 2%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Max. Velocity= 5.42 fps, Min. Travel Time= 0.2 min
Avg. Velocity= 2.51 fps, Avg. Travel Time= 0.4 min

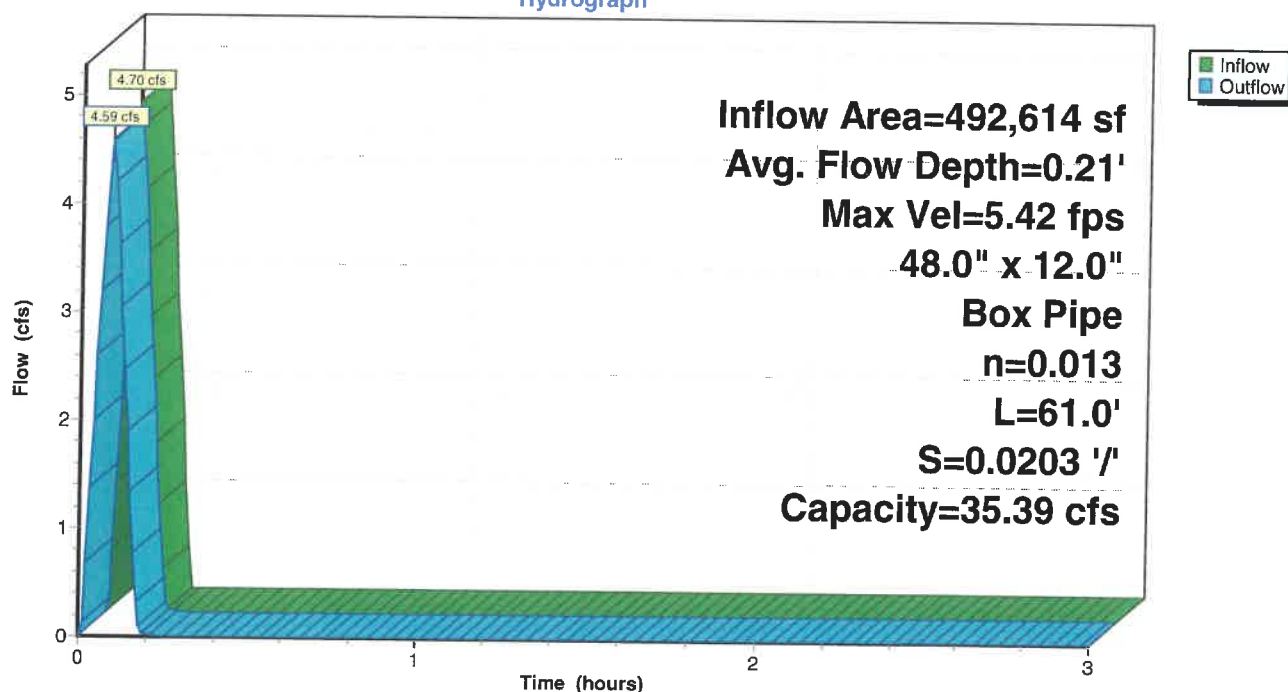
Peak Storage= 52 cf @ 0.09 hrs
Average Depth at Peak Storage= 0.21' , Surface Width= 4.00'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 35.39 cfs

48.0" W x 12.0" H Box Pipe
n= 0.013 Concrete, trowel finish
Length= 61.0' Slope= 0.0203 '/
Inlet Invert= 301.34', Outlet Invert= 300.10'



Reach 1R: 4'x 1' Box Culvert

Hydrograph



Summary for Reach 2R: New Box Cumvert

Inflow Area = 2,754,935 sf, 0.00% Impervious, Inflow Depth = 0.00" for 25-yr event
 Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 0.00 fps, Avg. Travel Time= 0.0 min

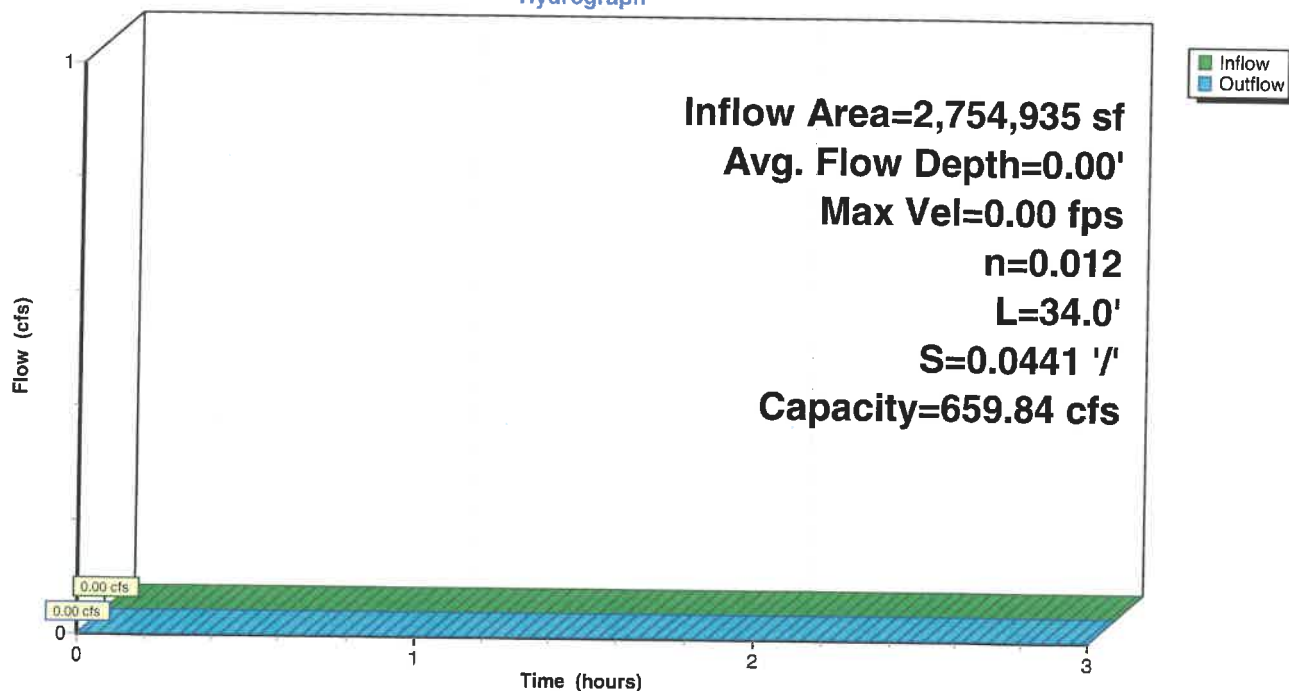
Peak Storage= 0 cf @ 0.00 hrs
 Average Depth at Peak Storage= 0.00'
 Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 659.84 cfs

10.00' x 2.00' deep channel, n= 0.012 Concrete pipe, finished
 Length= 34.0' Slope= 0.0441 '/'
 Inlet Invert= 294.00', Outlet Invert= 292.50'



Reach 2R: New Box Cumvert

Hydrograph



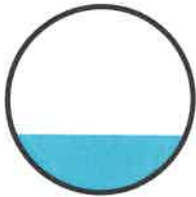
Summary for Reach 3R: CB1 to DMH 1

Inflow Area = 13,094 sf, 0.00% Impervious, Inflow Depth = 0.48" for 25-yr event
Inflow = 1.68 cfs @ 0.08 hrs, Volume = 522 cf
Outflow = 1.68 cfs @ 0.08 hrs, Volume = 522 cf, Atten = 0%, Lag = 0.0 min

Routing by Stor-Ind+Trans method, Time Span = 0.00-3.00 hrs, dt = 0.01 hrs
Max. Velocity = 8.09 fps, Min. Travel Time = 0.0 min
Avg. Velocity = 6.21 fps, Avg. Travel Time = 0.0 min

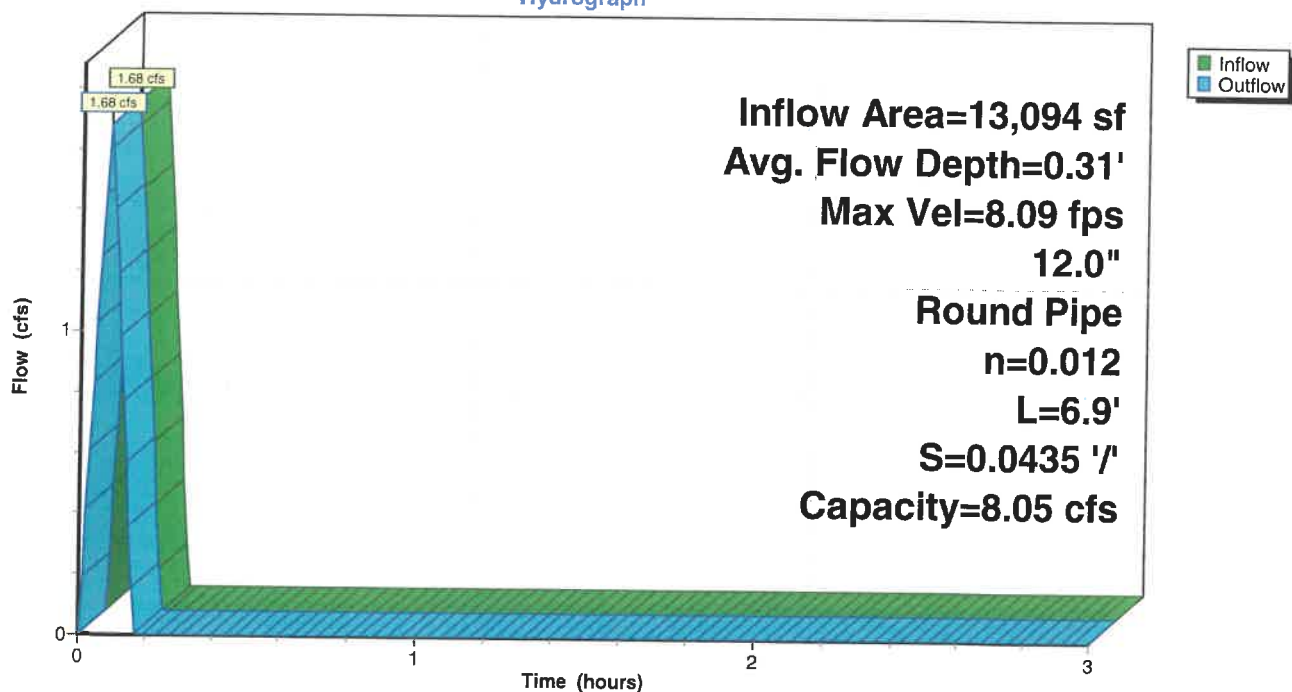
Peak Storage = 1 cf @ 0.08 hrs
Average Depth at Peak Storage = 0.31', Surface Width = 0.93'
Bank-Full Depth = 1.00' Flow Area = 0.8 sf, Capacity = 8.05 cfs

12.0" Round Pipe
n = 0.012 Corrugated PP, smooth interior
Length = 6.9' Slope = 0.0435 '/'
Inlet Invert = 295.20', Outlet Invert = 294.90'



Reach 3R: CB1 to DMH 1

Hydrograph



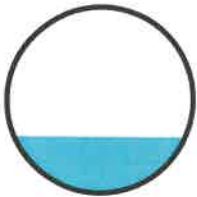
Summary for Reach 4R: CB2 to DMH 1

Inflow Area = 11,319 sf, 0.00% Impervious, Inflow Depth = 0.52" for 25-yr event
 Inflow = 1.58 cfs @ 0.08 hrs, Volume= 491 cf
 Outflow = 1.58 cfs @ 0.08 hrs, Volume= 491 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 7.95 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 6.10 fps, Avg. Travel Time= 0.0 min

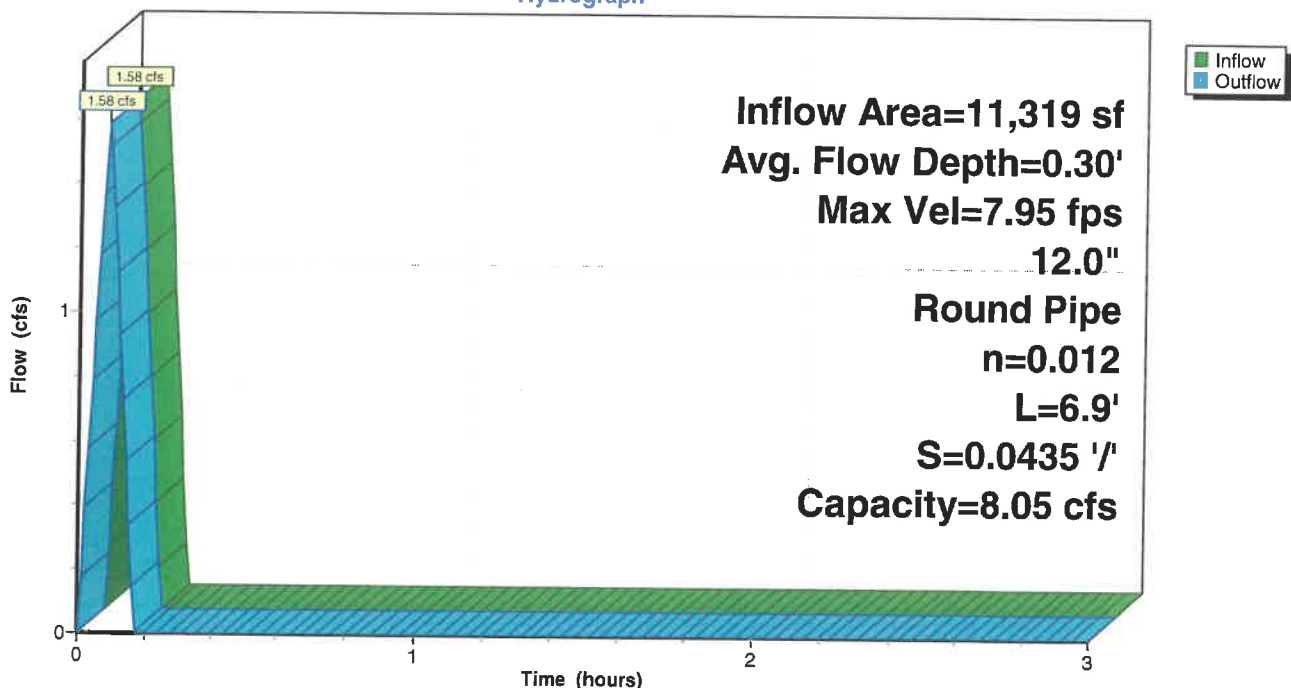
Peak Storage= 1 cf @ 0.08 hrs
 Average Depth at Peak Storage= 0.30' , Surface Width= 0.92'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.05 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 6.9' Slope= 0.0435 '/'
 Inlet Invert= 295.20', Outlet Invert= 294.90'



Reach 4R: CB2 to DMH 1

Hydrograph



Summary for Reach 5R: DMH1 to DMH 2

Inflow Area = 24,413 sf, 0.00% Impervious, Inflow Depth = 0.50" for 25-yr event
 Inflow = 3.26 cfs @ 0.08 hrs, Volume= 1,014 cf
 Outflow = 3.17 cfs @ 0.09 hrs, Volume= 1,014 cf, Atten= 3%, Lag= 0.5 min

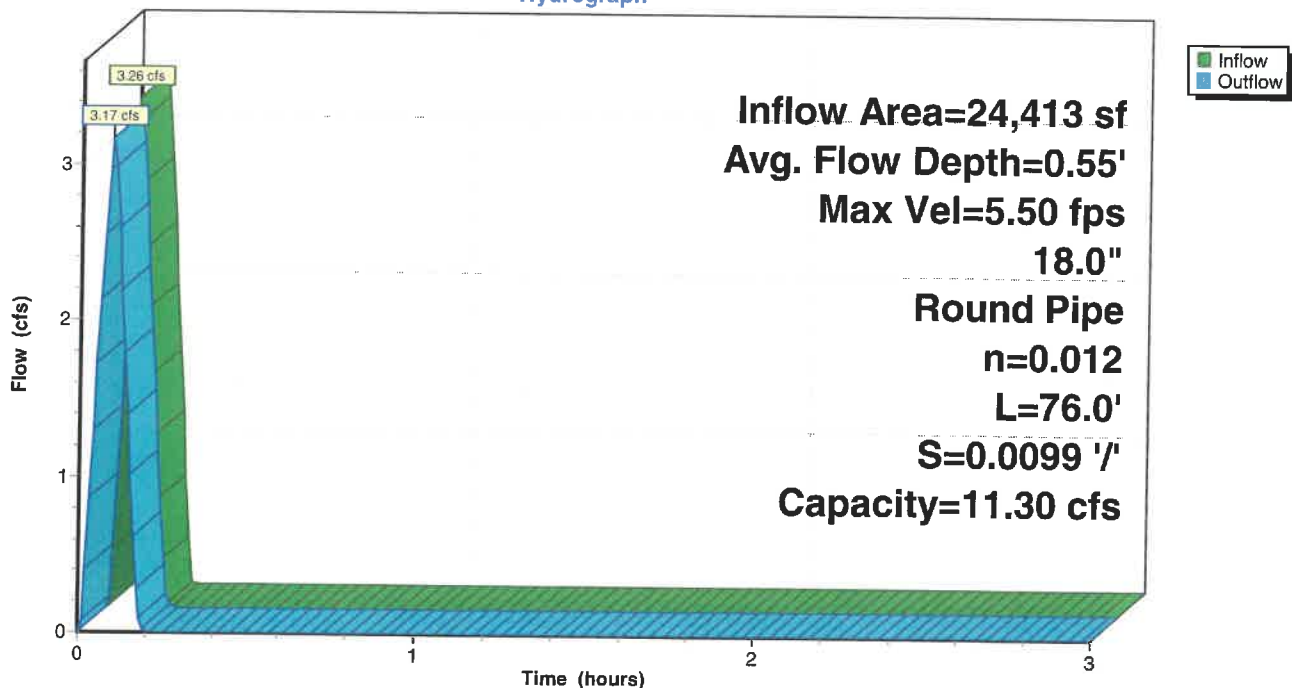
Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 5.50 fps, Min. Travel Time= 0.2 min
 Avg. Velocity= 2.44 fps, Avg. Travel Time= 0.5 min

Peak Storage= 44 cf @ 0.09 hrs
 Average Depth at Peak Storage= 0.55' , Surface Width= 1.44'
 Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 11.30 cfs

18.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 76.0' Slope= 0.0099 '/'
 Inlet Invert= 291.75', Outlet Invert= 291.00'

**Reach 5R: DMH1 to DMH 2**

Hydrograph



Summary for Reach 6R: CB3 to DMH 2

Inflow Area = 3,124 sf, 0.00% Impervious, Inflow Depth = 0.44" for 25-yr event
 Inflow = 0.37 cfs @ 0.08 hrs, Volume= 114 cf
 Outflow = 0.36 cfs @ 0.08 hrs, Volume= 114 cf, Atten= 0%, Lag= 0.0 min

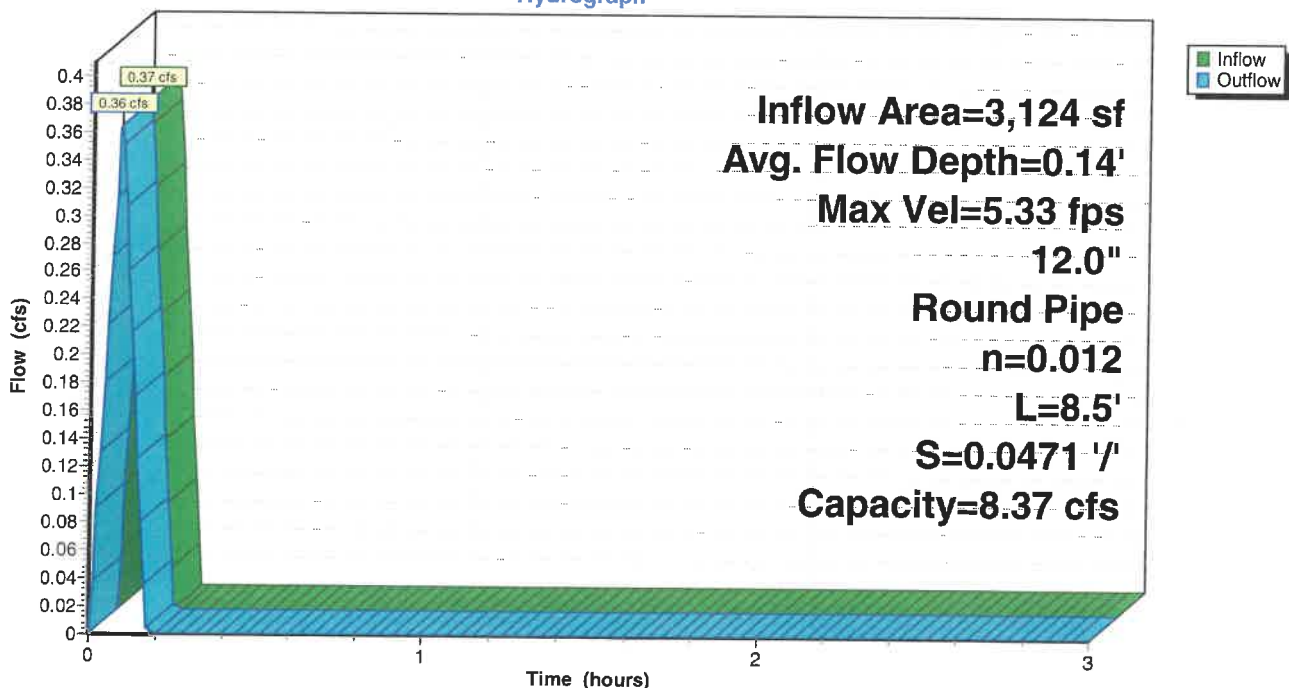
Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 5.33 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 4.08 fps, Avg. Travel Time= 0.0 min

Peak Storage= 1 cf @ 0.08 hrs
 Average Depth at Peak Storage= 0.14' , Surface Width= 0.70'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.37 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 8.5' Slope= 0.0471 '/'
 Inlet Invert= 299.00', Outlet Invert= 298.60'

**Reach 6R: CB3 to DMH 2**

Hydrograph



Summary for Reach 7R: CB4 to DMH 2

Inflow Area = 2,550 sf, 0.00% Impervious, Inflow Depth = 0.44" for 25-yr event
Inflow = 0.30 cfs @ 0.08 hrs, Volume= 94 cf
Outflow = 0.30 cfs @ 0.08 hrs, Volume= 94 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Max. Velocity= 4.22 fps, Min. Travel Time= 0.1 min
Avg. Velocity= 3.25 fps, Avg. Travel Time= 0.1 min

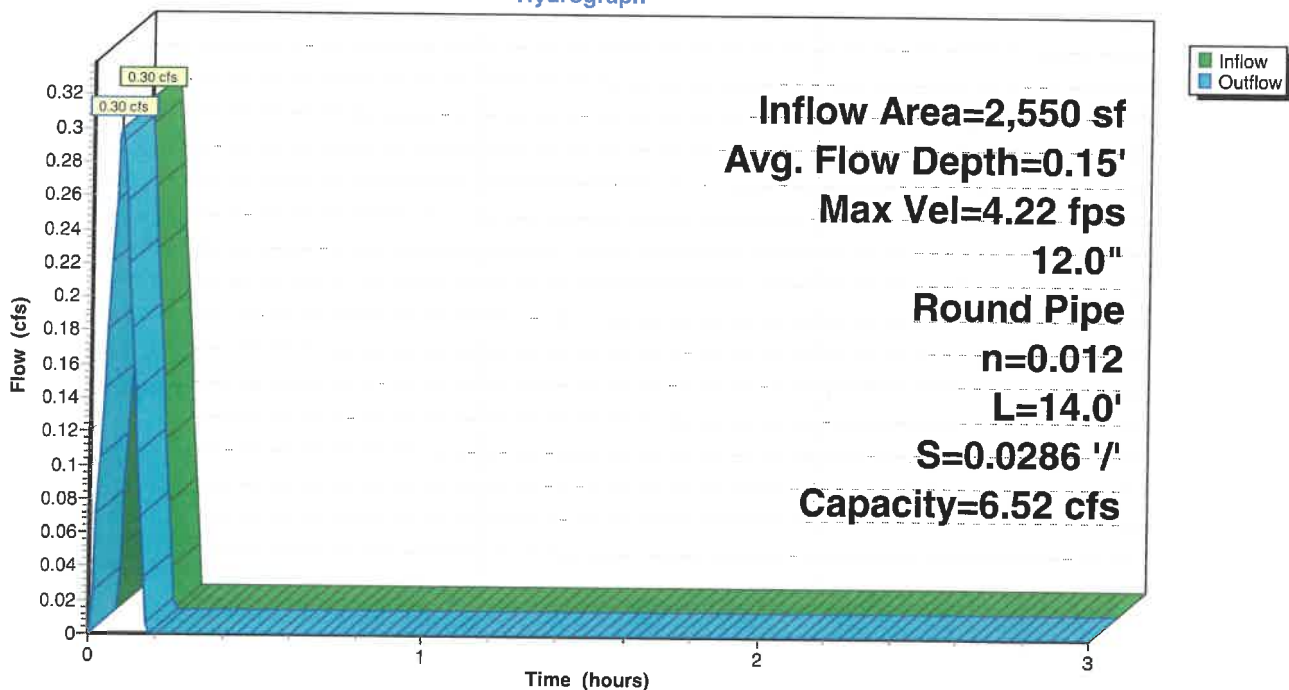
Peak Storage= 1 cf @ 0.08 hrs
Average Depth at Peak Storage= 0.15' , Surface Width= 0.71'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 6.52 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 14.0' Slope= 0.0286 '/'
Inlet Invert= 299.00', Outlet Invert= 298.60'



Reach 7R: CB4 to DMH 2

Hydrograph



Summary for Reach 8R: DMH 2 TO DMH 7

Inflow Area = 12,862 sf, 0.00% Impervious, Inflow Depth = 0.44" for 25-yr event
Inflow = 1.44 cfs @ 0.09 hrs, Volume = 474 cf
Outflow = 1.38 cfs @ 0.11 hrs, Volume = 474 cf, Atten = 4%, Lag = 1.2 min

Routing by Stor-Ind+Trans method, Time Span = 0.00-3.00 hrs, dt = 0.01 hrs
Max. Velocity = 5.58 fps, Min. Travel Time = 0.7 min
Avg. Velocity = 1.97 fps, Avg. Travel Time = 2.0 min

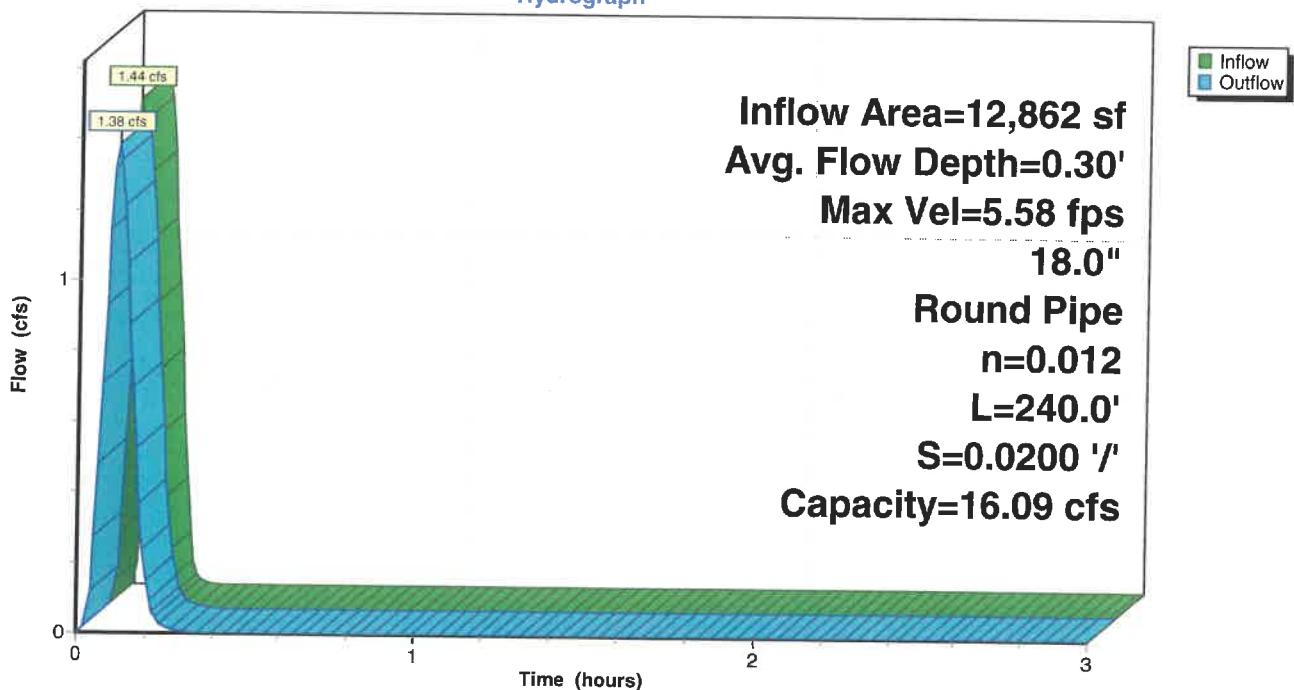
Peak Storage = 60 cf @ 0.10 hrs
Average Depth at Peak Storage = 0.30', Surface Width = 1.20'
Bank-Full Depth = 1.50' Flow Area = 1.8 sf, Capacity = 16.09 cfs

18.0" Round Pipe
n = 0.012 Corrugated PP, smooth interior
Length = 240.0' Slope = 0.0200 '/'
Inlet Invert = 294.80', Outlet Invert = 290.00'



Reach 8R: DMH 2 TO DMH 7

Hydrograph



Summary for Reach 9R: DMH 3 to DMH 2

Inflow Area = 7,188 sf, 0.00% Impervious, Inflow Depth = 0.44" for 25-yr event
 Inflow = 0.85 cfs @ 0.08 hrs, Volume= 266 cf
 Outflow = 0.82 cfs @ 0.10 hrs, Volume= 266 cf, Atten= 4%, Lag= 0.8 min

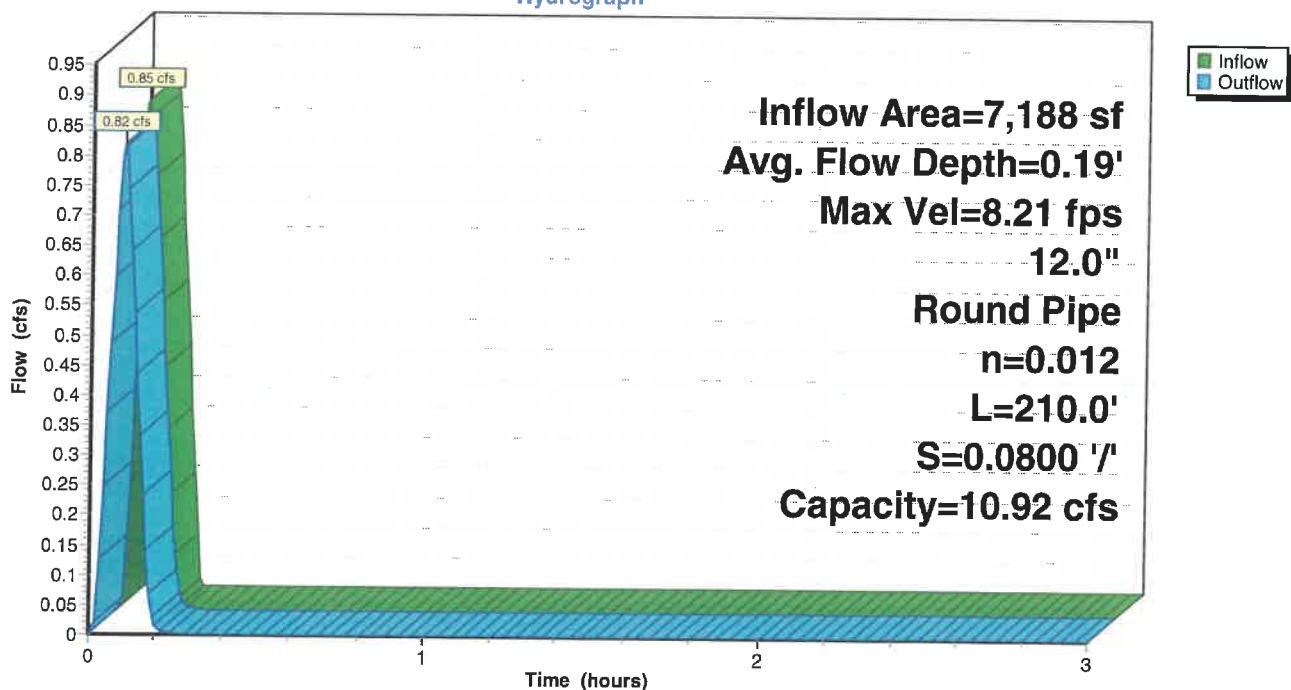
Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 8.21 fps, Min. Travel Time= 0.4 min
 Avg. Velocity= 3.80 fps, Avg. Travel Time= 0.9 min

Peak Storage= 21 cf @ 0.09 hrs
 Average Depth at Peak Storage= 0.19' , Surface Width= 0.78'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 210.0' Slope= 0.0800 '/'
 Inlet Invert= 315.40', Outlet Invert= 298.60'

**Reach 9R: DMH 3 to DMH 2**

Hydrograph



Summary for Reach 10R: CB5 to DMH 3

Inflow Area = 3,692 sf, 0.00% Impervious, Inflow Depth = 0.41" for 25-yr event
 Inflow = 0.40 cfs @ 0.08 hrs, Volume= 125 cf
 Outflow = 0.40 cfs @ 0.08 hrs, Volume= 125 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 5.36 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 4.10 fps, Avg. Travel Time= 0.0 min

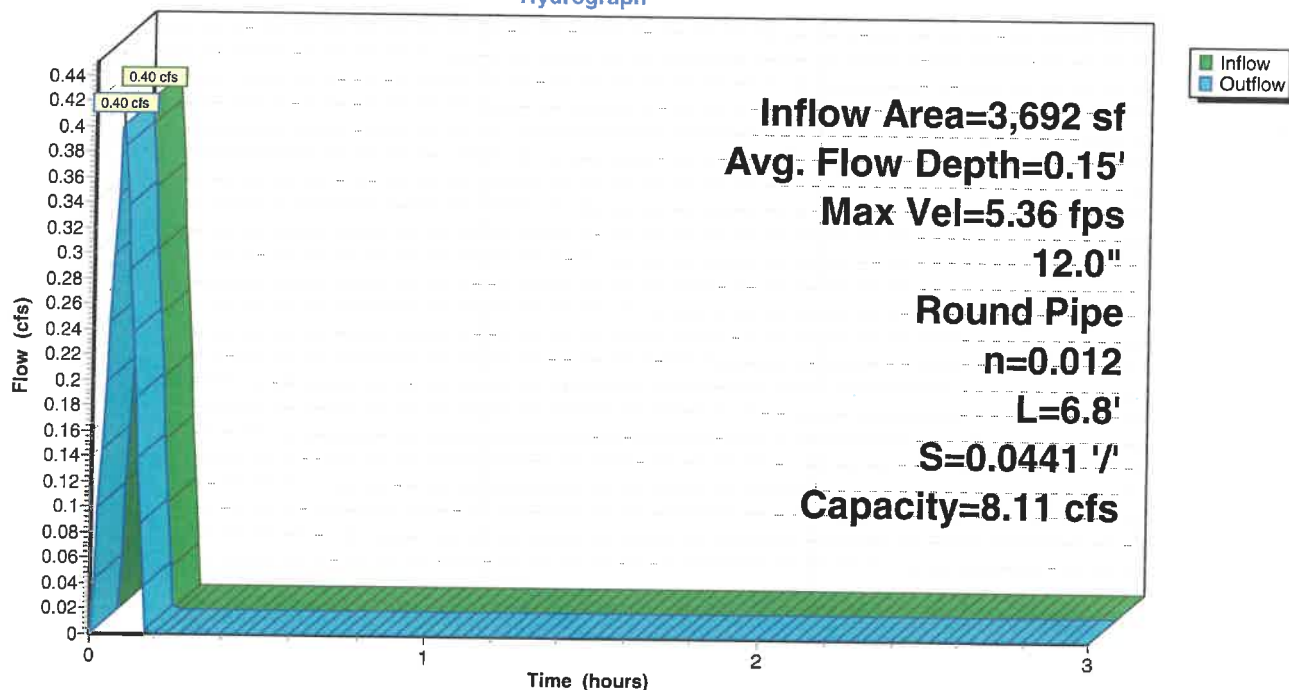
Peak Storage= 1 cf @ 0.08 hrs
 Average Depth at Peak Storage= 0.15', Surface Width= 0.72'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 8.11 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 6.8' Slope= 0.0441 '/'
 Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 10R: CB5 to DMH 3

Hydrograph



Summary for Reach 11R: CB6 to DMH 3

Inflow Area = 3,496 sf, 0.00% Impervious, Inflow Depth = 0.48" for 25-yr event
 Inflow = 0.45 cfs @ 0.08 hrs, Volume= 141 cf
 Outflow = 0.45 cfs @ 0.08 hrs, Volume= 141 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 4.37 fps, Min. Travel Time= 0.1 min
 Avg. Velocity= 3.36 fps, Avg. Travel Time= 0.1 min

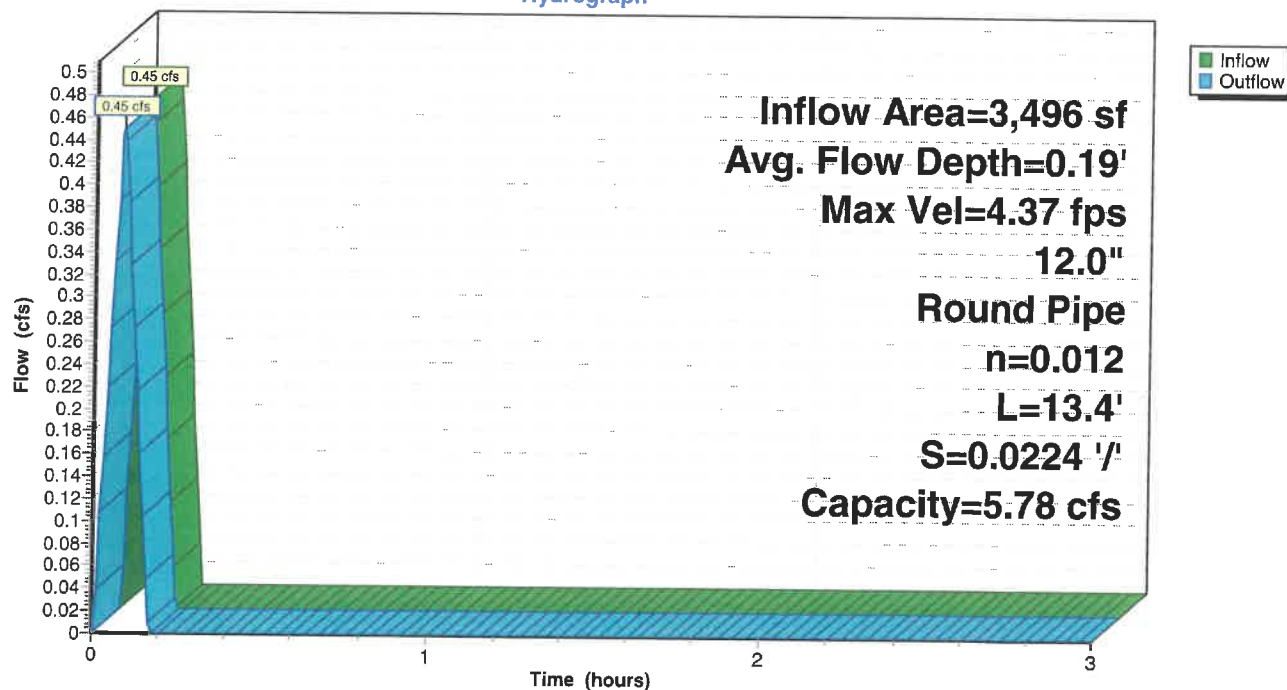
Peak Storage= 1 cf @ 0.08 hrs
 Average Depth at Peak Storage= 0.19' , Surface Width= 0.78'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.78 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 13.4' Slope= 0.0224 '/'
 Inlet Invert= 319.00', Outlet Invert= 318.70'



Reach 11R: CB6 to DMH 3

Hydrograph



Summary for Reach 12R: DMH 7 TO BASIN

Inflow Area = 12,862 sf, 0.00% Impervious, Inflow Depth = 0.44" for 25-yr event
Inflow = 1.38 cfs @ 0.11 hrs, Volume = 474 cf
Outflow = 1.35 cfs @ 0.13 hrs, Volume = 474 cf, Atten = 3%, Lag = 1.1 min

Routing by Stor-Ind+Trans method, Time Span = 0.00-3.00 hrs, dt = 0.01 hrs
Max. Velocity = 6.32 fps, Min. Travel Time = 0.6 min
Avg. Velocity = 2.15 fps, Avg. Travel Time = 1.9 min

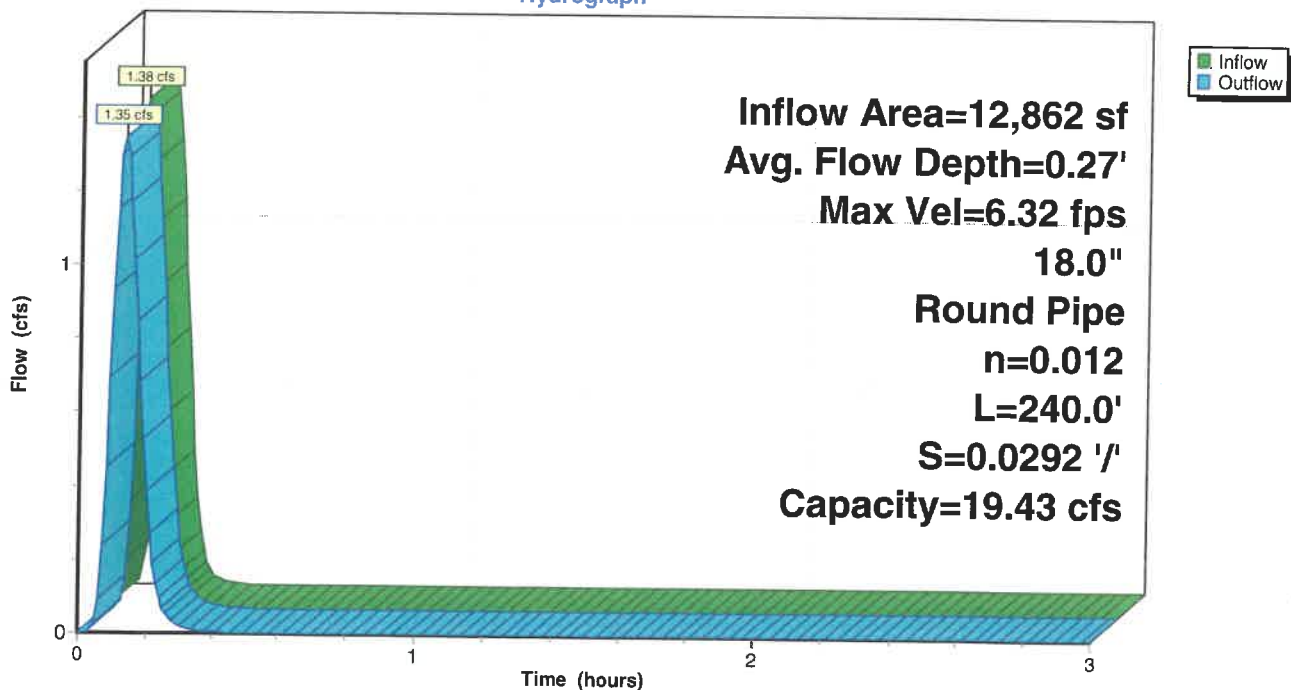
Peak Storage = 51 cf @ 0.12 hrs
Average Depth at Peak Storage = 0.27', Surface Width = 1.15'
Bank-Full Depth = 1.50' Flow Area = 1.8 sf, Capacity = 19.43 cfs

18.0" Round Pipe
n = 0.012 Corrugated PP, smooth interior
Length = 240.0' Slope = 0.0292 '/'
Inlet Invert = 290.00', Outlet Invert = 283.00'



Reach 12R: DMH 7 TO BASIN

Hydrograph



Summary for Reach 13R: CB7 TO DMH 4

Inflow Area = 4,352 sf, 0.00% Impervious, Inflow Depth = 0.41" for 25-yr event
 Inflow = 0.48 cfs @ 0.08 hrs, Volume = 149 cf
 Outflow = 0.48 cfs @ 0.08 hrs, Volume = 149 cf, Atten = 0%, Lag = 0.0 min

Routing by Stor-Ind+Trans method, Time Span = 0.00-3.00 hrs, dt = 0.01 hrs

Max. Velocity = 5.22 fps, Min. Travel Time = 0.0 min

Avg. Velocity = 4.00 fps, Avg. Travel Time = 0.0 min

Peak Storage = 1 cf @ 0.08 hrs

Average Depth at Peak Storage = 0.17', Surface Width = 0.76'

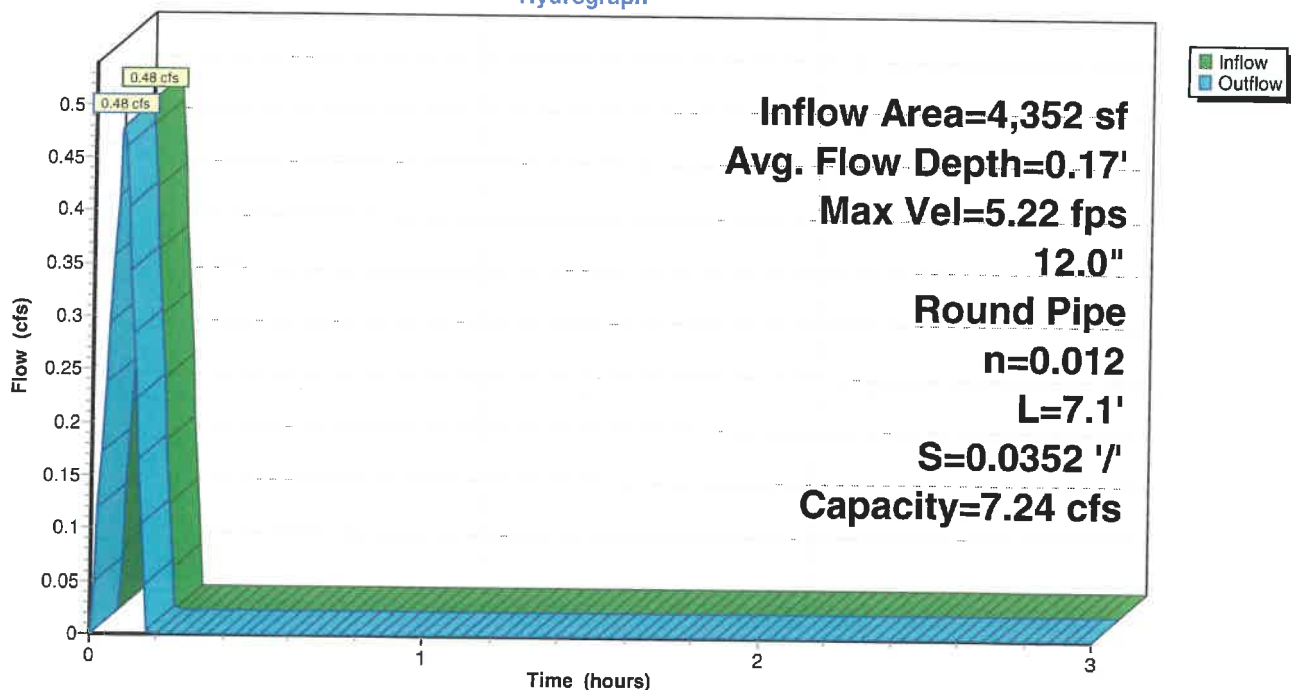
Bank-Full Depth = 1.00' Flow Area = 0.8 sf, Capacity = 7.24 cfs

12.0" Round Pipe

n = 0.012 Corrugated PP, smooth interior

Length = 7.1' Slope = 0.0352 '/'

Inlet Invert = 321.00', Outlet Invert = 320.75'

**Reach 13R: CB7 TO DMH 4****Hydrograph**

Summary for Reach 14R: CB 8 TO DMH 4

Inflow Area = 4,078 sf, 0.00% Impervious, Inflow Depth = 0.43" for 25-yr event
 Inflow = 0.47 cfs @ 0.08 hrs, Volume= 146 cf
 Outflow = 0.47 cfs @ 0.08 hrs, Volume= 146 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 4.14 fps, Min. Travel Time= 0.1 min
 Avg. Velocity= 3.19 fps, Avg. Travel Time= 0.1 min

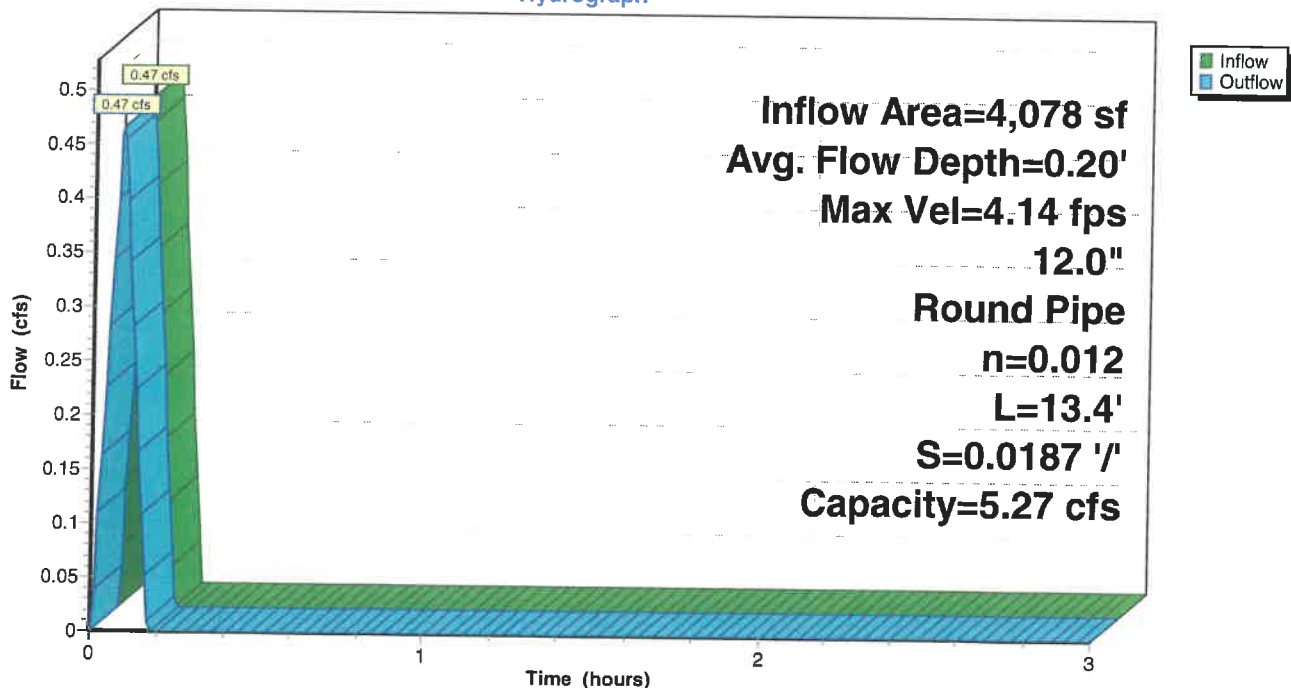
Peak Storage= 2 cf @ 0.08 hrs
 Average Depth at Peak Storage= 0.20', Surface Width= 0.80'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.27 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 13.4' Slope= 0.0187 '/'
 Inlet Invert= 321.00', Outlet Invert= 320.75'



Reach 14R: CB 8 TO DMH 4

Hydrograph



Summary for Reach 15R: DMH 4 TO DMH 5

Inflow Area = 8,430 sf, 0.00% Impervious, Inflow Depth = 0.42" for 25-yr event
 Inflow = 0.95 cfs @ 0.08 hrs, Volume= 296 cf
 Outflow = 0.92 cfs @ 0.09 hrs, Volume= 296 cf, Atten= 2%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 8.49 fps, Min. Travel Time= 0.2 min
 Avg. Velocity= 4.94 fps, Avg. Travel Time= 0.3 min

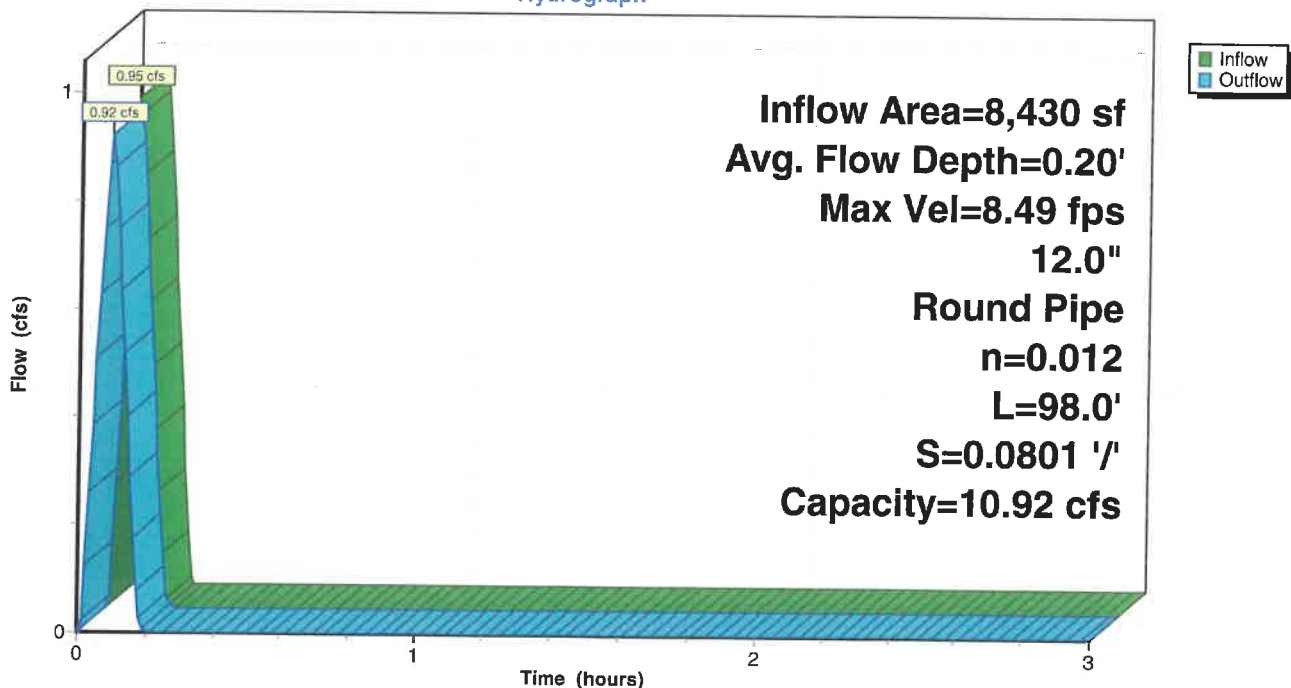
Peak Storage= 11 cf @ 0.09 hrs
 Average Depth at Peak Storage= 0.20', Surface Width= 0.80'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 10.92 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 98.0' Slope= 0.0801 '/'
 Inlet Invert= 316.20', Outlet Invert= 308.35'



Reach 15R: DMH 4 TO DMH 5

Hydrograph



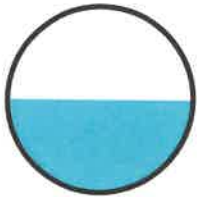
Summary for Reach 16R: DMH 5 TO DMH 6

Inflow Area = 36,775 sf, 0.00% Impervious, Inflow Depth = 0.38" for 25-yr event
 Inflow = 3.64 cfs @ 0.09 hrs, Volume = 1,167 cf
 Outflow = 3.47 cfs @ 0.11 hrs, Volume = 1,167 cf, Atten = 5%, Lag = 1.2 min

Routing by Stor-Ind+Trans method, Time Span = 0.00-3.00 hrs, dt = 0.01 hrs
 Max. Velocity = 5.73 fps, Min. Travel Time = 0.7 min
 Avg. Velocity = 1.53 fps, Avg. Travel Time = 2.6 min

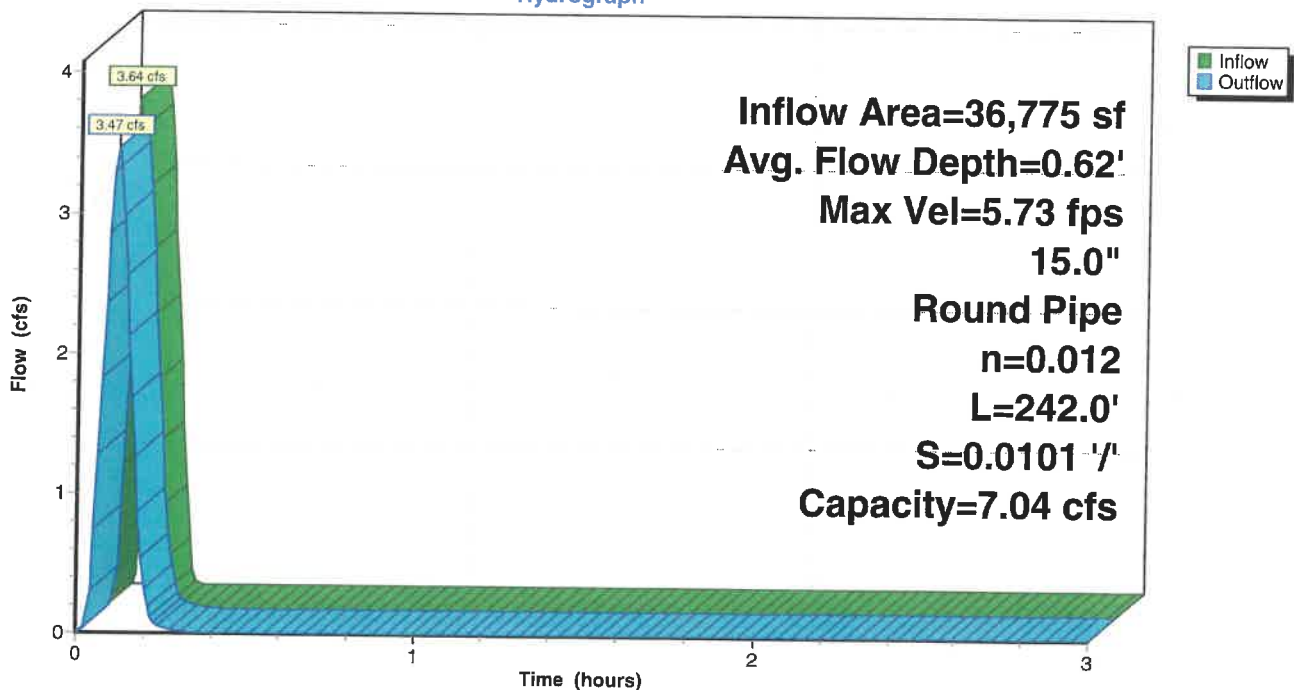
Peak Storage = 148 cf @ 0.10 hrs
 Average Depth at Peak Storage = 0.62', Surface Width = 1.25'
 Bank-Full Depth = 1.25' Flow Area = 1.2 sf, Capacity = 7.04 cfs

15.0" Round Pipe
 n = 0.012 Corrugated PP, smooth interior
 Length = 242.0' Slope = 0.0101 '/'
 Inlet Invert = 308.10', Outlet Invert = 305.65'



Reach 16R: DMH 5 TO DMH 6

Hydrograph



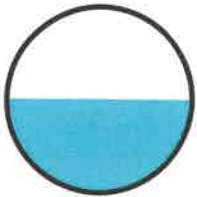
Summary for Reach 17R: DMH 6 TO DMH 5

Inflow Area = 28,345 sf, 0.00% Impervious, Inflow Depth = 0.37" for 25-yr event
 Inflow = 2.80 cfs @ 0.08 hrs, Volume= 872 cf
 Outflow = 2.72 cfs @ 0.09 hrs, Volume= 872 cf, Atten= 3%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 6.96 fps, Min. Travel Time= 0.2 min
 Avg. Velocity= 3.09 fps, Avg. Travel Time= 0.5 min

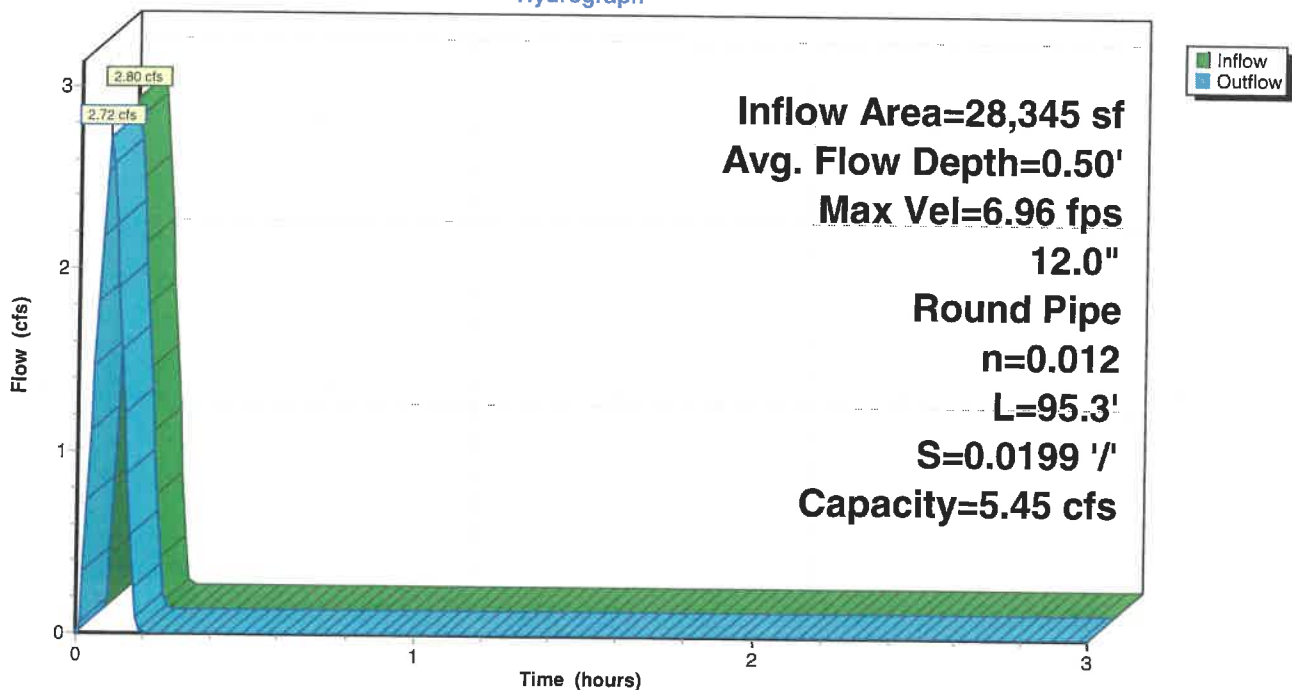
Peak Storage= 38 cf @ 0.09 hrs
 Average Depth at Peak Storage= 0.50', Surface Width= 1.00'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.45 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 95.3' Slope= 0.0199 '/'
 Inlet Invert= 310.25', Outlet Invert= 308.35'



Reach 17R: DMH 6 TO DMH 5

Hydrograph



Summary for Reach 18R: CB 10 TO DMH 6

Inflow Area = 14,794 sf, 0.00% Impervious, Inflow Depth = 0.36" for 25-yr event
 Inflow = 1.42 cfs @ 0.08 hrs, Volume= 441 cf
 Outflow = 1.41 cfs @ 0.08 hrs, Volume= 441 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 5.76 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 4.46 fps, Avg. Travel Time= 0.0 min

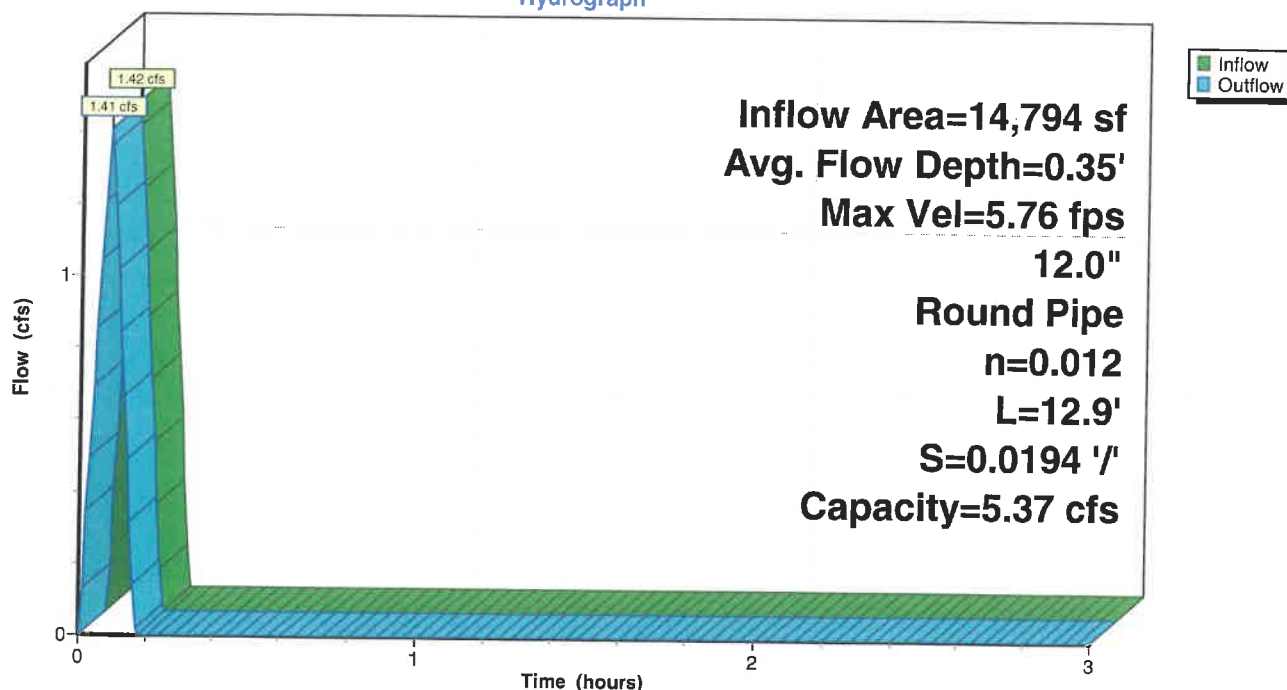
Peak Storage= 3 cf @ 0.08 hrs
 Average Depth at Peak Storage= 0.35' , Surface Width= 0.95'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.37 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 12.9' Slope= 0.0194 '/'
 Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 18R: CB 10 TO DMH 6

Hydrograph



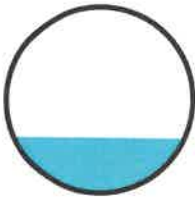
Summary for Reach 19R: CB 9 TO DMH 6

Inflow Area = 13,551 sf, 0.00% Impervious, Inflow Depth = 0.38" for 25-yr event
Inflow = 1.39 cfs @ 0.08 hrs, Volume= 431 cf
Outflow = 1.39 cfs @ 0.08 hrs, Volume= 431 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Max. Velocity= 7.14 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 5.48 fps, Avg. Travel Time= 0.0 min

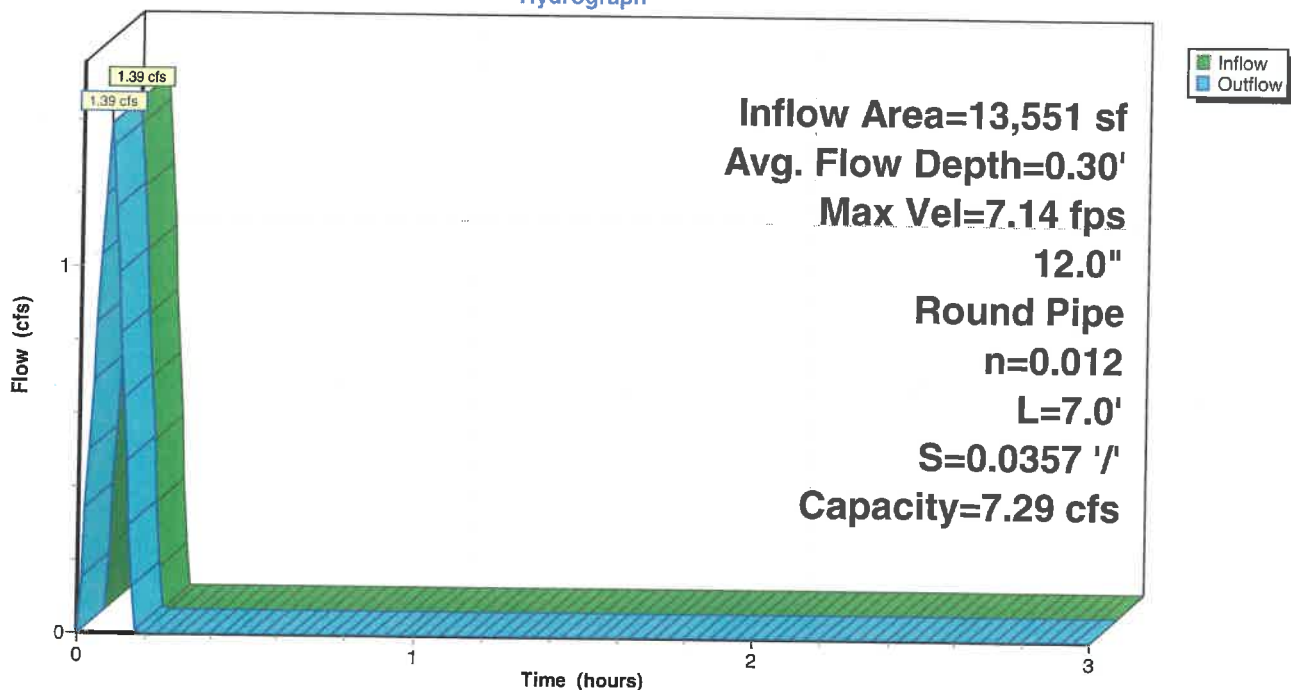
Peak Storage= 1 cf @ 0.08 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 0.91'
Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 7.29 cfs

12.0" Round Pipe
n= 0.012 Corrugated PP, smooth interior
Length= 7.0' Slope= 0.0357 '/'
Inlet Invert= 310.50', Outlet Invert= 310.25'



Reach 19R: CB 9 TO DMH 6

Hydrograph



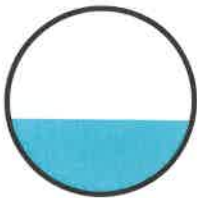
Summary for Reach 20R: DMH 6 to Basin

Inflow Area = 36,775 sf, 0.00% Impervious, Inflow Depth = 0.38" for 25-yr event
 Inflow = 3.47 cfs @ 0.11 hrs, Volume= 1,167 cf
 Outflow = 3.45 cfs @ 0.11 hrs, Volume= 1,167 cf, Atten= 1%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 7.54 fps, Min. Travel Time= 0.1 min
 Avg. Velocity= 2.11 fps, Avg. Travel Time= 0.2 min

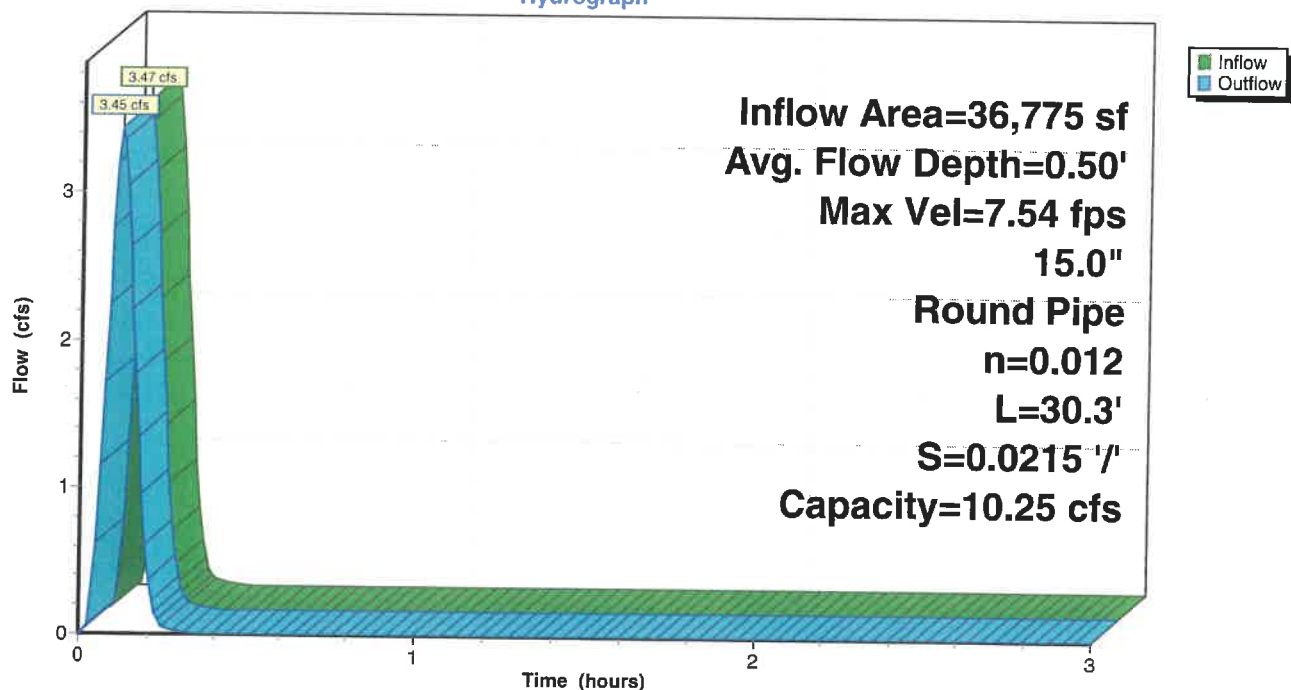
Peak Storage= 14 cf @ 0.11 hrs
 Average Depth at Peak Storage= 0.50' , Surface Width= 1.23'
 Bank-Full Depth= 1.25' Flow Area= 1.2 sf, Capacity= 10.25 cfs

15.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 30.3' Slope= 0.0215 '/'
 Inlet Invert= 305.65', Outlet Invert= 305.00'



Reach 20R: DMH 6 to Basin

Hydrograph



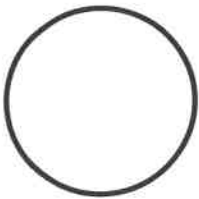
Summary for Reach 21R: 2-8" DI

Inflow Area = 318,022 sf, 0.00% Impervious, Inflow Depth = 0.00" for 25-yr event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity= 0.00 fps, Avg. Travel Time= 0.0 min

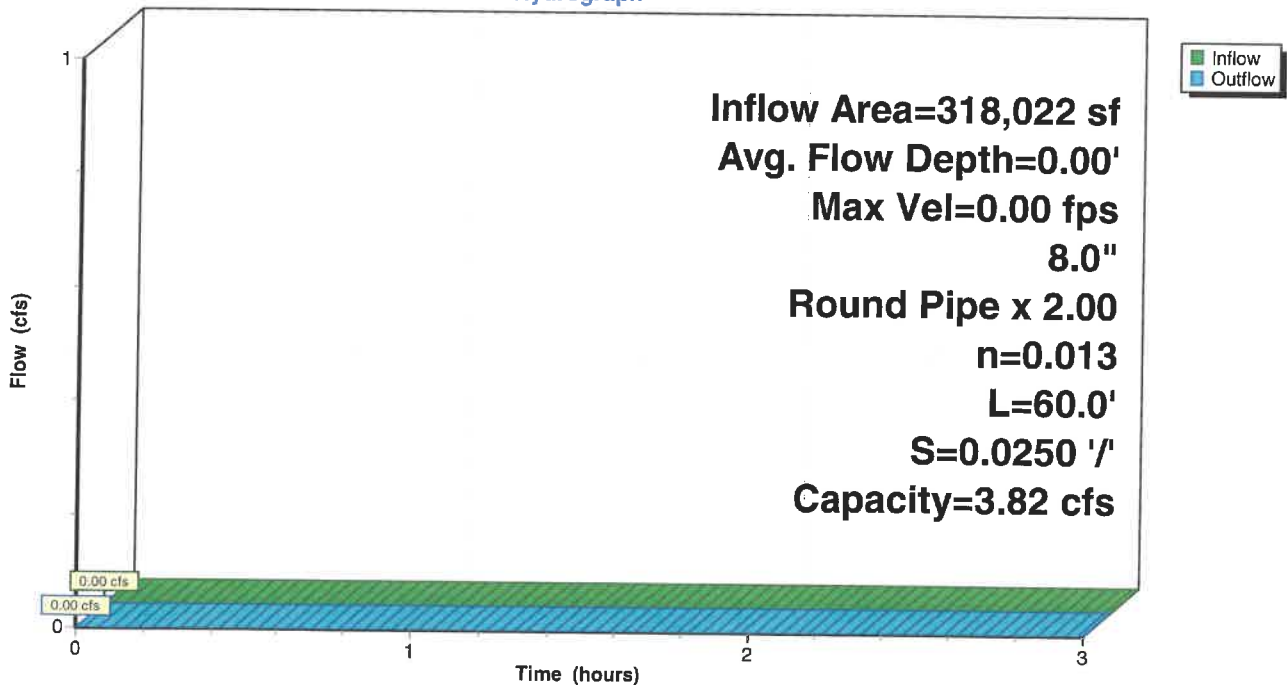
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.67' Flow Area= 0.7 sf, Capacity= 3.82 cfs

A factor of 2.00 has been applied to the storage and discharge capacity
8.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 60.0' Slope= 0.0250 '/'
Inlet Invert= 312.50', Outlet Invert= 311.00'



Reach 21R: 2-8" DI

Hydrograph



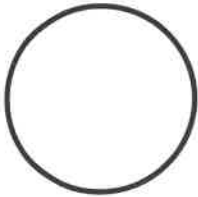
Summary for Reach 22R: Basin 2 Outfall

Inflow Area = 116,316 sf, 0.00% Impervious, Inflow Depth = 0.00" for 25-yr event
 Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

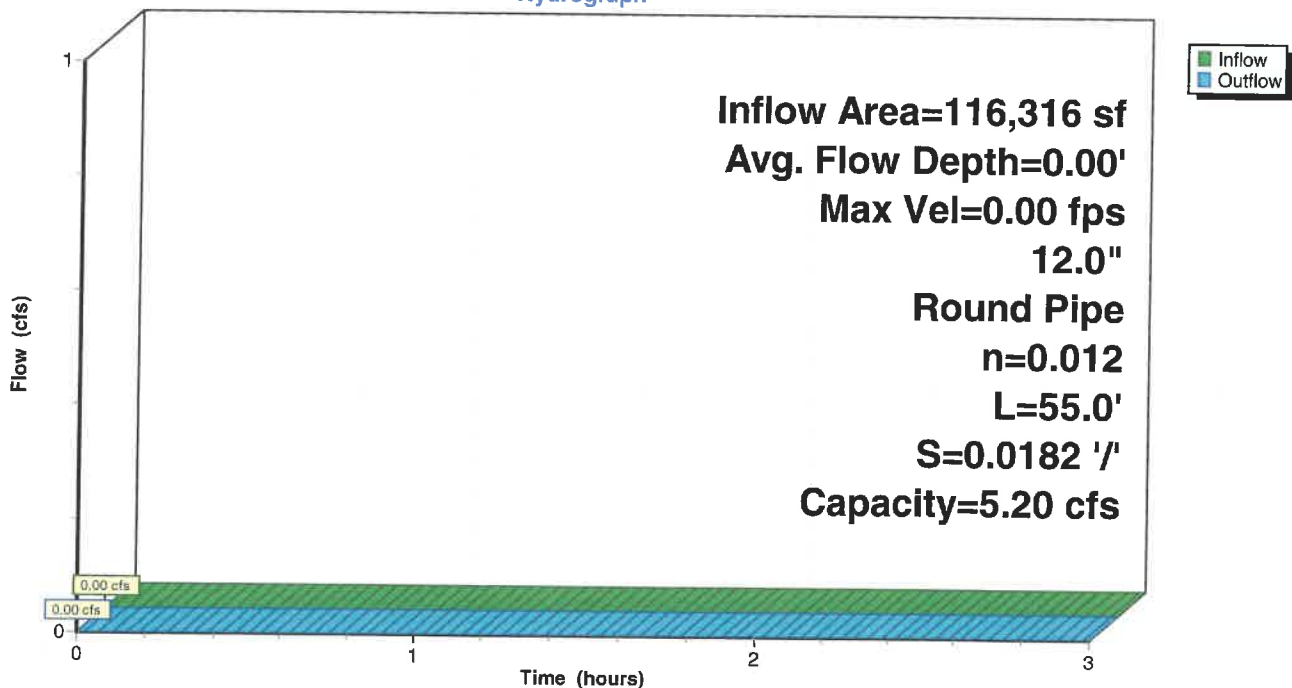
Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs
 Average Depth at Peak Storage= 0.00'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.20 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 55.0' Slope= 0.0182 '/'
 Inlet Invert= 304.00', Outlet Invert= 303.00'

**Reach 22R: Basin 2 Outfall**

Hydrograph



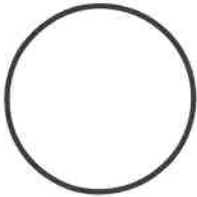
Summary for Reach 23R: Basin 1 Outfall

Inflow Area = 124,279 sf, 0.00% Impervious, Inflow Depth = 0.00" for 25-yr event
 Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

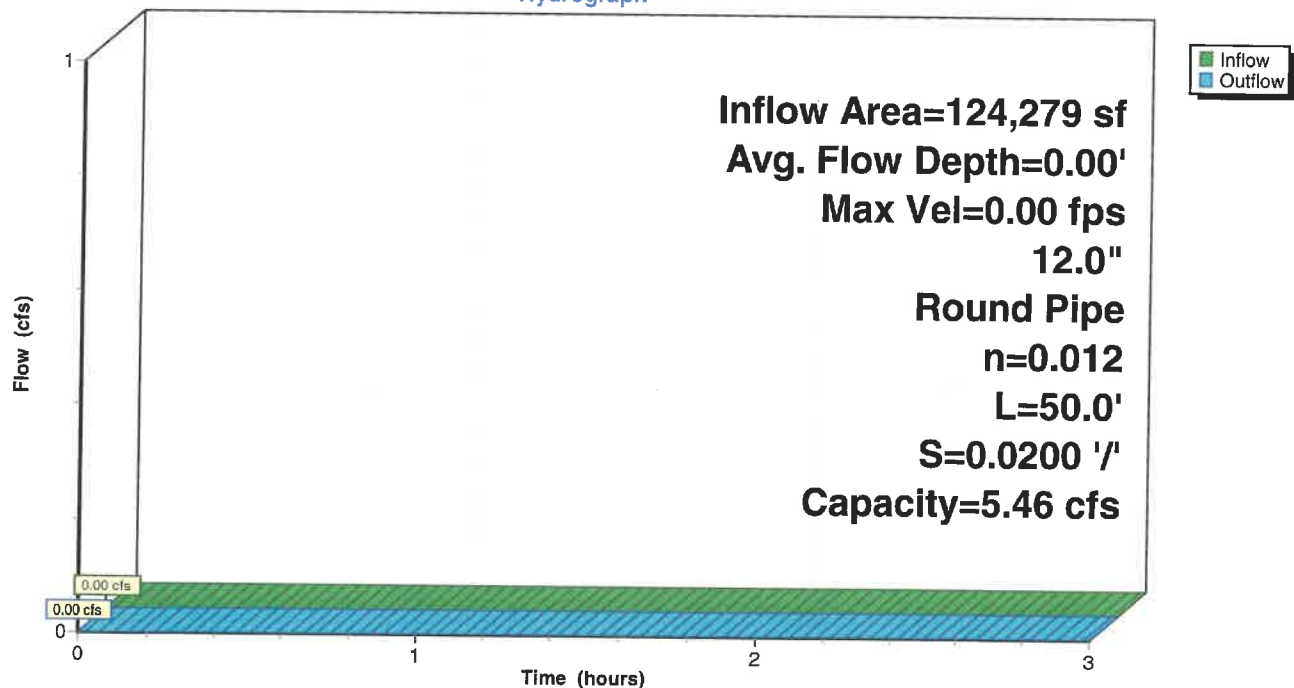
Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
 Avg. Velocity= 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs
 Average Depth at Peak Storage= 0.00'
 Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 5.46 cfs

12.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 50.0' Slope= 0.0200 '/'
 Inlet Invert= 281.00', Outlet Invert= 280.00'

**Reach 23R: Basin 1 Outfall**

Hydrograph



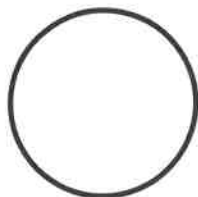
Summary for Reach 24R: Basin 3 Outfall

Inflow Area = 55,723 sf, 0.00% Impervious, Inflow Depth = 0.00" for 25-yr event
 Inflow = 0.00 cfs @ 0.00 hrs, Volume = 0 cf
 Outflow = 0.00 cfs @ 0.00 hrs, Volume = 0 cf, Atten = 0%, Lag = 0.0 min

Routing by Stor-Ind+Trans method, Time Span = 0.00-3.00 hrs, dt = 0.01 hrs
 Max. Velocity = 0.00 fps, Min. Travel Time = 0.0 min
 Avg. Velocity = 0.00 fps, Avg. Travel Time = 0.0 min

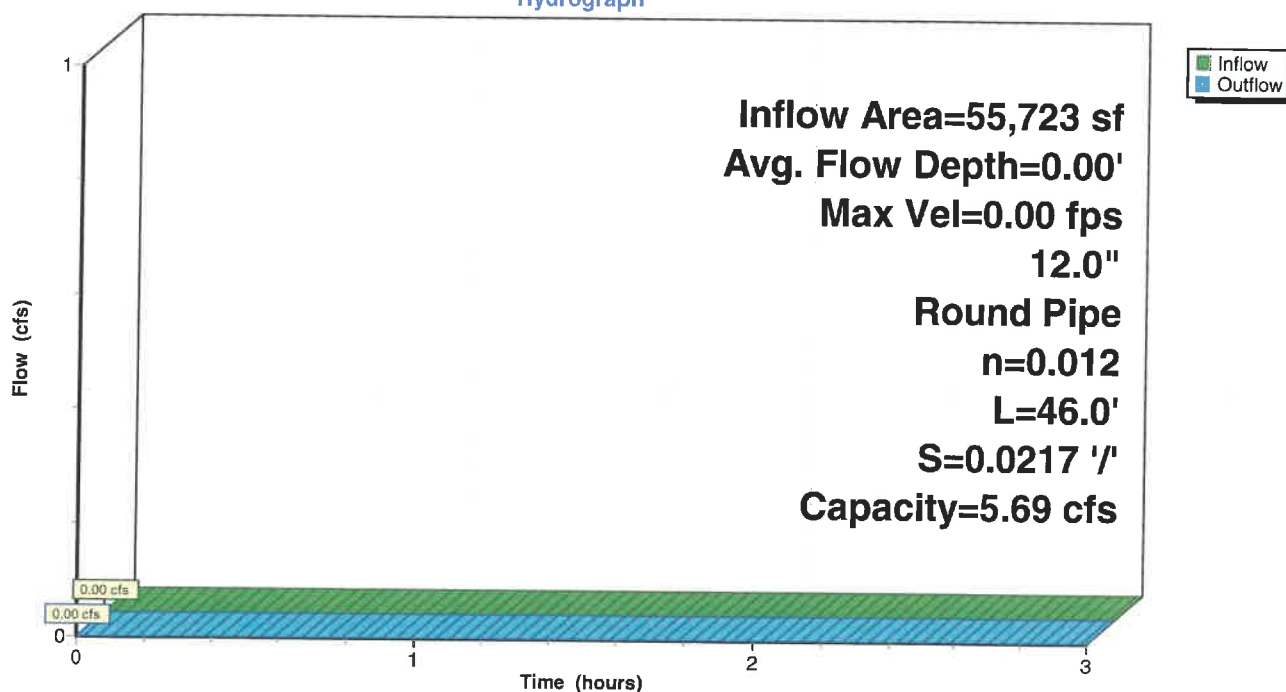
Peak Storage = 0 cf @ 0.00 hrs
 Average Depth at Peak Storage = 0.00'
 Bank-Full Depth = 1.00' Flow Area = 0.8 sf, Capacity = 5.69 cfs

12.0" Round Pipe
 n = 0.012 Corrugated PP, smooth interior
 Length = 46.0' Slope = 0.0217 '/'
 Inlet Invert = 289.00', Outlet Invert = 288.00'



Reach 24R: Basin 3 Outfall

Hydrograph



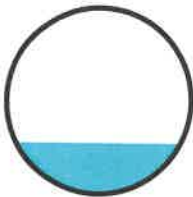
Summary for Reach 25R: 24" Bypass Culvert

Inflow Area = 492,614 sf, 0.00% Impervious, Inflow Depth = 0.04" for 25-yr event
 Inflow = 4.59 cfs @ 0.09 hrs, Volume= 1,459 cf
 Outflow = 4.50 cfs @ 0.09 hrs, Volume= 1,459 cf, Atten= 2%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
 Max. Velocity= 6.62 fps, Min. Travel Time= 0.2 min
 Avg. Velocity= 2.92 fps, Avg. Travel Time= 0.4 min

Peak Storage= 52 cf @ 0.09 hrs
 Average Depth at Peak Storage= 0.54' , Surface Width= 1.78'
 Bank-Full Depth= 2.00' Flow Area= 3.1 sf, Capacity= 28.30 cfs

24.0" Round Pipe
 n= 0.012 Corrugated PP, smooth interior
 Length= 75.0' Slope= 0.0133 '/'
 Inlet Invert= 292.50', Outlet Invert= 291.50'



Reach 25R: 24" Bypass Culvert

Hydrograph

